

Terrace Views Subdivision, Parton Road, Tauranga

Stage 3D-3E Geotechnical
Completion Report

Zariba Properties Limited

Reference: 247593-320

Revision: 3

8 March 2019

aurecon

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Document control record

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Document control					aurecon	
Report title		Stage 3D-3E Geotechnical Completion Report				
Document ID		247593-0000-REP-GG-0001	Project number		247593-320	
File path		P:\Projects\247593 - Zariba HASHA Parton Road Papamoa\009 - Geotechnical\Geotechnical Completion Reports\Stage 3D-3E\Stage 3D-3E GCR\Issue				
Client		Zariba Properties Limited				
Client contact		Jeff Fletcher	Client reference			
Rev	Date	Revision details/status	Author	Reviewer	Verifier (if required)	Approver
0	1 February 2019	First Issue	M Buob	R Griffiths		R Griffiths
1	5 February 2019	Update to include BRL discussion and confirm finished ground levels (as opposed to engineered fill level)	M Buob	R Griffiths	R Griffiths	B O'Loughlin
2	13 February 2019	Update to Appendix D to include Final Logs	M Buob	R Griffiths	R Griffiths	B O'Loughlin
3	8 March 2019	Update Section 5.4.5 On site stormwater disposal to include Lots 475-479 and updated discussion on minimum building platform level as per RC26514	M Buob		R Griffiths	B O'Loughlin
Current revision		3				

Approval			
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1 Introduction

Zariba Properties Limited (Zariba, the client) has commissioned Aurecon New Zealand Ltd (Aurecon) to prepare a Geotechnical Completion Report (GCR) for Stage 3D and 3E of the Terrace Views Subdivision development. Terrace Views Subdivision is a multistage residential subdivision planned to be developed in four stages. This report is limited to Stage 3D and 3E (the 'Site') which collectively comprise a total of 64 residential lots.

The Terrace Views Subdivision site was originally investigated by Aurecon in 2011 as part of an initial geotechnical assessment to support commercial and light industrial land development. Since preparing the 2011 report, the original development proposal was revised to residential land use under a Special Housing Accord resource consent application. To support the new resource consent application for residential subdivision, additional geotechnical investigations and reporting was completed by Aurecon in 2016. Following this, Aurecon have provided further geotechnical investigations, design, and construction observations for our client through all stages of development.

The following reports were prepared by Aurecon which are relevant to the Site:

- Geotechnical Suitability of Land for Development Papamoa East Block, Parton Road Papamoa. Reference 220166-001, Revision 1, dated 06 September 2011.
- Geotechnical review of landform design and suitability for residential subdivision, Reference 247593-Geotechnical Review Parton Road Subdivision, Revision 1, dated 29 January 2016
- Geotechnical suitability for residential subdivision of Stages 1D, 3A and 3B of the Parton Road Subdivision, Papamoa, dated 15 March 2016

The reader is referred to the above reports for background to the geotechnical investigations carried out at the Site, interpretation and assessment of soil conditions and requirements for development.

This Geotechnical Completion Report (GCR) relates only to the residential Lots 391 to 400 (inclusive) and 439 to 493 (inclusive) within Stage 3D and 3E of the subdivision. The GCR presents discussion on:

- Earthworks and compliance testing
- Geotechnical investigations of each individual residential lot to determine suitability for building
- Specific requirements for building development on each residential lot

Data is provided within the report appendices that support the comments, conclusions and recommendations given in this report. A statement of professional opinion as to the suitability of land for building is provided in Appendix A. This report forms part of the s224 application to Tauranga City Council.

2 Site location and description

The Terrace Views Subdivision is located at 162 Parton Road, Papamoa and measures an area of approximately 10 hectares. The original land title was defined by the legal property boundary for Lot 1 and 2 of DP 429801, Lot 1 and 2 of DP 325745, Lot 1, 2 and 3 of DP 82613, as well as land to the north-east formerly owned by Tauranga City Council. The property is roughly triangular in shape and is located between Parton Road and the Tauranga Eastern Link corridors.

Stages 3D and 3E are located within the central-western portion of the Terrace Views Subdivision, to the west of the Te Okuroa Drive extension. Stage 3D and 3E is bordered by Garrison Road to the east, a Garden Nursery to the south, and Fetlock way to the west. The overall Terrace Views Subdivision layout and Stage 3D and 3E location is shown on the subdivision masterplan included in Appendix B along with the following Stage 3D and 3E drawings:

- The pre-development landform - 247593-320-DRG-GG-0001-A
- The cut to fill earthworks and compliance test location plan - 247593-320-DRG-GG-0002-A
- The as built subgrade landform and lot by lot test location plan - 247593-320-DRG-GG-0003-A

3 Proposed subdivision development

Stage 3D and 3E of the development comprises a total of 64 residential lots. It is anticipated that dwellings within each of the lots developed within Stage 3D and 3E will be shallow founded at the designed platform level, designated to be at or above a minimum elevation of +5.1 mRL, as specified by Tauranga City Council Resource Consent RC26514.

It is our understanding that the proposed residential buildings will be one or two storey residential structures designed in accordance with the requirements of NZS 3604:2011 *Timber Framed Buildings*. All lots will be serviced with wastewater and town water supply at each lot boundary. Stormwater from residential lots will be disposed of via on-site ground soakage within each lot. Stormwater from the internal road network will be reticulated and discharged into a swale drain which flows to a wetland storm water pond on the southern boundary of the Terrace Views subdivision.

4 Geotechnical considerations

4.1 Pre-development geotechnical assessment

The geotechnical requirements for development of the Site are outlined in the "Suitability for Subdivision" reports dated 15 March. The key geotechnical requirements and conclusions from these reports were consistent, and are summarised as follows:

4.1.1 Peat

Where exposed in the natural foundation subgrade, Aurecon advised the undercut and removal of peat and replacement with engineered fill.

"The peat undercut and replacement is expected to largely remove the risk of settlement (both primary and secondary creep consolidation) affecting the development. As discussed in our previous reports, settlement of the underlying loose sands and firm sandy silt layers may occur as a result of replacing the peat with denser engineered fill in combination with raising site levels with engineered fill."

4.1.2 Settlement

"The magnitude of settlement due to fill placement is likely to vary based on a number of factors including the fill thickness, groundwater levels and nature of the underlying sediments below the peat."

"Considering the sandy nature of the underlying soils, fill induced settlements are expected to occur rapidly during construction. Settlement monitoring is proposed as part of the earthworks contract. The settlement data will be reviewed by a TCC Category 1 geo-professional to confirm timing for building development."

4.1.3 Groundwater level

"Groundwater was encountered within the test pits excavated within peat areas. Groundwater was generally encountered approximately 1.0m below ground level (~+2.7mRL)."

4.1.4 Liquefaction

"For elevated areas of the site, liquefaction may occur below the groundwater table which, following earthworks, is expected to be around 2.4-3.5m below finished ground levels. Given the assumed thickness of the non-liquefiable crust (ranging between 2.4 and 5.0m), the likely effects of liquefaction for surface foundations are anticipated to meet the requirements of the NZ Building Code which states that buildings must remain serviceable following a SLS event with only minor repair. During a ULS event buildings must retain their structural integrity such that they can be safely evacuated, however the resulting damage may require significant repair and/or rebuild. With this in mind, raft foundations for future residential buildings are recommended for the purpose of residential building construction."

4.1.5 Foundations

"We understand that the subdivision will comprise residential buildings that include materials composition and dimensional characteristics as defined by NZS 3604:2011. Given that the site is located within an area of moderate liquefaction potential we recommend the specification of engineer designed or proprietary raft foundations for each residential building."

"It is our expectation that following earthworks cuts and filling, all structures can be shallow founded in accordance with the provisions of the Standard. Each building structure shall be supported on a shallow founded engineer designed or proprietary raft foundation (i.e. TC2 Rib-Raft or equivalent)".

"Final foundation recommendations and bearing capacities shall be confirmed as part of the geotechnical completion report for each stage of development following lot by lot testing and certification of the fill."

4.1.6 Stormwater and effluent disposal

"Stormwater from roads will be collected and disposed of into the proposed subdivision stormwater swales and ponds. It is proposed to dispose of storm water from individual lots via in-ground soakage." "We recommend soakage percolation testing is carried out following filling to confirm the viability of onsite soakage systems and to provide parameters for design".

4.2 Site earthworks

The earthworks covering Stage 3D and 3E were completed between October 2017 and January 2019 by JMC. Pre-development, final development and cut to fill drawings are included in Appendix B.

The original landform within Stage 3D and 3E comprised a combination of elevated dunes covering the northern boundary of Stage 3D and lower lying areas capped with a layer of peat, approximately 1.5 to 2.0 metres in thickness).

To achieve the design landform, cut and fill earthworks were required within the Site to strip topsoil and excavate underlying peat to expose a natural dune sand subgrade at ~ 1 m RL. The excavations were then replaced with site-won engineered sand fill up to ~+2.7 mRL using site-won dune sand. Pumice sand imported from Paengaroa Sands, Marsh Street, and Marbles Quarry was then placed on the dune sand and generally above groundwater level, up to the final design subgrade level.

The earthworks equipment comprised mainly excavators to carry out localised undercuts, and motor scrapers to transport and compact the fill. Throughout the earthworks operation JMC controlled groundwater using large pumps installed in deep subsoil trenches in the western, southern and south-eastern areas of the site. These pumps effectively dewatered the entire excavation allowing bulk filling to proceed without the need for underwater filling.

4.3 Earthworks subgrade inspection and compliance testing

4.3.1 Subgrade observations

Aurecon carried out regular geotechnical subgrade inspections following topsoil stripping and peat undercutting. Instruction to commence earthworks filling was only provided following certification of the subgrade by one of Aurecon's Geotechnical Engineers.

4.3.2 Earthworks compliance testing specification

Fill compliance testing was undertaken during earthworks construction. Compliance testing was carried out in accordance with the contract earthworks specification (refer to 152870 - Zariba Stage 2 Earthworks Scope and Specification dated 17 February 2017). The specification provided requirements for subgrade inspection, laboratory and field density testing, and compaction criteria. The specification required all engineered fill to be tested in accordance with NZS4431:1989 Code for *earth fill for residential development* as required under the Tauranga City Council Infrastructure Development Code (2017). All laboratory testing was specified to be

carried out by an independent IANZ-accredited laboratory. The testing schedule specified for Stage 3D and 3E, as per the requirements of the contract specification, is presented in Table 1.

Table 1: Testing schedule for engineered sand fill

Soil Classification & Specific Gravity	Standard Compaction Test	In-situ Density Test (NDM)	DCP Tests Cohesionless Soils (sands / gravels)
4 specific gravity tests per soil type (all soils). 4 PSD grading curves per representative soil type (granular soils)	Four compaction tests per representative soil type. NZS 4402, Test 4.1.1. Minimum of 5 points shall be determined. Lab shear vanes for each point (for cohesive soils only). Material description to be recorded in accordance with NZGS guidelines	Minimum of 1 NDM test per 500 m³ of filling placed evenly distributed over the extent filling. Testing at every 750 mm lift (or part thereof). 1x oven corrected moisture content per NDM test	1 test per in-situ density test. DCP tests to be to depth of not less than 0.9m.

As the source fill material tested comprised predominantly sand, percentage compaction was used to determine fill compliance. Due to a lack of site-won sand for the earthworks, Paengaroa, Marsh, and Marbles pumice sand was selected as the other source of fill material. Target maximum dry density was selected from the field Plateau Testing undertaken as part of the earthworks.

The acceptance criteria for the fill and pumice sand compaction was:

- DCP: Minimum 5 blows per 100mm
- NDM: Minimum 95% Maximum Dry Density

4.3.3 Results of testing

The total volume of fill is approximately 45,500m³ based on the as-built cut-fill plan provided by Lysaght Consultants Ltd (LCL). Based on this volume 92 NDMs and DCP tests were required to meet the specification.

Nuclear Densometer testing (NDM) were undertaken at 121 locations with accompanying DCP tests conducted at 110 of those locations. Based on this number and distribution of testing, Aurecon was satisfied with compliance testing in relation to the earthworks specification.

The targets for the fill material were initially based upon maximum dry density curves. On 27 November 2018 three plateau tests were conducted on fill material from Paengaroa Quarry, Marbles Quarry, and reworked Paengaroa material to adjust the target maximum dry density. All NDM tests targets were adjusted and reissued based upon these targets and the type of material placed. All NDM compliance tests were carried out by Fulton Hogan at their IANZ-accredited Tauranga laboratory.

A summary of the results of NDM testing are presented in Table 2. Test locations for all compliance testing are shown on the Cut/Fill drawing in Appendix B, and the laboratory reports are included in Appendix C.

Table 2: Summary of nuclear densometer testing (conducted by Fulton Hogan)

Test Number	Material	Oven adjusted dry density ¹ (t/m ³)	Oven corrected moisture content (%)	Target Maximum dry density (t/m ³) ⁴	Compaction ² (%)
245	Paengaroa Rhyolite Pumice Sand	1.36	24.3	1.22	111%
246	Paengaroa Rhyolite Pumice Sand	1.28	25.7	1.22	105%
247	Paengaroa Rhyolite Pumice Sand	1.35	24.2	1.22	111%
248RT ³	Paengaroa Rhyolite Pumice Sand	1.47	15.5	1.22	120%
249	Paengaroa Rhyolite Pumice Sand	1.37	25.1	1.22	112%
250RT	Paengaroa Rhyolite Pumice Sand	1.41	17.4	1.22	116%
251	Paengaroa Rhyolite Pumice Sand	1.33	26.4	1.22	109%
252	Paengaroa Rhyolite Pumice Sand	1.35	27.1	1.22	111%
253	Paengaroa Rhyolite Pumice Sand	1.34	23.6	1.22	110%
254	Paengaroa Rhyolite Pumice Sand	1.35	24.8	1.22	111%
275	Marbles/Paengaroa (70/30 blend)	1.51	19	1.27	119%
277	Marbles/Paengaroa (70/30 blend)	1.54	16.7	1.27	121%
279	Marbles/Paengaroa (70/30 blend)	1.52	15.9	1.27	120%
286	Marbles/Paengaroa (70/30 blend)	1.42	17.1	1.27	112%
287	Marbles/Paengaroa (70/30 blend)	1.45	19.9	1.27	114%
288RT	Paengaroa Rhyolite Pumice Sand	1.36	24.4	1.22	111%
295	Marbles/Paengaroa (70/30 blend)	1.37	21.1	1.27	108%
296	Paengaroa Rhyolite Pumice Sand	1.4	19.3	1.22	115%
297	Marbles/Paengaroa (70/30 blend)	1.37	18.4	1.27	108%
298	Paengaroa Rhyolite Pumice Sand	1.54	16.3	1.22	126%
299	Paengaroa Rhyolite Pumice Sand	1.4	21.3	1.22	115%
300RT	Paengaroa Rhyolite Pumice Sand	1.46	21.6	1.22	120%
301	Paengaroa Rhyolite Pumice Sand	1.42	19.6	1.22	116%
302	Marsh Pit Pumice Sand	0.78	73.9	0.82	95%
303RT	Marsh Pit Pumice Sand	1.07	34.6	0.82	130%
315	Paengaroa Rhyolite Pumice Sand	1.38	23.2	1.22	113%
316	Paengaroa Rhyolite Pumice Sand	1.39	26.7	1.22	114%
317	Paengaroa Rhyolite Pumice Sand	1.4	24.2	1.22	115%
318	Paengaroa Rhyolite Pumice Sand	1.45	23.3	1.22	119%
319	Paengaroa Rhyolite Pumice Sand	1.47	22.8	1.22	120%
320	Paengaroa Rhyolite Pumice Sand	1.41	26	1.22	116%
321	Paengaroa Rhyolite Pumice Sand	1.46	23	1.22	120%
322	Paengaroa Rhyolite Pumice Sand	1.37	25.3	1.22	112%
324	Paengaroa Rhyolite Pumice Sand	1.42	21.6	1.22	116%
337	Paengaroa Rhyolite Pumice Sand	1.42	19.1	1.22	116%
338	Paengaroa Rhyolite Pumice Sand	1.54	17.6	1.22	126%
339	Paengaroa Rhyolite Pumice Sand	1.4	23.7	1.22	115%
340	Paengaroa Rhyolite Pumice Sand	1.44	18.5	1.22	118%

Test Number	Material	Oven adjusted dry density ¹ (t/m ³)	Oven corrected moisture content (%)	Target Maximum dry density (t/m ³) ⁴	Compaction ² (%)
341	Paengaroa Rhyolite Pumice Sand	1.48	21	1.22	121%
342	Paengaroa Rhyolite Pumice Sand	1.47	17	1.22	120%
343	Paengaroa Rhyolite Pumice Sand	1.47	20.7	1.22	120%
344	Paengaroa Rhyolite Pumice Sand	1.42	25.9	1.22	116%
345RT	Paengaroa Rhyolite Pumice Sand	1.38	23.1	1.22	113%
346	Paengaroa Rhyolite Pumice Sand	1.41	23.7	1.22	116%
347RT	Paengaroa Rhyolite Pumice Sand	1.42	23.1	1.22	116%
348	Paengaroa Rhyolite Pumice Sand	1.45	20.5	1.22	119%
349	Paengaroa Rhyolite Pumice Sand	1.52	17.7	1.22	125%
350RT	Paengaroa Rhyolite Pumice Sand	1.39	24.2	1.22	114%
351	Paengaroa Rhyolite Pumice Sand	1.4	23.4	1.22	115%
366	Paengaroa Rhyolite Pumice Sand	1.41	28.7	1.22	116%
367	Paengaroa Rhyolite Pumice Sand	1.37	24.9	1.22	112%
368	Paengaroa Rhyolite Pumice Sand	1.5	17.2	1.22	123%
369	Paengaroa Rhyolite Pumice Sand	1.45	26	1.22	119%
370	Paengaroa Rhyolite Pumice Sand	1.36	21	1.22	111%
371	Paengaroa Rhyolite Pumice Sand	1.34	21.2	1.22	110%
372	Paengaroa Rhyolite Pumice Sand	1.32	24.6	1.22	108%
373	Paengaroa Rhyolite Pumice Sand	1.29	22.5	1.22	106%
374	Paengaroa Rhyolite Pumice Sand	1.35	23.7	1.22	111%
375	Paengaroa Rhyolite Pumice Sand	1.35	21.9	1.22	111%
376	Marbles/Paengaroa (70/30 blend)	1.2	29.9	1.27	94%
380	Paengaroa Rhyolite Pumice Sand	1.25	34.5	1.22	102%
381	Paengaroa Rhyolite Pumice Sand	1.46	21.9	1.22	120%
382	Paengaroa Rhyolite Pumice Sand	1.27	30.9	1.22	104%
383	Paengaroa Rhyolite Pumice Sand	1.36	28	1.22	111%
384	Paengaroa Rhyolite Pumice Sand	1.37	28	1.22	112%
390	Paengaroa Rhyolite Pumice Sand	1.23	26	1.22	101%
391	Paengaroa Rhyolite Pumice Sand	1.15	33.7	1.22	94%
392	Paengaroa Rhyolite Pumice Sand	1.5	15.5	1.22	123%
393	Paengaroa Rhyolite Pumice Sand	1.36	28.1	1.22	111%
394	Paengaroa Rhyolite Pumice Sand	1.32	23.2	1.22	108%
395	Paengaroa Rhyolite Pumice Sand	1.44	23.7	1.22	118%
396	Paengaroa Rhyolite Pumice Sand	1.43	19.4	1.22	117%
397	Paengaroa Rhyolite Pumice Sand	1.55	10.2	1.22	127%
398	Paengaroa Rhyolite Pumice Sand	1.24	27.2	1.22	102%
399	Paengaroa Rhyolite Pumice Sand	1.36	22.8	1.22	111%
404	Marbles/Paengaroa (50/50 blend)	1.24	25.7	1.26	98%
405	Marbles/Paengaroa (50/50 blend)	1.29	27.8	1.26	102%

Test Number	Material	Oven adjusted dry density ¹ (t/m ³)	Oven corrected moisture content (%)	Target Maximum dry density (t/m ³) ⁴	Compaction ² (%)
406	Marbles/Paengaroa (70/30 blend)	1.47	19.5	1.27	116%
407	Marbles/Paengaroa (50/50 blend)	1.47	18.9	1.26	117%
408	Marbles/Paengaroa (50/50 blend)	1.25	21.7	1.26	99%
409	Marbles/Paengaroa (50/50 blend)	1.19	31.8	1.26	94%
411	Marbles/Paengaroa (50/50 blend)	1.21	33.1	1.26	96%
413	Marbles/Paengaroa (50/50 blend)	1.23	30.8	1.26	98%
414	Marbles/Paengaroa (70/30 blend)	1.26	27	1.27	99%
415	Marbles/Paengaroa (70/30 blend)	1.23	26.4	1.27	97%
416	Marbles/Paengaroa (70/30 blend)	1.3	24.2	1.27	102%
417	Marbles/Paengaroa (70/30 blend)	1.3	21.9	1.27	102%
426	Paengaroa Rhyolite Pumice Sand	1.5	19.5	1.22	123%
427	Paengaroa Rhyolite Pumice Sand	1.14	40.5	1.22	93%
428	Marbles/Paengaroa (50/50 blend)	1.23	25.8	1.26	98%
429	Marbles/Paengaroa (50/50 blend)	1.26	30.7	1.26	100%
430	Marbles/Paengaroa (50/50 blend)	1.31	27.7	1.26	104%
431	Marbles/Paengaroa (50/50 blend)	1.3	22.5	1.26	103%
432	Paengaroa Rhyolite Pumice Sand	1.29	23.7	1.22	106%
433	Paengaroa Rhyolite Pumice Sand	1.31	25.4	1.22	107%
434RT	Marbles/Paengaroa (50/50 blend)	1.3	24.1	1.26	103%
435	Marbles Pumice Sand	1.28	30.1	1.29	99%
436	Marbles Pumice Sand	1.32	24.3	1.29	102%
437	Marbles Pumice Sand	1.27	26.4	1.29	98%
438	Marbles Pumice Sand	1.27	26.5	1.29	98%
439	Marbles Pumice Sand	1.34	16.8	1.29	104%
440RT	Marbles Pumice Sand	1.3	21.3	1.29	101%
441	Marbles Pumice Sand	1.33	27.9	1.29	103%
442	Marbles Pumice Sand	1.31	17.7	1.29	102%
443	Marbles Pumice Sand	1.33	19.7	1.29	103%
444	Marbles Pumice Sand	1.31	19.5	1.29	102%
445	Marbles Pumice Sand	1.37	19	1.29	106%
446	Paengaroa Rhyolite Pumice Sand	1.25	30.8	1.22	102%
447	Paengaroa Rhyolite Pumice Sand	1.27	27.3	1.22	104%
448	Paengaroa Rhyolite Pumice Sand	1.22	28.5	1.22	100%
449	Marbles Pumice Sand	1.22	36.4	1.29	95%
450	Marbles Pumice Sand	1.25	30.7	1.29	97%
451	Marbles Pumice Sand	1.27	30.4	1.29	98%
452	Marbles Pumice Sand	1.41	16.3	1.29	109%
453	Marbles Pumice Sand	1.35	16.1	1.29	105%
454	Marbles Pumice Sand	1.28	26.4	1.29	99%

Test Number	Material	Oven adjusted dry density ¹ (t/m ³)	Oven corrected moisture content (%)	Target Maximum dry density (t/m ³) ⁴	Compaction ² (%)
455	Marbles Pumice Sand	1.27	26.7	1.29	98%
456	Marbles Pumice Sand	1.32	23.7	1.29	102%
458	Marbles Pumice Sand	1.3	22.9	1.29	101%
459	Marbles Pumice Sand	1.28	24.8	1.29	99%
460	Marbles Pumice Sand	1.28	26.2	1.29	99%

1) Dry density measured in the field by Nuclear Density Meter (NDM) and adjusted following laboratory measurement of water content.

2) Ratio of oven adjusted dry density to maximum dry density expressed as a percentage.

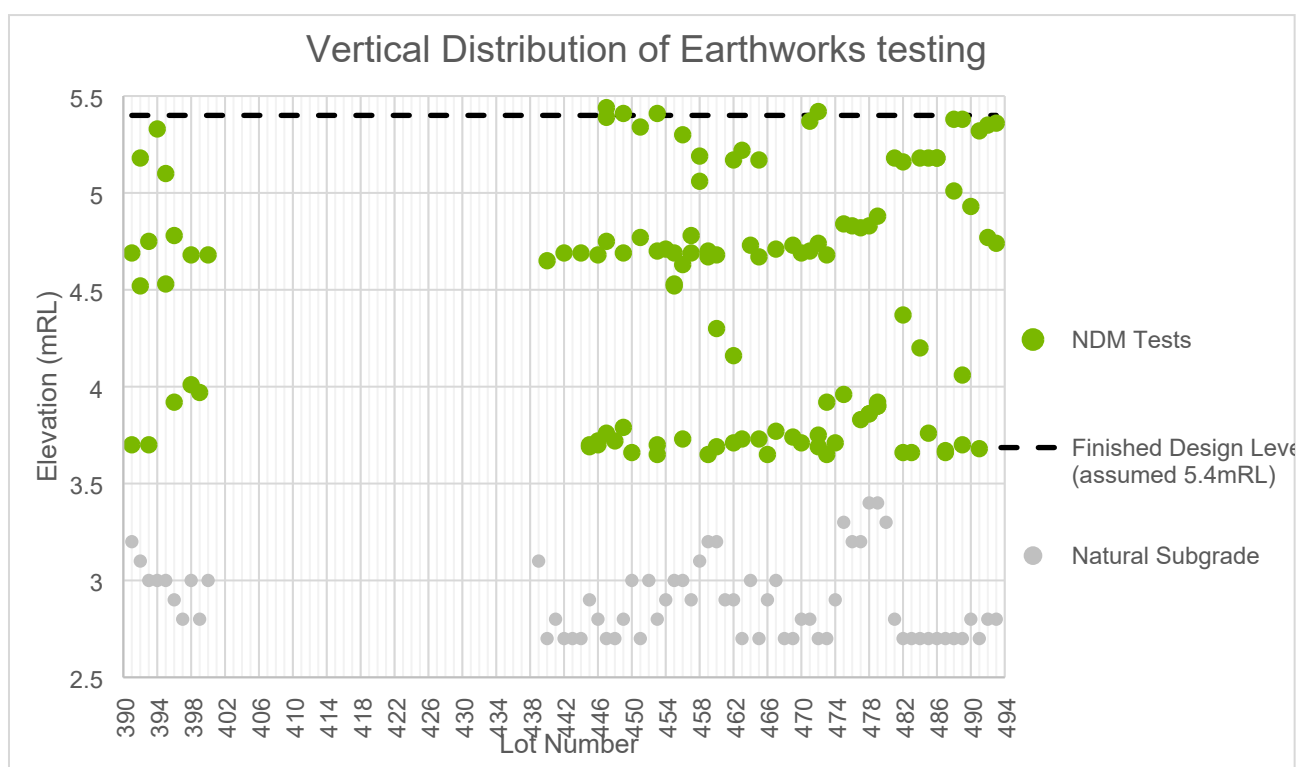
3) Test results denoted with RT indicate a re-test of the original compliance test where it did not originally pass.

4) Targets were adjusted based on three plateau tests conducted on fill material from Paengaroa Quarry, Marbles Quarry, and reworked Paengaroa fill material. Targets are adjusted with a weighted average when material was blended.

The NDM test sheets shown in Appendix C list the target maximum dry density prior to the Plateau tests that were undertaken. These targets were adjusted to the Plateau test targets by Aurecon given the high sensitivity of the material to compaction. Note we have not included the DCP results in Table 2 above however we confirm that all DCP compliance tests met or exceeded the specification requirement of minimum 5 blows per 100mm.

Figure 1 shows the vertical distribution of Nuclear Density testing that was done across the lots. The finished level and subgrade elevations were taken as the approximate value within the centre of the lot. Site won sand was mined and reused to an elevation of at least ~ 2.7 m R.L. Based on the reported results we confirm that all of the compliance testing (NDM) met or exceeded the minimum acceptance criteria for the fill compaction as outlined in the technical earthworks specification.

Figure 1: Vertical distribution of Nuclear Density Testing conducted over the 64 lots.



4.4 Ground conditions

The geological model for the site was developed from Cone Penetration Tests (CPT) and test pitting carried out during the investigations for the Suitability for Subdivision report dated 15 March 2016.

The central half of 3D originally comprised elevated dune ridges (above +4.0mRL) comprising dune sands. Lower lying areas (under +4.0mRL) within the eastern and western part of Stage 3D and all of stage 3E was originally capped by a layer of peat ranging in thickness between 1.2m and 2.0m. It was inferred from the results of the predevelopment CPT that the peat is underlain by sand which are typically in the order of 13m to 15m thick but locally up to 19m in the elevated areas.

The dune sands were broadly separated into an upper 4m to 5m thick layer of loose to medium dense sand, underlain by medium dense to dense sands. The dune sands are underlain by a variable sequence of alluvial deposits comprising interbedded firm to stiff sandy silts and loose to medium dense sands, and dense sands down to the maximum investigation depth (20m below the original ground surface). The groundwater elevation was measured to be approximately +2.7mRL.

Bulk earthworks and movement of the motor scrapers across all areas of the Site are expected to have resulted in the densification of the upper 2 to 3m of the site surface, as evidenced by the subsequent lot by lot verification testing. Finished platform ground levels for the subject lots range between +4.9mRL and +5.6mRL.

4.5 Liquefaction

A liquefaction assessment using parameters derived from NZS 1170.0 and NZS1170.5 was carried out based on the method of Tokimatsu and Seed was included in the Suitability for Subdivision report dated 29 August 2011. The reported analysis predicted that, during a ULS earthquake event, up to 200 mm of settlement could occur. Updated liquefaction assessment guidelines are now available using the methods outlined in Module 3 of MBIE's *Earthquake Geotechnical Engineering Practice Guidelines*¹.

The post-construction liquefaction risk has been re-evaluated using updated CPT data from Stage 3C and following analytical methods prescribed within the Module 3 of the MBIE guidelines. The method of Boulanger and Idriss (2014) was used in our updated analysis of the Site's liquefaction potential.

The results of this additional analysis generally agree with the earlier liquefaction analysis findings, with no settlement predicted in a SLS design event, and moderate levels (94 mm to 150mm) of liquefaction induced settlement was predicted to occur following an intermediate level 1/200 year earthquake event, and more significant settlement in a ULS design event (up to 205mm). In accordance with Table 5.1 of the MBIE Module 3 guideline, the site effects from excess pore water pressure and liquefaction under intermediate case is assessed to be L1 to L2 (Mild to Moderate), and under ULS to be L3 (High). Foundation recommendations with regards to liquefaction mitigation are discussed in Section 5.4.1.

5 Requirements for building development

5.1 General

64 residential lots have been considered within Stage 3D and 3E, which comprise Lots 391 to 400 (inclusive) and 439 to 493 (inclusive).

This section of the report deals with:

- a) The suitability of the created residential lots for building complying with the characteristics of NZS 3604:2011: Timber Framed Buildings
- b) Specific requirements for residential construction on the lots

¹ New Zealand Geotechnical Society (NZGS) and Ministry of Business Innovation & Employment (MBIE) guidelines for Earthquake Geotechnical Practice in New Zealand; Module 3: Identification, assessment and mitigation of liquefaction hazards, Revision 0, May 2016.

c) General recommendations for the building development on the lots

d) Advisory notes to owners and lot developers

The specific requirements should be read in conjunction with the general recommendations for building and the advisory notes contained in this section of the report.

5.2 Lot by lot investigations

5.2.1 Testing

Each residential lot has been individually investigated by one hand auger borehole completed within the centre of the lot, which extended to 2.0 m below the finished ground surface. In addition, two dynamic cone penetrometer (DCP) tests were conducted within each lot to a maximum depth of 1.95m or prior refusal (>20 blows per 100mm) to verify compaction compliance and foundation requirements. Lot by lot DCP testing was completed by Aurecon on 19 December 2018 and 18 January 2019 within Lots 391, 445-447 (inclusive), 458, 459, and 472-474 (inclusive). Testing within the remaining lots and hand augers were completed by Geotechnics Ltd between 10 December and 11 January 2018. The locations of the hand auger/DCP tests are shown on Drawing 247593-0320-DRG-GG-0003, Appendix B. Geotechnical logs for the hand auger boreholes and DCP tests are included in Appendix D.

5.2.2 Topsoil depth

Hand auger testing indicates that the depth of topsoil generally ranges from 0mm to 500 mm from finished ground surface level at the specific locations investigated. The depth of topsoil is shown on the individual hand auger logs included in Appendix D. Refer to Section 5.6.1 for further recommendations regarding topsoil thickness. Compacted sand fill and site-won dune sands were encountered below the topsoil in some of the locations investigated, however most of the investigations revealed over 2.0 m of engineered fill.

5.2.3 Test results

The DCP test results report values of between 5 and 30 blows per 100 mm across Stage 3D and 3E. DCP test results indicated generally dense engineered sand fill in the upper 1 m, with between 10 and 15 blows per 100mm and 6 and 30 blows per 100 mm below 1m to the maximum depth tested.

5.3 Suitability of lots for building

Based on the post earthworks verification testing, all lots (comprise Lots 391 to 400 (inclusive) and 439 to 493 (inclusive)) within Stage 3D and 3E are considered suitable on which to construct residential dwellings that comply with the characteristics defined by NZS3604:2011 *Timber framed buildings*. There are specific requirements that relate to the lots as given in Section 5.4, and summarised in the lot summary sheets included in Appendix A. The suitability of lots for building is contingent on the specific requirements being included in the planning, design and construction of dwellings and any other structure requiring a building consent under the Building Act 2004.

5.4 Specific requirements for building

The following paragraphs outline the specific requirements for building on the subject lots within Stage 3D and 3E of the subdivision. The reader is also referred to the General Recommendations (Section 5.5) and the Advisory Notes (Section 5.6) when assessing the implications of these requirements.

5.4.1 Building foundations - Lots 391 to 400 (inclusive) and 445 to 493 (inclusive)

MBIE Guidelines 'TC2' building foundations

The subject lots are founded on engineered fill, natural ground, or a combination of both. Due to the liquefaction induced settlements predicted to occur within these lots, it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011.

In accordance with Table 5.1 within Module 3 of the MBIE guidelines the site effects from excess pore water pressure and liquefaction is assessed to be 'L3 - High' under ULS conditions. The ULS liquefaction induced settlements are predicted to range up to 205mm.

In accordance with Module 4 of the MBIE guidelines², for cases where the predicted effects are deemed to be intolerable (L3 or higher), it is necessary to consider options for either ground improvement or the use of robust raft foundations, or deep foundations. We consider that the most practicable solution for this site is to specify robust raft foundations.

We have assessed the type of raft foundation system using the general guidance provided in the MBIE guidelines for repairing and rebuilding houses affected by the Canterbury earthquakes³. In consideration of the predicted liquefaction settlement and thickness of the non-liquefiable crust, we classify the site as Technical Category 2 (TC2) in accordance with this guideline.

Specific engineered design of a MBIE 'TC2' foundation is required for all residential buildings constructed on the subject lots with Stage 3D and 3E. Section 5.5.1 provides further discussions of TC2 foundation options/types.

Foundation bearing capacity

An ultimate rupture bearing capacity of 300kPa may be used for the design of foundations for these lots within Stage 3D and 3E. The above ultimate bearing capacities should be used in conjunction with the following reduction factors:

- SLS loading: 0.33
- ULS loading: 0.5
- Earthquake Loading: 0.7

Foundation requirements and inspections

Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents.

As the site is not considered "good ground" as specified under NZS3604, the foundation formation is to be inspected by a suitably qualified and experienced geotechnical engineer prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building.

5.4.2 Building foundations – Lots 439 to 444 (inclusive)

Specific Investigation and Design

The subject lots are founded on engineered fill, cut down natural ground, or a combination of both. It is our opinion that these lots are suitable for residential building development. Due to the presence of sloping ground

² New Zealand Geotechnical Society (NZGS) and Ministry of Business Innovation & Employment (MBIE), Earthquake geotechnical engineering practice; Module 4; Earthquake resistant foundation design; Revision 0 November 2016

³ MBIE, Repairing and rebuilding houses affected by the Canterbury earthquakes; Version 3, December 2012

specific investigation and design of foundations is recommended for these lots by a TCC Category 1 Geo-Professional to support a future building consent application.

We make the following general recommendations to inform the specific investigation and design:

- Due to the liquefaction induced settlements predicted to occur within these lots, it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011.
- The specific foundation should consider Table 5.1 within Module 3 of the MBIE guidelines for site effects from excess pore water pressure and liquefaction.
- The specific foundation should be assessed against general guidance provided in the MBIE guidelines for repairing and rebuilding houses affected by the Canterbury earthquakes⁴.
- Shallow or deep foundations should consider the finished landform and implications of the proposed building development in context to the proximity to the western boundary slope and the associated risk of instability under both static and seismic conditions.

5.4.3 Building Restriction Lines (Lots 475 – 479 inclusive, and 481- 492 inclusive)

Aurecon has reviewed the as-built landform as part of our assessment to confirm the final positions of building restriction lines (BRLs) within the development area related to setbacks from slope batters which vary in height between 1.5 to 2.0 m and a ~1V:3H slope.

BRLs mitigate building foundations against slope instability risk and the areas between lot boundaries and BRLs are defined as 'building exclusion zones'. These zones have been identified as areas of potential instability, or areas where the required factor of safety against instability is less than that normally applied to residential subdivision. Land contained by the slope BRL which define the building exclusion zone may experience instability and slips over the lifetime of the development.

Any structure requiring a building consent as defined by the Building Act 2004,²³ any landscaping fills, retaining structures of any proportion, and any excavations that alter the topography are not to extend over the BRL into the 'building exclusion zone' without specific investigation, assessment and design by a Tauranga City Council Category 1 Geo-professional.

5.4.4 Minimum building platform and finished floor levels (all lots)

The minimum building platform level for any structure requiring a building consent in accordance with RC26514 is set at +5.1mRL (Moturiki Datum). The finished ground level (which equates to the building platform level) on all subject lots varies between +5.10mRL and +6.0 mRL based on the as-built finished landform survey provided by Lysaght.

Where topsoil is encountered during construction, undercut and replacement with engineered fill will be required to achieve the minimum finished floor level. Please refer to Section 6.2 for details on floor levels required to mitigate flood hazard.

5.4.5 On-site disposal of stormwater

The stormwater collected on all lots (excluding Lots 475-479) is to be disposed of via in-ground soakage. Lots 475 to 479 inclusive will have stormwater reticulated via a TCC pipe network.

The ground comprises predominantly pumice sand, fine to medium grained dune site-won sand fill, and natural cut sands. Stormwater disposed of via in-ground soakage will be reticulated within the natural dune sand material. Soakage testing was carried out by Aurecon for Stage 3D and 3E to determine the permeability of the ground to derive recommended design soakage rates.

⁴ MBIE, Repairing and rebuilding houses affected by the Canterbury earthquakes; Version 3, December 2012

The soakage test assessment report prepared by Aurecon is included with the Civil Asset Completion Report prepared by Lysaghts. Specific requirements for the design and construction of the soakage system are provided by Lysaght in the Terrace Views Subdivision Stage 3D-3E Civil Asset Completion Report.

The distance of foundation construction to the soak hole rings or ATLANTIS® FLO-TANK® crate system is dependent on the depth of the soak holes. The following formula should be used to determine the building foundation offset distance:

Offset distance = $D \div \sqrt{3}$, where D is the depth to base of the soakage measured from ground level.

5.4.6 Effluent disposal

Effluent disposal from all lots is to be piped via the established reticulation system provided at the boundary of each lot. On-site disposal of effluent is not permitted.

5.5 General recommendations for building

These recommendations are given so that owners and lot developers apply good construction practice to lot development to avoid costly and time-consuming delays. There may be other practices that equally achieve good outcomes and therefore these recommendations are not conditions placed on lots but are given as general advice.

5.5.1 TC2 foundation types

Foundation systems compliant with the TC2 Category of the MBIE guidelines are available in several types and options. Types include shallow pile and piles with perimeter footings (Types A and B) and concrete slab on grade (Type C). Based on current construction practice in the Papamoa area, Type C foundations are considered likely to be used in new residential construction. A Type C, Option 4 design (Waffle slab over existing soil, Section 5.3.1 of the MBIE guidelines) is a commonly adopted TC2 solution for foundation design in Papamoa and is considered to be an appropriate option for the Site.

5.5.2 Placement of fill inside perimeter foundations

Where additional fill is placed inside the perimeter foundation walls to support a ground-bearing slab, it should be limited to 1.2 m deep including the slab thickness and be made of imported granular fill placed in accordance with NZS4431:1989 and the Tauranga City Council Infrastructure Development Code. Where infill beneath building foundations is greater than 0.6m deep it is recommended that a TCC Category 2 Geo-Professional be engaged by the lot developer to oversee and certify the earthworks. It is recommended that fill be placed to achieve DCP test results of at least 5 blows per 100mm.

5.6 Advisory notes

5.6.1 Topsoil thickness

The thickness of topsoil has been determined from hand auger logs in the lot by lot testing. The placement of topsoil by motor scraper does result in varying thicknesses across the subdivision. The thickness of topsoil is 0 to 0.5m based upon the results of the lot by lot testing. The results of the lot by lot testing are found in the hand auger borehole logs included in Appendix D.

Problems may arise when foundations are excavated to find that they need to be deepened because the thickness of topsoil is greater than 0.3m. This can be largely overcome at the time of planning the building development, when local spade excavations are conducted on the lines of the foundations to determine: the depth of topsoil, the amount of under slab fill to make up for topsoil removal, and any requirement to deepen foundations or raise the site to the underside of the foundation with engineering fill. No guarantee is given or implied that the topsoil depth on any lot created by this subdivision is between 0 and 0.5m.

5.6.2 Natural soil layering

Excavations for building platforms and/or foundations may intersect soil layers of loose sands with greater compressibility. The quality, strength and density of naturally occurring sands varies across the Site and where loose sand is encountered in excavations then foundation deepening, broadening or both may be necessary.

6 Natural hazard completion report

6.1 General

All of the subject units are suitable for building development with respect to natural hazards without the need to issue building consents pursuant to s72 of the Building Act. Please refer to Table 3 for a summary review of natural hazards, and Section 6.2 for further discussion of flood hazard.

Table 3: Natural hazards

Natural hazard	Applicable to Site?	Mitigation (if required)
Land below Tauranga City Council's minimum building platform level	N	The minimum building platform level set by Tauranga City Council for the subject lots is +5.1mRL (Moturiki Datum). Final landform of the lots consented under RC26514 is between +5.10mRL and +6.0mRL.
Specific design for stormwater disposal	Y	Stormwater is disposed of via on-site soakage pits or other approved options. Soakage disposal shall be in accordance with the recommendations of the Civil Asset Completion Report.
Coastal flooding	N	–
Coastal erosion	N	–
Harbour inundation	N	–
Flood prone land	N	The subdivision is not located in an area considered to be of flood risk according to the Tauranga City Council online flood maps (accessed 27/09/2016). Refer Section 6.2 for more details on mitigation of flood hazard.
Stormwater sensitive	N	–
Overland flows	N	–
Slope instability	Y	Building Restriction Lines have been designated on all lots with slope instability risk. The building restriction lines are based upon slope stability assessment models. Lots requiring building on sloping ground have been designated Specific Investigation and Design.
Differential Settlement	N	Non-liquefaction related differential settlement is not predicted to occur within the subdivision.
Liquefaction of submerged sand layer	Y	The probability of damage to residential NZS3604 type buildings is considered to be low during Serviceability Limit State conditions. There is a risk of liquefaction induced ground damage as a result of an Ultimate Limit State event with up to approximately 205mm of settlement predicted to occur. Lateral spread is considered to be unlikely. The effects of liquefaction are expected to be mitigated by construction of a TC2 foundation system as described in Section 5.4.
Seismic shaking	Y	The effect to residential NZS3604 type buildings from seismic shaking can be influenced by sound construction methods and materials composition (lightweight and ductile roofing and cladding). Shaking hazards are enhanced by the use of construction products such as masonry cladding and concrete/tile roofing.

6.2 Flood hazard

The mitigation of flood hazard to buildings is managed through addressing four key criteria, as summarised in Table 4.

Table 4 Summary of flood hazard criteria

Criteria	Reference	Discussion
Lot levels	Tauranga City Council Resource Consent RC26514	Condition 17 of Resource Consent #26514 states that: <i>“all residential lots have a minimum building platform level of RL+5.1 m Moturiki Datum and that is 500 millimetres above a maximum ponding level of a 100 year (1% AEP) rainfall event as calculated by a Chartered Professional Engineer”.</i> The as-built finished ground level plans show that final constructed lot levels range between +5.10mRL and +6.0mRL. A detailed freeboard assessment completed by Lysaghts Consultants Limited (LCL) confirms that the formation level exceeds RL +5.1, which is 500 mm above a 100-year rainfall event. Site grading activities must not: ■ Lower the elevation of residential lot levels below that reported in Section 5.4.4. ■ Create areas of localised ponding ■ Direct stormwater run-off to adjacent residential properties or buildings For further information, please refer to LCL Stage 3D-E Development Completion Report (Revision 0, Reference 152870, 01 February 2019)
Localised Ponding	New Zealand Building Code (NZBC), Clause E1 (Stormwater) Tauranga City Council Infrastructure Development Code, Design Standard DS-5 (Stormwater)	The lots covered by this report are not subject to localised ponding. A detailed freeboard assessment completed by Lysaghts Consultants Limited (LCL) confirms that the formation level exceeds RL +5.10, which is 500 mm above a 100 year rainfall event. The as-built finished ground level plans show that final constructed lot levels range between +5.10mRL and +6.0mRL. The LCL report confirms that finished lot levels achieve a minimum freeboard in excess of 500 mm where overland flows are deeper than 100 mm. Site grading activities must not: ■ Lower the elevation of residential lot levels below that reported in Section 5.4.4. ■ Create areas of localised ponding ■ Direct stormwater run-off to adjacent residential properties or buildings For further information, please refer to LCL Stage 3D-E Development Completion Report (Revision 0, Reference 152870, 01 February 2019)
Site Drainage	New Zealand Building Code (NZBC), Clause E1 (Surface water)	The site surface shall be graded such that all excess surface water run-off is diverted away from adjacent residential properties or building structures. Clause E1/AS1 of the building code establishes the minimum acceptable floor level above any stormwater associated with site drainage. Suspended floors and slabs on ground shall be at least 150 mm above the finished level of the surrounding ground, and: ■ For finished lots level with or above the level of the adjacent road or accessway, the floor level shall be <i>“no less than 150 mm above the road crown on at least one section through the building”</i>. ■ For finished lots below the road, the floor level shall be <i>“no less than 150 mm above the lowest point on the site boundary”</i>.
Ground Clearance	NZS3604:2011	The finished building design within each of the shall meet the requirements of NZS3604 and the building code.

7 Limitations

All assessments made in this report are based on the ground conditions indicated from published sources, site inspection and hand auger boreholes. Variations in ground conditions may occur between the investigation locations. In addition, there may be special conditions pertaining to the Site which have not been revealed by the investigation and which have not therefore been taken into account in the report. No warranty (either expressed or implied) that the actual conditions will conform exactly to the assessments contained in this report, is included.

This report does not remove the responsibility of the Owner / Builder / Building Certifier to satisfy themselves of foundation depth and suitability at the finally selected house location and to ensure foundation conditions are not allowed to deteriorate due to saturation or other causes.

Where data supplied by the developer or other external sources, including previous site inspection data, has been used, it has been assumed that the information is correct unless otherwise stated. No responsibility can be accepted by Aurecon Ltd for inaccuracies within the data supplied by others.

Any comments on groundwater conditions are based on observations made at the time of investigation unless otherwise stated. It should be noted that groundwater levels vary as a result of seasonal or other effects.

This report has been prepared to be read in full, including all appendices. No responsibility can be accepted by Aurecon Ltd for parties' relying on partial information from an incomplete report

This report is provided for sole use by the client and subsequent lot owners. It shall not be used, reproduced or interpreted for any purpose other than is specifically set out in the text of this report.

8 References

220166 – Geotechnical Suitability Report, Papamoa East Block, Revision 0 dated 09 September 2011, Aurecon.

247593 - Geotechnical review of landform design and suitability for residential subdivision, Revision 1, dated 29th January 2016

247593 - Geotechnical suitability for residential subdivision of Stages 1D, 3A and 3B of the Parton Road Subdivision, Papamoa, dated 15 March 2016

MBIE, Repairing and rebuilding houses affected by the Canterbury earthquakes; Version 3, December 2012

NZGS, 2005. Guidelines for the Classification and Field Description of Soils and Rocks in Engineering. NZ Geotechnical Society Inc., Wellington, New Zealand

Department of Building and Housing, 2011. Compliance Document for New Zealand Building Code, Clause B1, Structures.

NZS3604:2011. *New Zealand Standard, Timber framed buildings*. Standards New Zealand, Wellington, New Zealand

Tauranga City Council 2017 Infrastructure Development Code, *Design Standard DS-10 Natural Hazards and Earthworks*



A

Statement of Professional
Opinion and Lot Summary
Sheets

Appendix A

Statement of Professional Opinion and Lot Summary Sheets

Statement of professional opinion as to the geotechnical suitability of the land for building

Development: Terrace Views Subdivision, Parton Road, Papamoa
Engineer Responsible for investigation: Richard Griffiths
Qualifications: BSc(Hons); CGeol; TCC Category 1 Geo-Professional

I, *Richard Griffiths*, an employee of Aurecon NZ Limited

Hereby confirm that I am an accredited Category 1 Engineering Geologist appropriately qualified with experience in geotechnical engineering to ascertain the suitability of the land for building development and was retained as the soils engineer to the above development.

- 1) An appropriate level of site investigation and construction supervision has been carried out under my direction and as described in our Geotechnical Completion Report reference 247593-Stage 3D-E-GCR Rev 3 dated 8 March 2019.
- 2) In my professional opinion, not to be construed as a guarantee of certainty or completeness, I consider that;
 - i. All residential subject lots within Stage 3D and E of the development (Lots 391 to 400(inclusive), Lot 439 to 492 (inclusive) are suitable on which to erect dwellings that comply with NZS 3604:2011 Timber Framed Buildings subject to the specific requirements of the Geotechnical Completion Report reference reference 247593-Stage 3D-E-GCR Rev 3 dated 8 March 2019 being met.
 - ii. Each lot is suitable for building development without the need to issue a building consent pursuant to s72 of the Building Act.

This opinion is given on the condition that:

- iii. The content of the Geotechnical Completion Report reference reference 247593-Stage 3D-E-GCR Rev 3 dated 8 March 2019 is made available to each lot owner and that the specific requirements for building development set out in Section 5.4 of this report as they apply to each lot are followed.
 - iv. The general recommendations and advisory notes (Sections 5.5 and 5.6) should be read and understood by lot owners and builders prior to building development.
- 3) This professional opinion is furnished to Tauranga City Council and Zariba Properties Limited, the developer, for their purpose alone, on the express condition that it will not be relied upon by any other person and does not relieve lot owners, their advisers, or the Territorial Authority from their respective responsibilities and does not remove the necessity for normal inspection of foundation conditions at the time of erection for any dwelling.

Signed: for and on behalf of Aurecon NZ Limited



Richard Griffiths
Tauranga City Council Category 1 Geo-Professional

Date
08/03/19

Summary of Geotechnical Data/Recommendations/Requirements for Individual Lots

The comments and notations included on this summary sheet are outlined in the support documents. These shall be read in conjunction with this summary.

Subdivision: Terrace Views Subdivision Stage 3D – 3E

Lot No.	Subsurface Data							Building Restriction Line	Recommendations and Restrictions
	DCP Blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
391	15 (0.2)	Y	2.0	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

Summary of Geotechnical Data/Recommendations/Requirements for Individual Lots

The comments and notations included on this summary sheet are outlined in the support documents. These shall be read in conjunction with this summary.

Subdivision: Terrace Views Subdivision Stage 3D – 3E

Lot No.	Subsurface Data							Building Restriction Line	Recommendations and Restrictions
	DCP Blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
392	16 (0.2)	Y	2.5	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

Summary of Geotechnical Data/Recommendations/Requirements for Individual Lots

The comments and notations included on this summary sheet are outlined in the support documents. These shall be read in conjunction with this summary.

Subdivision: Terrace Views Subdivision Stage 3D – 3E

Lot No.	Subsurface Data							Building Restriction Line	Recommendations and Restrictions
	DCP Blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
393	12 (0.2)	Y	2.5	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

Summary of Geotechnical Data/Recommendations/Requirements for Individual Lots

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Subdivision: Terrace Views Subdivision Stage 3D – 3E

Lot No.	Subsurface Data							Building Restriction Line	Recommendations and Restrictions
	DCP Blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
394	11 (0.3)	Y	2.0	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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	DCP Blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
395	12 (0.3)	Y	2.5	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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	DCP Blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
396	8 (0.05)	Y	2.5	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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Lot No.	Subsurface Data							Building Restriction Line	Recommendations and Restrictions
	DCP Blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
397	10 (0.2)	Y	3.0	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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	DCP Blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
398	13 (0.3)	Y	3.0	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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	DCP Blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
399	14 (0.3)	Y	3.0	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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Lot No.	Subsurface Data							Building Restriction Line	Recommendations and Restrictions
	DCP Blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
400	5 (0.45)	Y	3.0	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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	DCP Blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
439	14 (0.4)	Y	3.0	N	N	N/A	Y	N	Specific Investigation and Design This lot is founded on engineered fill, cut down natural ground, or a combination of both. It is our opinion that these lots are suitable for residential building development. Due to the presence of sloping ground specific investigation and design of foundations is recommended for these lots by a TCC Category 1 Geo-Professional to support a future building consent application. We make the following general recommendations to inform the specific investigation and design: ■ Due to the liquefaction induced settlements predicted to occur within these lots, it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. ■ The specific foundation should consider Table 5.1 within Module 3 of the MBIE guidelines for site effects from excess pore water pressure and liquefaction. ■ The specific foundation should be assessed against general guidance provided in the MBIE guidelines for repairing and rebuilding houses affected by the Canterbury earthquakes¹. ■ Shallow or deep foundations should consider the finished landform and position to the western boundary. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

¹ MBIE, Repairing and rebuilding houses affected by the Canterbury earthquakes; Version 3, December 2012

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
440	12 (0.5)	Y	3.0	N	N	N/A	Y	N	Specific Investigation and Design This lot is founded on engineered fill, cut down natural ground, or a combination of both. It is our opinion that these lots are suitable for residential building development. Due to the presence of sloping ground specific investigation and design of foundations is recommended for these lots by a TCC Category 1 Geo-Professional to support a future building consent application. We make the following general recommendations to inform the specific investigation and design: ■ Due to the liquefaction induced settlements predicted to occur within these lots, it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. ■ The specific foundation should consider Table 5.1 within Module 3 of the MBIE guidelines for site effects from excess pore water pressure and liquefaction. ■ The specific foundation should be assessed against general guidance provided in the MBIE guidelines for repairing and rebuilding houses affected by the Canterbury earthquakes². ■ Shallow or deep foundations should consider the finished landform and position to the western boundary. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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441	14 (0.5)	Y	3.0	N	N	N/A	Y	N	Specific Investigation and Design This lot is founded on engineered fill, cut down natural ground, or a combination of both. It is our opinion that these lots are suitable for residential building development. Due to the presence of sloping ground specific investigation and design of foundations is recommended for these lots by a TCC Category 1 Geo-Professional to support a future building consent application. We make the following general recommendations to inform the specific investigation and design: ■ Due to the liquefaction induced settlements predicted to occur within these lots, it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. ■ The specific foundation should consider Table 5.1 within Module 3 of the MBIE guidelines for site effects from excess pore water pressure and liquefaction. ■ The specific foundation should be assessed against general guidance provided in the MBIE guidelines for repairing and rebuilding houses affected by the Canterbury earthquakes³. ■ Shallow or deep foundations should consider the finished landform and position to the western boundary. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
442	15 (0.3)	Y	3.0	N	N	N/A	Y	N	Specific Investigation and Design This lot is founded on engineered fill, cut down natural ground, or a combination of both. It is our opinion that these lots are suitable for residential building development. Due to the presence of sloping ground specific investigation and design of foundations is recommended for these lots by a TCC Category 1 Geo-Professional to support a future building consent application. We make the following general recommendations to inform the specific investigation and design: ■ Due to the liquefaction induced settlements predicted to occur within these lots, it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. ■ The specific foundation should consider Table 5.1 within Module 3 of the MBIE guidelines for site effects from excess pore water pressure and liquefaction. ■ The specific foundation should be assessed against general guidance provided in the MBIE guidelines for repairing and rebuilding houses affected by the Canterbury earthquakes⁴. ■ Shallow or deep foundations should consider the finished landform and position to the western boundary. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

⁴ MBIE, Repairing and rebuilding houses affected by the Canterbury earthquakes; Version 3, December 2012

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
443	8 (0.25)	Y	3.0	N	N	N/A	Y	N	Specific Investigation and Design This lot is founded on engineered fill, cut down natural ground, or a combination of both. It is our opinion that these lots are suitable for residential building development. Due to the presence of sloping ground specific investigation and design of foundations is recommended for these lots by a TCC Category 1 Geo-Professional to support a future building consent application. We make the following general recommendations to inform the specific investigation and design: ■ Due to the liquefaction induced settlements predicted to occur within these lots, it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. ■ The specific foundation should consider Table 5.1 within Module 3 of the MBIE guidelines for site effects from excess pore water pressure and liquefaction. ■ The specific foundation should be assessed against general guidance provided in the MBIE guidelines for repairing and rebuilding houses affected by the Canterbury earthquakes⁵. ■ Shallow or deep foundations should consider the finished landform and position to the western boundary. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
444	6 (0.25)	Y	3.0	N	N	N/A	Y	N	Specific Investigation and Design This lot is founded on engineered fill, cut down natural ground, or a combination of both. It is our opinion that these lots are suitable for residential building development. Due to the presence of sloping ground specific investigation and design of foundations is recommended for these lots by a TCC Category 1 Geo-Professional to support a future building consent application. We make the following general recommendations to inform the specific investigation and design: ■ Due to the liquefaction induced settlements predicted to occur within these lots, it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. ■ The specific foundation should consider Table 5.1 within Module 3 of the MBIE guidelines for site effects from excess pore water pressure and liquefaction. ■ The specific foundation should be assessed against general guidance provided in the MBIE guidelines for repairing and rebuilding houses affected by the Canterbury earthquakes⁶. ■ Shallow or deep foundations should consider the finished landform and position to the western boundary. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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Subdivision: Terrace Views Subdivision Stage 3D – 3E

Lot No.	Subsurface Data							Building Restriction Line	Recommendations and Restrictions
	DCP blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
445	6 (0.3)	Y	3.0	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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Subdivision: Terrace Views Subdivision Stage 3D – 3E

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	DCP blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
446	10 (0.35)	Y	3.0	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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	DCP blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
447	10 (0.3)	Y	2.5	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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	DCP blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
448	9 (0.4)	Y	2.5	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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	DCP blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
449	10 (0.5)	Y	2.5	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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	DCP blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
450	15 (0.35)	Y	2.5	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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	DCP blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
451	6 (0)	Y	2.5	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
452	6 (0)	Y	2.5	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
453	5 (0)	Y	3.0	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
454	10 (0)	Y	3.0	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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455	5 (0)	Y	3.0	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
456	8 (0)	Y	2.5	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
457	9 (0)	Y	1.5	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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	DCP blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
458	5 (0.3)	Y	2.5	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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Subdivision: Terrace Views Subdivision Stage 3D – 3E

Lot No.	Subsurface Data							Building Restriction Line	Recommendations and Restrictions
	DCP blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
459	6 (0.3)	Y	3.0	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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	DCP blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
460	6 (0.3)	Y	0.5	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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	DCP blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
461	9 (0.25)	Y	3.0	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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	DCP blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
462	10 (0.4)	Y	3.0	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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	DCP blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
463	6 (0.35)	Y	2.5	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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	DCP blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
464	9 (0.35)	Y	1.5	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
465	9 (0.4)	Y	1.5	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
466	7 (0.3)	Y	2.5	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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467	6 (0.3)	Y	3.0	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
468	8 (0.35)	Y	3.0	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
469	6 (0.45)	Y	3.0	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
471	7 (0.3)	Y	3.0	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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Subdivision: Terrace Views Subdivision Stage 3D – 3E

Lot No.	Subsurface Data							Building Restriction Line	Recommendations and Restrictions
	DCP blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
472	10 (0.35)	Y	3.0	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
473	11 (0.3)	Y	3.5	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
474	11 (0.35)	Y	2.5	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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475	7 (0.3)	Y	3.0	N	N	N/A	N	Y	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level of +5.1 mRL. This lot was consented prior to the Wairakei residential designation and updated floor levels. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Building Restriction Line (BRL) This lot is situated above sloping ground. The area contained by the site boundary and the nominated BRL is defined as a 'building exclusion zone'. This zone has been identified as an area of potential instability, or an area where the required factor of safety against instability is less than that normally applied to residential subdivision. Land contained by the slope BRL which define the building exclusion zone may experience instability and slips over the lifetime of the development. Any structure requiring a building consent as defined by the Building Act 2004, any landscaping fills, retaining structures of any proportion and excavation to alter the slope of the topography are not to extend over the BRL into the 'building exclusion zone' without specific investigation, assessment and design by a Tauranga City Council Category 1 Geo-professional. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via the Tauranga City Council stormwater pipe network. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
476	5 (0.3)	Y	3.5	N	N	N/A	N	Y	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level of +5.1 mRL. This lot was consented prior to the Wairakei residential designation and updated floor levels. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Building Restriction Line (BRL) This lot is situated above sloping ground. The area contained by the site boundary and the nominated BRL is defined as a 'building exclusion zone'. This zone has been identified as an area of potential instability, or an area where the required factor of safety against instability is less than that normally applied to residential subdivision. Land contained by the slope BRL which define the building exclusion zone may experience instability and slips over the lifetime of the development. Any structure requiring a building consent as defined by the Building Act 2004, any landscaping fills, retaining structures of any proportion and excavation to alter the slope of the topography are not to extend over the BRL into the 'building exclusion zone' without specific investigation, assessment and design by a Tauranga City Council Category 1 Geo-professional. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via the Tauranga City Council stormwater pipe network. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
477	18 (0.3)	Y	3.0	N	N	N/A	N	Y	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level of +5.1 mRL. This lot was consented prior to the Wairakei residential designation and updated floor levels. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Building Restriction Line (BRL) This lot is situated above sloping ground. The area contained by the site boundary and the nominated BRL is defined as a 'building exclusion zone'. This zone has been identified as an area of potential instability, or an area where the required factor of safety against instability is less than that normally applied to residential subdivision. Land contained by the slope BRL which define the building exclusion zone may experience instability and slips over the lifetime of the development. Any structure requiring a building consent as defined by the Building Act 2004, any landscaping fills, retaining structures of any proportion and excavation to alter the slope of the topography are not to extend over the BRL into the 'building exclusion zone' without specific investigation, assessment and design by a Tauranga City Council Category 1 Geo-professional. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via the Tauranga City Council stormwater pipe network. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
478	6 (0.3)	Y	3.0	N	N	N/A	N	Y	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level of +5.1 mRL. This lot was consented prior to the Wairakei residential designation and updated floor levels. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Building Restriction Line (BRL) This lot is situated above sloping ground. The area contained by the site boundary and the nominated BRL is defined as a 'building exclusion zone'. This zone has been identified as an area of potential instability, or an area where the required factor of safety against instability is less than that normally applied to residential subdivision. Land contained by the slope BRL which define the building exclusion zone may experience instability and slips over the lifetime of the development. Any structure requiring a building consent as defined by the Building Act 2004, any landscaping fills, retaining structures of any proportion and excavation to alter the slope of the topography are not to extend over the BRL into the 'building exclusion zone' without specific investigation, assessment and design by a Tauranga City Council Category 1 Geo-professional. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via the Tauranga City Council stormwater pipe network. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
479	11 (0.5)	Y	3.0	N	N	N/A	N	Y	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level of +5.1 mRL. This lot was consented prior to the Wairakei residential designation and updated floor levels. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Building Restriction Line (BRL) This lot is situated above sloping ground. The area contained by the site boundary and the nominated BRL is defined as a 'building exclusion zone'. This zone has been identified as an area of potential instability, or an area where the required factor of safety against instability is less than that normally applied to residential subdivision. Land contained by the slope BRL which define the building exclusion zone may experience instability and slips over the lifetime of the development. Any structure requiring a building consent as defined by the Building Act 2004, any landscaping fills, retaining structures of any proportion and excavation to alter the slope of the topography are not to extend over the BRL into the 'building exclusion zone' without specific investigation, assessment and design by a Tauranga City Council Category 1 Geo-professional. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via the Tauranga City Council stormwater pipe network. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
480	9 (0.3)	Y	2.5	N	N	N/A	N	N	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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481	5 (0.35)	Y	3.0	N	N	N/A	N	Y	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Building Restriction Line (BRL) This lot is situated above sloping ground. The area contained by the site boundary and the nominated BRL is defined as a 'building exclusion zone'. This zone has been identified as an area of potential instability, or an area where the required factor of safety against instability is less than that normally applied to residential subdivision. Land contained by the slope BRL which define the building exclusion zone may experience instability and slips over the lifetime of the development. Any structure requiring a building consent as defined by the Building Act 2004, any landscaping fills, retaining structures of any proportion and excavation to alter the slope of the topography are not to extend over the BRL into the 'building exclusion zone' without specific investigation, assessment and design by a Tauranga City Council Category 1 Geo-professional. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
482	5 (0.2)	Y	3.5	N	N	N/A	N	Y	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Building Restriction Line (BRL) This lot is situated above sloping ground. The area contained by the site boundary and the nominated BRL is defined as a 'building exclusion zone'. This zone has been identified as an area of potential instability, or an area where the required factor of safety against instability is less than that normally applied to residential subdivision. Land contained by the slope BRL which define the building exclusion zone may experience instability and slips over the lifetime of the development. Any structure requiring a building consent as defined by the Building Act 2004, any landscaping fills, retaining structures of any proportion and excavation to alter the slope of the topography are not to extend over the BRL into the 'building exclusion zone' without specific investigation, assessment and design by a Tauranga City Council Category 1 Geo-professional. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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Lot No.	Subsurface Data							Building Restriction Line	Recommendations and Restrictions
	DCP blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
483	5 (0.35)	Y	3.5	N	N	N/A	N	Y	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Building Restriction Line (BRL) This lot is situated above sloping ground. The area contained by the site boundary and the nominated BRL is defined as a 'building exclusion zone'. This zone has been identified as an area of potential instability, or an area where the required factor of safety against instability is less than that normally applied to residential subdivision. Land contained by the slope BRL which define the building exclusion zone may experience instability and slips over the lifetime of the development. Any structure requiring a building consent as defined by the Building Act 2004, any landscaping fills, retaining structures of any proportion and excavation to alter the slope of the topography are not to extend over the BRL into the 'building exclusion zone' without specific investigation, assessment and design by a Tauranga City Council Category 1 Geo-professional. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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Subdivision: Terrace Views Subdivision Stage 3D – 3E

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	DCP blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
484	16 (0.35)	Y	4.0	N	N	N/A	N	Y	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Building Restriction Line (BRL) This lot is situated above sloping ground. The area contained by the site boundary and the nominated BRL is defined as a 'building exclusion zone'. This zone has been identified as an area of potential instability, or an area where the required factor of safety against instability is less than that normally applied to residential subdivision. Land contained by the slope BRL which define the building exclusion zone may experience instability and slips over the lifetime of the development. Any structure requiring a building consent as defined by the Building Act 2004, any landscaping fills, retaining structures of any proportion and excavation to alter the slope of the topography are not to extend over the BRL into the 'building exclusion zone' without specific investigation, assessment and design by a Tauranga City Council Category 1 Geo-professional. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
485	9 (0.3)	Y	4.0	N	N	N/A	N	Y	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Building Restriction Line (BRL) This lot is situated above sloping ground. The area contained by the site boundary and the nominated BRL is defined as a 'building exclusion zone'. This zone has been identified as an area of potential instability, or an area where the required factor of safety against instability is less than that normally applied to residential subdivision. Land contained by the slope BRL which define the building exclusion zone may experience instability and slips over the lifetime of the development. Any structure requiring a building consent as defined by the Building Act 2004, any landscaping fills, retaining structures of any proportion and excavation to alter the slope of the topography are not to extend over the BRL into the 'building exclusion zone' without specific investigation, assessment and design by a Tauranga City Council Category 1 Geo-professional. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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	DCP blows per 100mm (min) (0.3m or below)	Subdivision Filling		Natural Topography Unworked	Natural Topography Cut		Specific Design?		
		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
486	7 (0.25)	Y	4.0	N	N	N/A	N	Y	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Building Restriction Line (BRL) This lot is situated above sloping ground. The area contained by the site boundary and the nominated BRL is defined as a 'building exclusion zone'. This zone has been identified as an area of potential instability, or an area where the required factor of safety against instability is less than that normally applied to residential subdivision. Land contained by the slope BRL which define the building exclusion zone may experience instability and slips over the lifetime of the development. Any structure requiring a building consent as defined by the Building Act 2004, any landscaping fills, retaining structures of any proportion and excavation to alter the slope of the topography are not to extend over the BRL into the 'building exclusion zone' without specific investigation, assessment and design by a Tauranga City Council Category 1 Geo-professional. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
487	9 (0.2)	Y	4.0	N	N	N/A	N	Y	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Building Restriction Line (BRL) This lot is situated above sloping ground. The area contained by the site boundary and the nominated BRL is defined as a 'building exclusion zone'. This zone has been identified as an area of potential instability, or an area where the required factor of safety against instability is less than that normally applied to residential subdivision. Land contained by the slope BRL which define the building exclusion zone may experience instability and slips over the lifetime of the development. Any structure requiring a building consent as defined by the Building Act 2004, any landscaping fills, retaining structures of any proportion and excavation to alter the slope of the topography are not to extend over the BRL into the 'building exclusion zone' without specific investigation, assessment and design by a Tauranga City Council Category 1 Geo-professional. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
488	13 (0.35)	Y	4.0	N	N	N/A	N	Y	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Building Restriction Line (BRL) This lot is situated above sloping ground. The area contained by the site boundary and the nominated BRL is defined as a 'building exclusion zone'. This zone has been identified as an area of potential instability, or an area where the required factor of safety against instability is less than that normally applied to residential subdivision. Land contained by the slope BRL which define the building exclusion zone may experience instability and slips over the lifetime of the development. Any structure requiring a building consent as defined by the Building Act 2004, any landscaping fills, retaining structures of any proportion and excavation to alter the slope of the topography are not to extend over the BRL into the 'building exclusion zone' without specific investigation, assessment and design by a Tauranga City Council Category 1 Geo-professional. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
489	11 (0.3)	Y	3.5	N	N	N/A	N	Y	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Building Restriction Line (BRL) This lot is situated above sloping ground. The area contained by the site boundary and the nominated BRL is defined as a 'building exclusion zone'. This zone has been identified as an area of potential instability, or an area where the required factor of safety against instability is less than that normally applied to residential subdivision. Land contained by the slope BRL which define the building exclusion zone may experience instability and slips over the lifetime of the development. Any structure requiring a building consent as defined by the Building Act 2004, any landscaping fills, retaining structures of any proportion and excavation to alter the slope of the topography are not to extend over the BRL into the 'building exclusion zone' without specific investigation, assessment and design by a Tauranga City Council Category 1 Geo-professional. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
490	12 (0.3)	Y	3.0	N	N	N/A	N	Y	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Building Restriction Line (BRL) This lot is situated above sloping ground. The area contained by the site boundary and the nominated BRL is defined as a 'building exclusion zone'. This zone has been identified as an area of potential instability, or an area where the required factor of safety against instability is less than that normally applied to residential subdivision. Land contained by the slope BRL which define the building exclusion zone may experience instability and slips over the lifetime of the development. Any structure requiring a building consent as defined by the Building Act 2004, any landscaping fills, retaining structures of any proportion and excavation to alter the slope of the topography are not to extend over the BRL into the 'building exclusion zone' without specific investigation, assessment and design by a Tauranga City Council Category 1 Geo-professional. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
491	7 (0.3)	Y	2.5	N	N	N/A	N	Y	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Building Restriction Line (BRL) This lot is situated above sloping ground. The area contained by the site boundary and the nominated BRL is defined as a 'building exclusion zone'. This zone has been identified as an area of potential instability, or an area where the required factor of safety against instability is less than that normally applied to residential subdivision. Land contained by the slope BRL which define the building exclusion zone may experience instability and slips over the lifetime of the development. Any structure requiring a building consent as defined by the Building Act 2004, any landscaping fills, retaining structures of any proportion and excavation to alter the slope of the topography are not to extend over the BRL into the 'building exclusion zone' without specific investigation, assessment and design by a Tauranga City Council Category 1 Geo-professional. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

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		Y/N	Depth (m)	Y/N	Y/N	Depth (m)	Y/N	Y/N	
492	9 (0.4)	Y	2.0	N	N	N/A	N	Y	Building Foundations The subject lot is founded on engineered fill, cut down natural ground, or a combination of both. Due to the liquefaction settlements predicted to occur within this lot it is considered that the soils do not meet the definition of 'good ground' in accordance with NZS3604:2011. A MBIE 'TC2' foundation is required for this lot (refer Section 5.5.1 for further discussion of TC2 foundation options/types). An ultimate rupture bearing capacity of 300 kPa may be used for the design of foundations. This should be used in conjunction with the following reduction factors: ■ SLS loading: 0.33 ■ ULS loading: 0.5 ■ Earthquake Loading: 0.7 Buildings will need to be flexible to withstand up to 25mm of differential settlement over a 6m length, as indicated by Appendix B section B1 of the NZ Building Code Handbook and Approved Documents. The foundation formation is to be inspected by a Territorial Authority accredited building inspector prior to pouring the concrete. Foundations may need to be deepened or broadened or both should low strength soils be encountered during excavation. All topsoil shall be removed from beneath building foundations and floor slabs prior to building. Minimum Building Platform Level Minimum Building Platform Levels for all buildings constructed on these lots are required to achieve the minimum level set by Tauranga City Council of +5.1mRL. Where excessive topsoil thickness is encountered during construction, undercut and replacement with engineered fill may be required to achieve the minimum Building Platform Level. Building Restriction Line (BRL) This lot is situated above sloping ground. The area contained by the site boundary and the nominated BRL is defined as a 'building exclusion zone'. This zone has been identified as an area of potential instability, or an area where the required factor of safety against instability is less than that normally applied to residential subdivision. Land contained by the slope BRL which define the building exclusion zone may experience instability and slips over the lifetime of the development. Any structure requiring a building consent as defined by the Building Act 2004, any landscaping fills, retaining structures of any proportion and excavation to alter the slope of the topography are not to extend over the BRL into the 'building exclusion zone' without specific investigation, assessment and design by a Tauranga City Council Category 1 Geo-professional. Stormwater and effluent disposal Stormwater from this lot is to be disposed of via on site soakage. Specific requirements for construction of stormwater soakholes are addressed in a separate Civil Asset Completion Report. On site effluent disposal is not permitted. All effluent is to be reticulated to public services provided at lot boundaries and eventually to the stormwater pond.

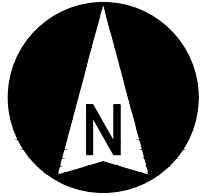
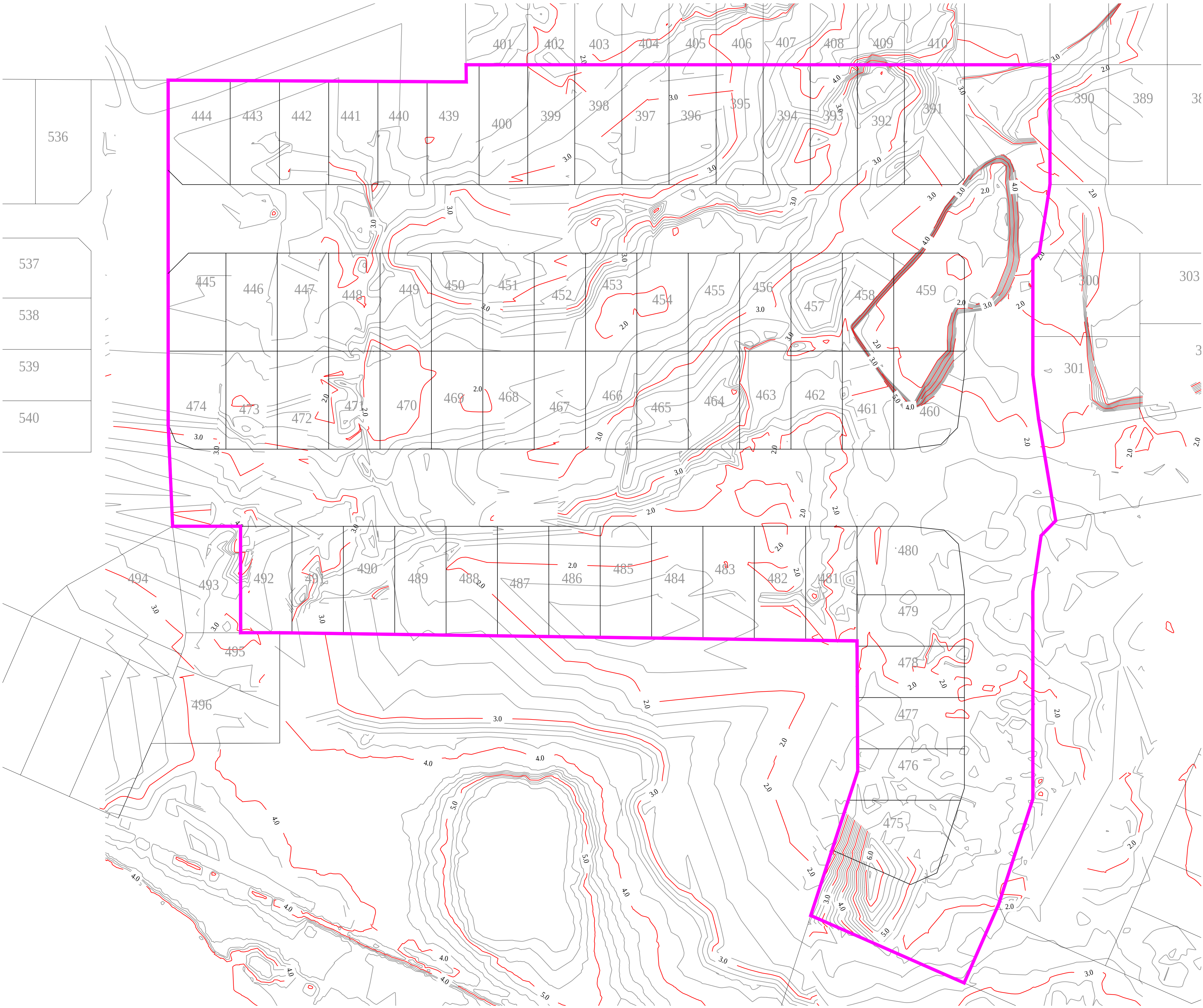
A large green parallelogram is the central element. A diagonal line runs from the bottom-left corner to the top-right corner. A yellow triangle is located at the bottom-left corner, with its hypotenuse along the diagonal line. The rest of the parallelogram is green.

B

Drawings

Appendix B

Drawings



LEGEND	
—	STAGE 3D AND 3E BOUNDARY
—	EXISTING MAJOR CONTOURS
—	NATURAL MINOR CONTOURS

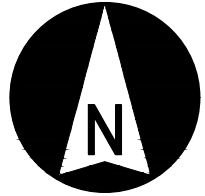
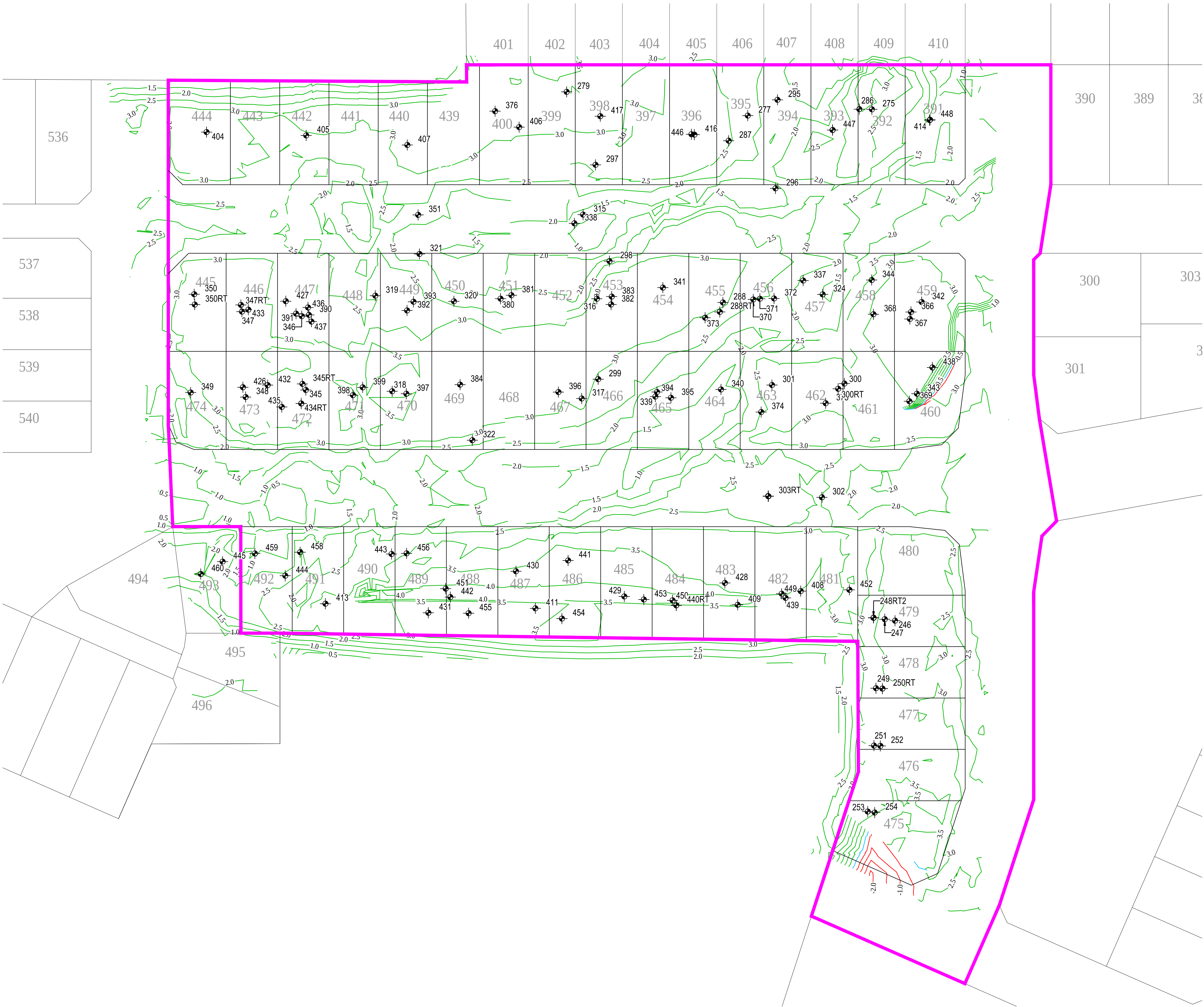
- NOTES:
1. SURVEY DATA SUPPLIED BY LYSAGHT CONSULTANTS LTD.
 2. LEVELS IN TERMS OF MOTURIKI DATUM 1953.
 3. COORDINATES IN TERMS OF GD2000, BOP CIRCUIT.
 4. CONTOUR AT 0.2m INTERVALS.

DRAFT

File Date: 26/10/2019 2:54:03 PM Office: NZREG Filename: P-PROJECTS\247593 - ZARIBA\ASSET PARTON ROAD PAPAMOA\ADD\BKG\CLUST\STAGES 3D - 3E\247593\ADD\BKG-CLUST.DWG



CLIENT		REV	DATE	REVISION DETAILS	APPROVED	SCALE	SIZE	WORK IN PROGRESS NOT FOR CONSTRUCTION	PROJECT	TERRACE VIEWS SUBDIVISION PARTON ROAD, PAPAMOA, TAURANGA										
ZARIBA PROPERTIES LIMITED	A	??????	FOR INFORMATION	R.GRIFFITHS		1:500	A1													
						DRAWN L.HODGSON		APPROVED DATE R.GRIFFITHS												
						DESIGNED M.BUOB			TITLE	STAGE 3D AND 3E PRE-DEVELOPMENT LANDFORM PLAN										
						CHECKED K.KISHORE			DRAWING No.	PROJECT No.	WBS	TYPE	DISC	NUMBER	REV					
										247593	-	0320	-	DRG	-	G	-	0001	-	A



LEGEND	
—	STAGE 3D AND 3E BOUNDARY
—	CUT CONTOURS AT 0.5m INTERVALS
—	FILL CONTOURS AT 0.5m INTERVALS
—	ZERO CUT FILL
	COMPLIANCE TEST LOCATIONS
	CPT TEST LOCATIONS

- NOTES:
- SURVEY DATA SUPPLIED BY LYSAGHT CONSULTANTS LTD.
 - TEST LOCATIONS SUPPLIED BY JMC LTD.
 - LEVELS IN TERMS OF MOTURIKI DATUM 1953.
 - COORDINATES IN TERMS OF GD2000, BOP CIRCUIT.



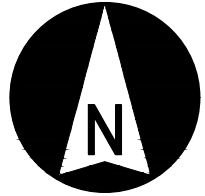
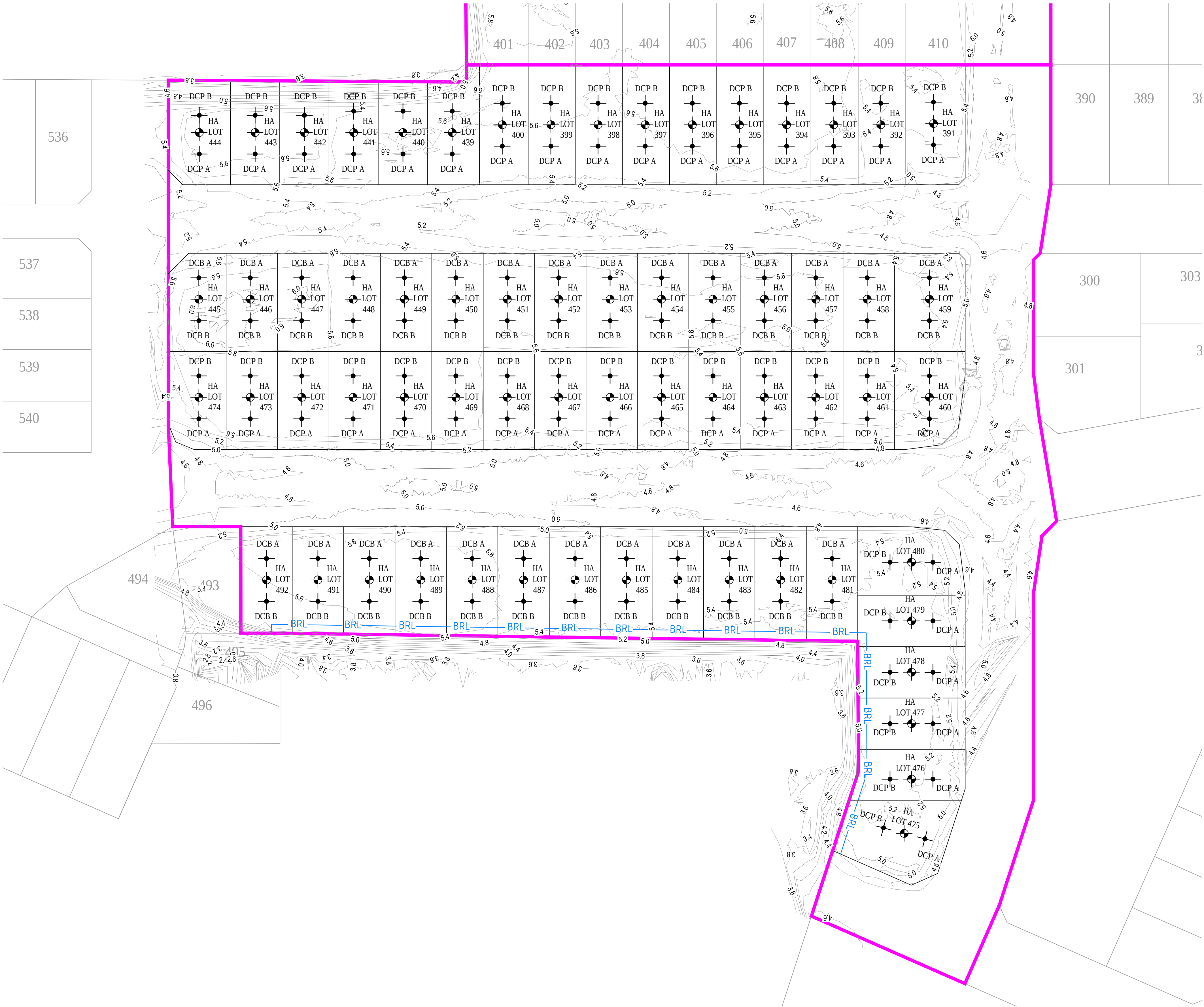
REV	DATE	REVISION DETAILS	APPROVED
A	?????	FOR INFORMATION	R.GRIFFITHS

SCALE	SIZE
1:500	A1
DRAWN L.HODGSON	
DESIGNED M.BUOB	
CHECKED K.KISHORE	

WORK IN PROGRESS NOT FOR CONSTRUCTION	
APPROVED	DATE
R.GRIFFITHS	

PROJECT	TERRACE VIEWS SUBDIVISION PARTON ROAD, PAPAMOA, TAURANGA
TITLE	STAGE 3D AND 3E CUT TO FILL EARTHWORKS AND COMPLIANCE TEST LOCATION PLAN
DRAWING No.	PROJECT No. 247593 - WBS 0320 - TYPE DRG - DISC G - NUMBER 0002 - REV A

DRAFT



LEGEND

5.8

AS BUILT LANDFORM MAJOR
CONTOURS AT 0.1m INTERVALS

HA

LOT
392

DCP A

HAND AUGER

DYNAMIC CONE PENETROMETER TEST

STAGE 3D AND 3E BOUNDARY

TOP OF BANK

BRL

BUILDING RESTRICTION LINE

- NOTES:
1. SURVEY INFORMATION SUPPLIED BY LYSAGHT CONSULTANTS LTD.

2. LEVELS IN TERMS OF MOTURIKI DATUM 1953.

3. COORDINATES IN TERMS OF GD2000, BOP CIRCUIT.

4. TEST POSITIONS INDICATIVE ONLY.

5. TOP OF BANK POSITION IS BASED ON SURVEY BY LYSAGHT FINISHED LANDFORM DATA.



REV	DATE	REVISION DETAILS
A	01.02.19	FOR INFORMATION
B	05.02.19	FOR INFORMATION

APPROVED
R.GRIFFITHS
R.GRIFFITHS

SCALE	SIZE
1:500	A1
DRAWN	
L.HODGSON	
DESIGNED	
M.BUOB	
CHECKED	
R.GRIFFITHS	

INFORMATION	
NOT FOR CONSTRUCTION	
APPROVED	DATE
R.GRIFFITHS	01.02.19
R.GRIFFITHS	

PROJECT	TERRACE VIEWS SUBDIVISION PARTON ROAD, PAPAMOA, TAURANGA
TITLE	STAGE 3D AND 3E FINAL AS-BUILT SUBGRADE LANDFORM AND LOT BY LOT TEST LOCATION PLAN
DRAWING No.	PROJECT No. 247593 - WBS 0320 - TYPE DRG - DISC G - NUMBER 0003 - REV B



C

Fill Compliance
Testing

Appendix C

Fill Compliance Testing

Report No: ND:BOP17W1054
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.



Approved Signatory: Rob Ermens
(Lab Manager)

Date of Issue: 9/11/2017

Testing Details

Site Tested: Lot Testing
Tested By: Jake Brown
Date Tested: 30/10/2017
Time Tested: 14:30
Material: Rhyolite Pumice Sand
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1 - 1986

Compaction Target Details

Material Sample ID: BOP17S-01212
MDD Method: NZS 4402:1986 Test 4.1.1 - 1986
Max. Dry Density: 1.34 t/m³ @ 27 %
Min. Dry Density (t/m³):
Solid Density Type: Measured

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
245	24.3	1.69	1.36	101
246	25.7	1.61	1.28	96
247	24.2	1.68	1.35	101
248	27.2	1.56	1.23	92
249	25.1	1.72	1.37	102
250	27.2	1.56	1.23	92
251	26.4	1.68	1.33	99
252	27.1	1.72	1.35	101
253	23.6	1.66	1.34	100
254	24.8	1.68	1.35	101

Comments

Report not IANZ endorsed due to density measurements being outside of NDM range of calibration.
GPS locations recorded by client on site.

Report No: ND:BOP18W0089
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The tests reported herein (unless otherwise indicated) have been performed in accordance with the laboratory's scope of accreditation. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.




Approved Signatory: Rob Ermens
(Lab Manager)
IANZ Accreditation No: 749
Date of Issue: 2/02/2018

Testing Details

Site Tested: Fill Area
Tested By: Jake Brown
Date Tested: 25/01/2018
Time Tested: 12:00
Material: Rhyolite Pumice Sand
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1 - 1986

Compaction Target Details

Material Sample ID: BOP18S-00172
MDD Method: NZS 4402:1986 Test 4.1.1 - 1986
Max. Dry Density: 1.48 t/m³ @ 23 %
Min. Dry Density (t/m³):
Solid Density Type: Measured

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
275	19.0	1.80	1.51	102
276	16.4	1.65	1.42	96
277	16.7	1.79	1.54	104
278	18.7	1.64	1.38	93
279	15.9	1.76	1.52	103

Comments

Wet density measurements that fall below 1.78t/m³ are not accredited due to being outside the NDM range of calibration.
GPS locations recorded by client on site.

Report No: ND:BOP18W0200
Issue No: 1

Nuclear Density Report

Client: Jason Mather JMC Limited PO Box 16070 Bethlehem Tauranga 3147 NZ Project: Zariba	The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.  Approved Signatory: Rob Ermens (Lab Manager) Date of Issue: 27/02/2018

Testing Details Site Tested: Lot Testing Tested By: Jake Brown Date Tested: 22/02/2018 Time Tested: 10:00 Material: Rhyolite Pumice Sand Material Source: Paengaroa Sands Ltd Quarry Specification: Contract Specification Field Methods: NZS 4407:2015 Test 4.2 Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1 - 1986	Compaction Target Details Material Sample ID: BOP18S-00172 MDD Method: NZS 4402:1986 Test 4.1.1 - 1986 Max. Dry Density: 1.48 t/m ³ @ 23 % Min. Dry Density (t/m³): Solid Density Type: Measured

Test Results				
Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
285	19.1	1.69	1.42	96
286	17.1	1.66	1.42	96
287	19.9	1.74	1.45	98
288	19.7	1.54	1.29	87

Comments

Report not IANZ endorsed due to density measurements being outside of NDM range of calibration.
GPS locations recorded by client on site.

Report No: ND:BOP18W0225
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.



Approved Signatory: Rob Ermens
(Lab Manager)

Date of Issue: 28/02/2018

Testing Details

Site Tested: Fill Area
Tested By: Rob Ermens
Date Tested: 26/02/2018
Time Tested: 12:30
Material: Rhyolite Pumice Sand
Material Source: Paengaroa Sands Ltd Quarry
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1 - 1986

Compaction Target Details

Material Sample ID: BOP18S-00172
MDD Method: NZS 4402:1986 Test 4.1.1 - 1986
Max. Dry Density: 1.48 t/m³ @ 23 %
Min. Dry Density (t/m³):
Solid Density Type: Measured

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
289	15.1	1.72	1.50	101
290	20.1	1.70	1.41	95
291	20.2	1.74	1.45	98
292	21.0	1.74	1.44	97
293	21.8	1.72	1.42	96
294	19.6	1.64	1.37	93
295	21.1	1.66	1.37	93
296	19.3	1.67	1.40	95
297	18.4	1.62	1.37	93
298	16.3	1.79	1.54	104
299	21.3	1.70	1.40	95

Comments

Report not IANZ endorsed due to density measurements being outside of NDM range of calibration.
GPS locations recorded by client on site.

Report No: ND:BOP18W0474
Issue No: 1

Nuclear Density Report

Client: Jason Mather JMC Limited PO Box 16070 Bethlehem Tauranga 3147 NZ Project: Zariba	The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.  Approved Signatory: Rob Ermens (Lab Manager) Date of Issue: 27/04/2018

Testing Details Site Tested: Fill Area Tested By: Andre Shine Date Tested: 24/04/2018 Time Tested: 12:30 Material: Rhyolite Pumice Sand Material Source: Paengaroa Sands Ltd Quarry Specification: Contract Specification Field Methods: NZS 4407:2015 Test 4.2 Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1 - 1986	Compaction Target Details Material Sample ID: BOP18S-00172 MDD Method: NZS 4402:1986 Test 4.1.1 - 1986 Max. Dry Density: 1.48 t/m ³ @ 23 % Min. Dry Density (t/m³): Solid Density Type: Measured

Test Results				
Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
300	22.7	1.69	1.37	93
301	19.6	1.70	1.42	96

Comments

Report not IANZ endorsed due to density measurements being outside of NDM range of calibration.
GPS locations recorded by client on site.

Report No: ND:BOP18W0476
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.



Approved Signatory: Rob Ermens
(Lab Manager)

Date of Issue: 27/04/2018

Testing Details

Site Tested: Fill Area
Tested By: Andre Shine
Date Tested: 24/04/2018
Time Tested: 12:30
Material: Pumice
Material Source: Marsh Pit - Maniatutu Rd
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1

Compaction Target Details

Material Sample ID: External
MDD Method: NZS 4402:1986 Test 4.1.1*
Max. Dry Density: 0.82 t/m³ @ 73.0 %
Min. Dry Density (t/m³):
Solid Density Type:

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
302	73.9	1.36	0.78	95
303	71.6	1.31	0.77	94

Comments

* Test was conducted externally and is not accredited by this laboratory.
Report not IANZ endorsed due to density measurements being outside of NDM range of calibration.
GPS locations recorded by client on site.
MDD-OMC target taken from FH report reference BOP18S-00793

Report No: ND:BOP18W0530
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.



Approved Signatory: Rob Ermens
(Lab Manager)

Date of Issue: 9/05/2018

Testing Details

Site Tested: Fill Area
Tested By: Andre Shine
Date Tested: 4/05/2018
Time Tested: 12:00
Material: Pumice
Material Source: Marsh Farm
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1

Compaction Target Details

Material Sample ID: External
MDD Method: NZS 4402:1986 Test 4.1.1*
Max. Dry Density: 0.82 t/m³ @ 73.0 %
Min. Dry Density (t/m³):
Solid Density Type:

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
303RT	34.6	1.45	1.07	130

Comments

* Test was conducted externally and is not accredited by this laboratory.
Report not IANZ endorsed due to density measurements being outside of NDM range of calibration
MDD-OMC target taken from FH report reference BOP18S-00793
GPS location recorded by client on site

Report No: ND:BOP18W0593
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The tests reported herein (unless otherwise indicated) have been performed in accordance with the laboratory's scope of accreditation. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.




Approved Signatory: Rob Ermens
(Lab Manager)
IANZ Accreditation No: 749
Date of Issue: 22/05/2018

Testing Details

Site Tested: Lot testing
Tested By: Andre Shine
Date Tested: 21/05/2018
Time Tested: 09:00
Material: Rhyolite Pumice Sand
Material Source: Paengaroa Sands Ltd Quarry
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1 - 1986

Compaction Target Details

Material Sample ID: BOP18S-00172
MDD Method: NZS 4402:1986 Test 4.1.1 - 1986
Max. Dry Density: 1.48 t/m³ @ 23 %
Min. Dry Density (t/m³):
Solid Density Type: Measured

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
315	23.2	1.70	1.38	93
316	26.7	1.76	1.39	94
317	24.2	1.74	1.40	95
318	23.3	1.79	1.45	98
319	22.8	1.81	1.47	99
320	26.0	1.77	1.41	95
321	23.0	1.79	1.46	99
322	25.3	1.72	1.37	93

Statistical Data

Mean Relative Compaction %: 95.7

Comments

Wet density measurements that fall below 1.72t/m³ are not accredited due to being outside the NDM range of calibration.
GPS locations were recorded by client on site
Relative compaction results and averages are not IANZ accredited

Report No: ND:BOP18W0946
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.



Approved Signatory: Rob Ermens
(Lab Manager)

Date of Issue: 21/08/2018

Testing Details

Site Tested: Zariba
Tested By: Andre Shine
Date Tested: 16/08/2018
Time Tested: 15:00
Material: Rhyolite Pumice Sand
Material Source: Paengaroa Sands Ltd Quarry
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1 - 1986

Compaction Target Details

Material Sample ID: BOP18S-00172
MDD Method: NZS 4402:1986 Test 4.1.1 - 1986
Max. Dry Density: 1.48 t/m³ @ 23 %
Min. Dry Density (t/m³):
Solid Density Type: Measured

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
324	21.6	1.73	1.42	96

Comments

Report not IANZ endorsed due to density measurements being outside of NDM range of calibration.
GPS locations recorded by client on site.

Report No: ND:BOP18W1030
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The tests reported herein (unless otherwise indicated) have been performed in accordance with the laboratory's scope of accreditation. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.




Approved Signatory: Rob Ermens
(Lab Manager)
IANZ Accreditation No: 749
Date of Issue: 17/09/2018

Testing Details

Site Tested: Zariba
Tested By: Andre Shine
Date Tested: 11/09/2018
Time Tested: 14:00
Material: Pumice Sand
Material Source: Paengaroa Sands Ltd Quarry
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1 - 1986

Compaction Target Details

Material Sample ID: BOP18S-00172
MDD Method: NZS 4402:1986 Test 4.1.1 - 1986
Max. Dry Density: 1.48 t/m³ @ 23 %
Min. Dry Density (t/m³):
Solid Density Type: Measured

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
337	19.1	1.70	1.42	96
338	17.6	1.81	1.54	104
339	23.7	1.73	1.40	95
340	18.5	1.71	1.44	97
341	21.0	1.79	1.48	100
342	17.0	1.72	1.47	99
343	20.7	1.77	1.47	99
344	25.9	1.79	1.42	96

Comments

Wet density measurements that fall below 1.78t/m³ are not accredited due to being outside the NDM range of calibration.
GPS locations recorded by client on site.

Report No: ND:BOP18W1039
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The tests reported herein (unless otherwise indicated) have been performed in accordance with the laboratory's scope of accreditation. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.




Approved Signatory: Rob Ermens
(Lab Manager)
IANZ Accreditation No: 749
Date of Issue: 18/09/2018

Testing Details

Site Tested: Zariba
Tested By: Andre Shine
Date Tested: 14/09/2018
Time Tested: 15:00
Material: Pumice Sand
Material Source: Paengaroa Sands Ltd Quarry
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1 - 1986

Compaction Target Details

Material Sample ID: BOP18S-00172
MDD Method: NZS 4402:1986 Test 4.1.1 - 1986
Max. Dry Density: 1.48 t/m³ @ 23 %
Min. Dry Density (t/m³):
Solid Density Type: Measured

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
345	27.9	1.66	1.30	88
346	23.7	1.74	1.41	95
347	28.1	1.69	1.32	89
348	20.5	1.75	1.45	98
349	17.7	1.78	1.52	103
350	32.1	1.62	1.23	83
351	23.4	1.73	1.40	95

Comments

Wet density measurements that fall below 1.78t/m³ are not accredited due to being outside the NDM range of calibration.
GPS locations recorded by client on site.
Relative compaction results and averages are not IANZ accredited.

Report No: ND:BOP18W1074
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.



Approved Signatory: Rob Ermens
(Lab Manager)

Date of Issue: 25/09/2018

Testing Details

Site Tested: Lot testing
Tested By: Rob Ermens
Date Tested: 21/09/2018
Time Tested: 10:30
Material: Rhyolite Pumice Sand
Material Source: Paengaroa Sands Ltd Quarry
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1 - 1986

Compaction Target Details

Material Sample ID: BOP18S-00172
MDD Method: NZS 4402:1986 Test 4.1.1 - 1986
Max. Dry Density: 1.48 t/m³ @ 23 %
Min. Dry Density (t/m³):
Solid Density Type: Measured

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
345RT	23.1	1.70	1.38	93
347RT	23.1	1.75	1.42	96
350RT	24.2	1.72	1.39	94

Comments

GPS locations recorded by client on site.
Relative compaction results and averages are not IANZ accredited
Wet density measurements that fall below 1.78t/m³ are not accredited due to being outside the NDM range of calibration.

Report No: ND:BOP18W1125
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.



Approved Signatory: Rob Ermens
(Lab Manager)

Date of Issue: 8/10/2018

Testing Details

Site Tested: Lot Testing
Tested By: Jake Brown
Date Tested: 4/10/2018
Time Tested: 09:00
Material: Rhyolite Pumice Sand
Material Source: Paengaroa Sands Ltd Quarry
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1 - 1986

Compaction Target Details

Material Sample ID: BOP18S-00172
MDD Method: NZS 4402:1986 Test 4.1.1 - 1986
Max. Dry Density: 1.48 t/m³ @ 23 %
Min. Dry Density (t/m³):
Solid Density Type: Measured

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
288RT	24.4	1.69	1.36	92

Comments

Report not IANZ endorsed due to density measurements being outside of NDM range of calibration.
GPS locations recorded by client on site.

Report No: ND:BOP18W1210
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The tests reported herein (unless otherwise indicated) have been performed in accordance with the laboratory's scope of accreditation. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.




Approved Signatory: Rob Ermens
(Lab Manager)
IANZ Accreditation No: 749
Date of Issue: 6/11/2018

Testing Details

Site Tested: Stage 3C/ Stage D/E
Tested By: Rob Ermens
Date Tested: 25/10/2018
Time Tested: 12:00
Material: Rhyolite Pumice Sand
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1 - 1986

Compaction Target Details

Material Sample ID: BOP18S-00005
MDD Method: NZS 4402:1986 Test 4.1.1 - 1986
Max. Dry Density: 1.48 t/m³ @ 24 %
Min. Dry Density (t/m³):
Solid Density Type: Measured

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
365	20.2	1.75	1.46	99
366	28.7	1.81	1.41	95
367	24.9	1.71	1.37	93
368	17.2	1.76	1.50	101
369	26.0	1.82	1.45	98

Comments

Wet density measurements that fall below 1.78 t/m³ are outside the NDM calibration range and are excluded from the endorsement of this report.
Relative compaction results and averages are not accredited.
GPS locations recorded by client on site

Report No: ND:BOP18W1248
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.



Approved Signatory: Rob Ermens
(Lab Manager)

Date of Issue: 7/11/2018

Testing Details

Site Tested: Stage D/E
Tested By: Rob Ermens
Date Tested: 25/10/2018
Time Tested: 12:00
Material: Rhyolite Pumice Sand
Material Source: Paengaroa Sands Ltd Quarry
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1 - 1986

Compaction Target Details

Material Sample ID: BOP18S-03601
MDD Method: NZS 4402:1986 Test 4.1.1 - 1986
Max. Dry Density: 1.40 t/m³ @ 25 %
Min. Dry Density (t/m³):
Solid Density Type: Measured

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
370	21.0	1.64	1.36	97
371	21.2	1.62	1.34	96
372	24.6	1.64	1.32	94
373	22.5	1.57	1.29	92
374	23.7	1.67	1.35	96
375	21.9	1.64	1.35	96

Comments

Report not IANZ endorsed due to density measurements being outside of NDM range of calibration.
GPS locations recorded by client on site.

Report No: ND:BOP18W1263
Issue No: 1

Nuclear Density Report

Client: Jason Mather JMC Limited PO Box 16070 Bethlehem Tauranga 3147 NZ Project: Zariba	The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.  Approved Signatory: Rob Ermens (Lab Manager) Date of Issue: 9/11/2018

Testing Details Site Tested: Stage D/E Tested By: Jake Brown Date Tested: 29/10/2018 Time Tested: 13:00 Material: Pumice Sand Material Source: Marbles - Maniatutu Rd Specification: Contract Specification Field Methods: NZS 4407:2015 Test 4.2 Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1	Compaction Target Details Material Sample ID: External MDD Method: NZS 4402:1986 Test 4.1.1* Max. Dry Density: 1.33 t/m ³ @ 29.0 % Min. Dry Density (t/m³): Solid Density Type:

Test Results				
Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
376	29.9	1.56	1.20	90
377	53.8	1.34	0.87	65
378	42.4	1.51	1.06	80
379	41.3	1.50	1.06	80

Comments

* Test was conducted externally and is not accredited by this laboratory.
Report not IANZ endorsed due to density measurements being outside of NDM range of calibration.
MDD-OMC target from FH report reference MDD:BOP18S-03298. GPS locations recorded by client on site.

Nuclear Density Report

Report No: ND:BOP18W1265
Issue No: 1

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The tests reported herein (unless otherwise indicated) have been performed in accordance with the laboratory's scope of accreditation. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.




Approved Signatory: Rob Ermens
(Lab Manager)
IANZ Accreditation No: 749
Date of Issue: 12/11/2018

Testing Details

Site Tested: Stage D/E
Tested By: Jake Brown
Date Tested: 30/10/2018
Time Tested: 11:30
Material: Rhyolite Pumice Sand
Material Source: Paengaroa Sands Ltd Quarry
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1 - 1986

Compaction Target Details

Material Sample ID: BOP18S-03601
MDD Method: NZS 4402:1986 Test 4.1.1 - 1986
Max. Dry Density: 1.40 t/m³ @ 25 %
Min. Dry Density (t/m³):
Solid Density Type: Measured

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
380	34.5	1.68	1.25	89
381	21.9	1.78	1.46	104
382	30.9	1.66	1.27	91
383	28.0	1.74	1.36	97
384	28.0	1.76	1.37	98

Comments

Wet density measurements that fall below 1.78t/m³ are not accredited due to being outside the NDM range of calibration. Relative compaction results and averages are not IANZ accredited. GPS locations recorded by client on site.

Report No: ND:BOP18W1284
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.



Approved Signatory: Rob Ermens
(Lab Manager)

Date of Issue: 13/11/2018

Testing Details

Site Tested: Stage D/E
Tested By: Jake Brown
Date Tested: 7/11/2018
Time Tested: 15:10
Material: Rhyolite Pumice Sand
Material Source: Paengaroa Sands Ltd Quarry
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1 - 1986

Compaction Target Details

Material Sample ID: BOP18S-03601
MDD Method: NZS 4402:1986 Test 4.1.1 - 1986
Max. Dry Density: 1.40 t/m³ @ 25 %
Min. Dry Density (t/m³):
Solid Density Type: Measured

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
390	26.0	1.55	1.23	88
391	33.7	1.53	1.15	82
392	15.5	1.73	1.50	107
393	28.1	1.74	1.36	97
394	23.2	1.63	1.32	94
395	23.7	1.78	1.44	103
396	19.4	1.71	1.43	102
397	10.2	1.71	1.55	111

Comments

Report not IANZ endorsed due to density measurements being outside of NDM range of calibration.
GPS locations recorded by client on site.

Report No: ND:BOP18W1286
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.



Approved Signatory: Rob Ermens
(Lab Manager)

Date of Issue: 13/11/2018

Testing Details

Site Tested: Stage D/E
Tested By: Joseph Hughes
Date Tested: 8/11/2018
Time Tested: 09:00
Material: Rhyolite Pumice Sand
Material Source: Paengaroa Sands Ltd Quarry
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1 - 1986

Compaction Target Details

Material Sample ID: BOP18S-03601
MDD Method: NZS 4402:1986 Test 4.1.1 - 1986
Max. Dry Density: 1.40 t/m³ @ 25 %
Min. Dry Density (t/m³):
Solid Density Type: Measured

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
398	27.2	1.57	1.24	89
399	22.8	1.66	1.36	97
400	33.8	1.43	1.07	76
401	45.7	1.57	1.08	77

Comments

Report not IANZ endorsed due to density measurements being outside of NDM range of calibration.
GPS locations recorded by client on site.

Report No: ND:BOP18W1288
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.



Approved Signatory: Rob Ermens
(Lab Manager)

Date of Issue: 14/11/2018

Testing Details

Site Tested: Fill Area
Tested By: Joseph Hughes
Date Tested: 8/11/2018
Time Tested: 10:00
Material: Pumice Sand
Material Source: Marbles - Maniatutu Rd
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1

Compaction Target Details

Material Sample ID: External
MDD Method: NZS 4402:1986 Test 4.1.1*
Max. Dry Density: 1.33 t/m³ @ 29.0 %
Min. Dry Density (t/m³):
Solid Density Type:

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
404	25.7	1.56	1.24	93
405	27.8	1.64	1.29	97
406	19.5	1.75	1.47	111
407	18.9	1.75	1.47	111

Comments

* Test was conducted externally and is not accredited by this laboratory.
Report not IANZ endorsed due to density measurements being outside of NDM range of calibration.
MDD-OMC target from FH report reference MDD:BOP18S-03298. GPS locations recorded by client on site.

Report No: ND:BOP18W1309
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.



Approved Signatory: Rob Ermens
(Lab Manager)

Date of Issue: 21/11/2018

Testing Details

Site Tested: Fill Area
Tested By: Jake Brown
Date Tested: 12/11/2018
Time Tested: 11:10
Material: Pumice Sand
Material Source: Marbles - Maniatutu Rd
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1 - 1986

Compaction Target Details

Material Sample ID: External
MDD Method: NZS 4402:1986 Test 4.1.1 - 1986*
Max. Dry Density: 1.33 t/m³ @ 29.0 %
Min. Dry Density (t/m³):
Solid Density Type: Measured

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
408	21.7	1.52	1.25	94
409	31.8	1.56	1.19	89
410	42.7	1.65	1.15	86
411	33.1	1.61	1.21	91
412	44.5	1.56	1.08	81
413	30.8	1.61	1.23	92

Comments

* Test was conducted externally and is not accredited by this laboratory.
Report not IANZ endorsed due to density measurements being outside of NDM range of calibration.
MDD-OMC target from report reference BOP18S-03298.

Report No: ND:BOP18W1310
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.



Approved Signatory: Rob Ermens
(Lab Manager)

Date of Issue: 21/11/2018

Testing Details

Site Tested: Fill Area
Tested By: Jake Brown
Date Tested: 12/11/2018
Time Tested: 11:10
Material: Rhyolite Pumice Sand
Material Source: Paengaroa Sands Ltd Quarry
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1 - 1986

Compaction Target Details

Material Sample ID: BOP18S-03601
MDD Method: NZS 4402:1986 Test 4.1.1 - 1986
Max. Dry Density: 1.40 t/m³ @ 25 %
Min. Dry Density (t/m³):
Solid Density Type: Measured

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
414	27.0	1.60	1.26	90
415	26.4	1.55	1.23	88
416	24.2	1.61	1.30	93
417	21.9	1.59	1.30	93

Comments

Report not IANZ endorsed due to density measurements being outside of NDM range of calibration.
GPS locations recorded by client on site.

Report No: ND:BOP18W1333
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The tests reported herein (unless otherwise indicated) have been performed in accordance with the laboratory's scope of accreditation. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.




Approved Signatory: Rob Ermens
(Lab Manager)
IANZ Accreditation No: 749
Date of Issue: 26/11/2018

Testing Details

Site Tested: Fill Area
Tested By: Jake Brown
Date Tested: 14/11/2018
Time Tested: 16:00
Material: Rhyolite Pumice Sand
Material Source: Paengaroa Sands Ltd Quarry
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1 - 1986

Compaction Target Details

Material Sample ID: BOP18S-00172
MDD Method: NZS 4402:1986 Test 4.1.1 - 1986
Max. Dry Density: 1.48 t/m³ @ 23.5 %
Min. Dry Density (t/m³):
Solid Density Type: Measured

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
426	19.5	1.79	1.50	102

Comments

Relative compaction results and averages are not IANZ accredited.
GPS locations recorded by client on site.

Report No: ND:BOP18W1334
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.



Approved Signatory: Rob Ermens
(Lab Manager)

Date of Issue: 26/11/2018

Testing Details

Site Tested: Fill Area
Tested By: Jake Brown
Date Tested: 14/11/2018
Time Tested: 16:00
Material: Pumice Sand
Material Source: Marbles - Maniatutu Rd
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1 - 1986

Compaction Target Details

Material Sample ID: External
MDD Method: NZS 4402:1986 Test 4.1.1 - 1986*
Max. Dry Density: 1.33 t/m³ @ 29 %
Min. Dry Density (t/m³):
Solid Density Type: Measured

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
427	40.5	1.60	1.14	86

Comments

* Test was conducted externally and is not accredited by this laboratory.
Report not IANZ endorsed due to density measurements being outside of NDM range of calibration.
MDD-OMC target from FH report reference BOP18S-03298.

Report No: ND:BOP18W1339
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.



Approved Signatory: Rob Ermens
(Lab Manager)

Date of Issue: 26/11/2018

Testing Details

Site Tested: Stage D/E
Tested By: Jake Brown
Date Tested: 16/11/2018
Time Tested: 12:30
Material: Pumice Sand
Material Source: Marbles - Maniatutu Rd
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1

Compaction Target Details

Material Sample ID: External
MDD Method: NZS 4402:1986 Test 4.1.1*
Max. Dry Density: 1.33 t/m³ @ 29 %
Min. Dry Density (t/m³):
Solid Density Type: Assumed

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
428	25.8	1.54	1.23	92
429	30.7	1.65	1.26	95
430	27.7	1.68	1.31	99
431	22.5	1.59	1.30	98
432	23.7	1.59	1.29	97
433	25.4	1.65	1.31	99

Comments

* Test was conducted externally and is not accredited by this laboratory.
 Report not IANZ endorsed due to density measurements being outside of NDM range of calibration.
 GPS locations recorded by client on site.

Report No: ND:BOP18W1409
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.



Approved Signatory: Rob Ermens
(Lab Manager)

Date of Issue: 11/12/2018

Testing Details

Site Tested: Lot testing
Tested By: Jake Brown
Date Tested: 7/12/2018
Time Tested: 11:00
Material: Pumice Sand
Material Source: Marbles - Maniatutu Rd
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS4402:1986 Test 4.1.1

Compaction Target Details

Material Sample ID: External
MDD Method: NZS4402:1986 Test 4.1.1*
Max. Dry Density: 1.33 t/m³ @ 29 %
Min. Dry Density (t/m³):
Solid Density Type: Assumed

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
434	34.7	1.53	1.14	85
435	30.1	1.67	1.28	97
436	24.3	1.64	1.32	100
437	26.4	1.61	1.27	96

Comments

* Test was conducted externally and is not accredited by this laboratory.
MDD-OMC was taken from FH report reference BOP18S-03289
GPS locations recorded by client on site.
Report not IANZ endorsed due to density measurements being outside of NDM range of calibration

Report No: ND:BOP18W1410
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.



Approved Signatory: Rob Ermens
(Lab Manager)

Date of Issue: 11/12/2018

Testing Details

Site Tested: Lot testing
Tested By: Jake Brown
Date Tested: 7/12/2018
Time Tested: 11:00
Material: Rhyolite Pumice Sand
Material Source: Paengaroa Sands Ltd Quarry
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, Plateau Test

Compaction Target Details

Material Sample ID: External
MDD Method: Plateau Test*
Max. Dry Density: 1.28 t/m³ @ 23 %
Min. Dry Density (t/m³):
Solid Density Type: Measured

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
438	26.5	1.60	1.27	99

Comments

* Test was conducted externally and is not accredited by this laboratory.
Report not IANZ endorsed due to density measurements being outside of NDM range of calibration
GPS location recorded by client on site.
Plateau target from FH report reference ND:BOP18W1361.

Report No: ND:BOP18W1428
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.



Approved Signatory: Rob Ermens
(Lab Manager)

Date of Issue: 18/12/2018

Testing Details

Site Tested: Lot Testing
Tested By: Jake Brown
Date Tested: 14/12/2018
Time Tested: 14:30
Material: Pumice Sand
Material Source: Marbles - Maniatutu Rd
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, Plateau Test

Compaction Target Details

Material Sample ID: External
MDD Method: Plateau Test*
Max. Dry Density: 1.29 t/m³ @ 25 %
Min. Dry Density (t/m³):
Solid Density Type: Assumed

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
439	16.8	1.56	1.34	104
440	29.1	1.53	1.18	92
441	27.9	1.70	1.33	103
442	17.7	1.54	1.31	101
443	19.7	1.59	1.33	103
444	19.5	1.56	1.31	101
445	19.0	1.63	1.37	106
449	36.4	1.67	1.22	95
450	30.7	1.64	1.25	97
451	30.4	1.65	1.27	98

Comments

* Test was conducted externally and is not accredited by this laboratory.
Report not IANZ endorsed due to density measurements being outside of NDM range of calibration.
MDD-OMC target from FH Plateau test report BOP18W1355.
GPS locations recorded by client on site.

Report No: ND:BOP18W1429
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.



Approved Signatory: Rob Ermens
(Lab Manager)

Date of Issue: 19/12/2018

Testing Details

Site Tested: Lot Testing
Tested By: Jake Brown
Date Tested: 14/12/2018
Time Tested: 14:30
Material: Rhyolite Pumice Sand
Material Source: Paengaroa Sands Ltd Quarry
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, Plateau Test

Compaction Target Details

Material Sample ID: External
MDD Method: Plateau Test*
Max. Dry Density: 1.22 t/m³ @ 25 %
Min. Dry Density (t/m³):
Solid Density Type: Assumed

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
446	30.8	1.63	1.25	102
447	27.3	1.62	1.27	104
448	28.5	1.57	1.22	100

Comments

* Test was conducted externally and is not accredited by this laboratory.
Report not IANZ endorsed due to density measurements being outside of NDM range of calibration.
MDD-OMC target from FH Plateau test report BOP18W1361.
GPS locations recorded by client on site.

Report No: ND:BOP18W1438
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.



Approved Signatory: Rob Ermens
(Lab Manager)

Date of Issue: 20/12/2018

Testing Details

Site Tested: Lot Testing
Tested By: Jake Brown
Date Tested: 19/12/2018
Time Tested: 08:45
Material: Rhyolite Pumice Sand
Material Source: Paengaroa Sands Ltd Quarry
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, Calculated

Compaction Target Details

Material Sample ID: External
MDD Method: Calculated*
Max. Dry Density: 1.26 t/m³ @ 25 %
Min. Dry Density (t/m³):
Solid Density Type: Assumed

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
434RT	24.1	1.62	1.30	103

Comments

* Test was conducted externally and is not accredited by this laboratory.
GPS location recorded by client on site. Report not IANZ endorsed due to density measurements being outside of NDM range of calibration.
MDD value was supplied by Aurecon based on Plateau tests from FH reports reference BOP18W1355, BOP18W1359 and BOP18W1361 with a 50:50 blend of Marbles & Paengaroa Sands Pumice Sand materials.

Report No: ND:BOP18W1440
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.



Approved Signatory: Rob Ermens
(Lab Manager)

Date of Issue: 20/12/2018

Testing Details

Site Tested: Lot testing
Tested By: Jake Brown
Date Tested: 19/12/2018
Time Tested: 08:45
Material: Pumice Sand
Material Source: Marbles - Maniatutu Rd
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, Plateau Test

Compaction Target Details

Material Sample ID: External
MDD Method: Plateau Test*
Max. Dry Density: 1.29 t/m³ @ 25 %
Min. Dry Density (t/m³):
Solid Density Type: Assumed

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
440 RT	21.3	1.58	1.30	101
452	16.3	1.63	1.41	109
453	16.1	1.56	1.35	104
454	26.4	1.62	1.28	100
455	26.7	1.60	1.27	98
456	23.7	1.63	1.32	102
457	29.3	1.56	1.20	93
458	22.9	1.60	1.30	101
459	24.8	1.59	1.28	99
460	26.2	1.62	1.28	99

Statistical Data

Mean Relative Compaction %: 100.7

Comments

* Test was conducted externally and is not accredited by this laboratory.
Report not IANZ endorsed due to density measurements being outside of NDM range of calibration
GPS locations recorded by client on site
MDD-OMC was taken from FH report reference BOP18W1355.

Nuclear Density Report

Report No: ND:BOP18W0086
Issue No: 2
This report replaces all previous issues of report no 'ND:BOP18W0086'.

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.



Approved Signatory: Rob Ermens
(Lab Manager)

Date of Issue: 1/02/2018

Testing Details

Site Tested: Fill Area - Retests

Tested By: Jake Brown

Date Tested: 24/01/2018

Time Tested: 14:15

Material: Rhyolite Pumice Sand

Specification: Contract Specification

Field Methods: NZS 4407:2015 Test 4.2

Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1 - 1986

Compaction Target Details

Material Sample ID: BOP17S-01212

MDD Method: NZS 4402:1986 Test 4.1.1 - 1986

Max. Dry Density: 1.34 t/m³ @ 27 %

Min. Dry Density (t/m³):

Solid Density Type: Measured

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
248RT	15.5	1.70	1.47	110
250RT	17.4	1.65	1.41	105
256RT	18.2	1.59	1.34	100
257RT	21.0	1.61	1.33	99
266RT	19.2	1.50	1.26	94
268RT	24.4	1.52	1.22	91

Comments

Report not IANZ endorsed due to density measurements being outside of NDM range of calibration.
GPS locations recorded by client on site.
Report re-issued with IANZ accreditation removed from report

Report No: ND:BOP18W0088
Issue No: 1

Nuclear Density Report

Client: Jason Mather
JMC Limited
PO Box 16070
Bethlehem

Tauranga 3147
NZ

Project: Zariba

The test results reported herein are not accredited. Contact the laboratory for further information. Samples are tested as received, in natural condition, unless stated otherwise in the comments. This report may only be reproduced in full.



Approved Signatory: Rob Ermens
(Lab Manager)

Date of Issue: 1/02/2018

Testing Details

Site Tested: Retests (1.5m deeper than previous retests)
Tested By: Jake Brown
Date Tested: 25/01/2018
Time Tested: 14:00
Material: Rhyolite Pumice Sand
Specification: Contract Specification
Field Methods: NZS 4407:2015 Test 4.2
Lab Methods: NZS 4402:1986 Test 2.1, NZS 4402:1986 Test 4.1.1 - 1986

Compaction Target Details

Material Sample ID: BOP17S-01212
MDD Method: NZS 4402:1986 Test 4.1.1 - 1986
Max. Dry Density: 1.34 t/m³ @ 27 %
Min. Dry Density (t/m³):
Solid Density Type: Measured

Test Results

Site No	Moisture (%)	Wet Density (t/m ³)	Dry Density (t/m ³)	Relative Compaction (%)
248RT	16.9	1.41	1.21	90
256RT	20.6	1.61	1.34	100

Comments

Report not IANZ endorsed due to density measurements being outside of NDM range of calibration.
GPS locations recorded by client on site.
A colour change was noted in the material on test site 248RT which may indicate the excavation was into original ground.

Report No: ND:BOP18W1355

Nuclear Density Report

Client: Jason Mather JMC Limited PO Box 16070 Bethlehem Tauranga 3147 NZ Project: Zariba	PRELIMINARY REPORT ONLY Date of Issue: 28/11/2018

Testing Details Site Tested: Fill Area - Marbles Pumice Sand Tested By: Rob Ermens Date Tested: 27/11/2018 Time Tested: 13:00 Material: Pumice Sand Specification: No Specification Field Methods: NZS 4407:2015 Test 4.3	Compaction Target Details Material Sample ID: MDD Method: Max. Dry Density: Min. Dry Density (t/m³): Solid Density Type: Assumed

Test Results			
Passes	Moisture (%)	Wet Density (t/m³)	Dry Density (t/m³)
2	26.6	1.54	1.22
6	26.5	1.56	1.23
10	26.4	1.56	1.23
16	24.5	1.60	1.29
22	24.8	1.60	1.28

Comments Plateau 1. Compaction plant was a loaded motor scraper. Report not IANZ endorsed due to density measurements being outside of NDM range of calibration.

Report No: ND:BOP18W1359

Nuclear Density Report

Client: Jason Mather JMC Limited PO Box 16070 Bethlehem Tauranga 3147 NZ Project: Zariba	PRELIMINARY REPORT ONLY Date of Issue: 28/11/2018

Testing Details Site Tested: Fill Area - Paengaroa Rhyolite Pumice Sand Tested By: Jake Brown Date Tested: 27/11/2018 Time Tested: 14:30 Material: Rhyolite Pumice Sand Specification: No Specification Field Methods: NZS 4407:2015 Test 4.3	Compaction Target Details Material Sample ID: MDD Method: Max. Dry Density: Min. Dry Density (t/m³): Solid Density Type: Assumed

Test Results			
Passes	Moisture (%)	Wet Density (t/m³)	Dry Density (t/m³)
5	32.8	1.51	1.13
11	31.9	1.60	1.22
17	33.2	1.63	1.22
25	32.6	1.64	1.23
31	33.5	1.64	1.23
37	31.5	1.65	1.26
43	33.0	1.65	1.24

Comments Plateau 2. Compaction plant was a loaded motor scraper. Report not IANZ endorsed due to density measurements being outside the NDM range of calibration.



D

Lot-by-Lot Hand
Auger Logs

Appendix D

Lot-by-Lot Hand Auger Logs



Our Ref: 1009168.0.0.0/REP01
Customer Ref: Terrace Views Stage 3 D & E
11 February 2019

Aurecon New Zealand Ltd
Ground Level
247 Cameron Road
Tauranga 3110

Attention: Sam Webber

Dear Sam

Terrace Views, Papamoa - Stage 3 D & E: Site Investigation Site Report

Customer's Instructions

We were instructed to:

- Complete Hand Auger (HA) investigations at the locations indicated on the provided plan and marked out by JMC Ltd.
- Carry out Scala Penetrometer (SC) testing 5m towards the front and rear of each section from the indicated Hand Auger location.

Specifications

None issued.

Dates of Procedures

10, 11, 13, 14, December 2018

08, 16, 31 January 2019

01 February 2019

Determining the Test Locations

A location plan illustrating proposed investigation locations was provided to Geotechnics by Aurecon. Hand Auger locations were pegged and marked in the centre of each lot prior to Geotechnics' arrival on site by JMC Ltd. Geotechnics measured five metres towards the front and rear of each lot's centre position to locate the A and B Scala penetrometer test locations respectively.

Location Plans

The attached plan provides indicative locations only and is not to scale.

Methods

NZS 4402:1988 Test 6.5.2 - Determination of the penetration resistance of a soil (Hand method using a dynamic cone penetrometer) - Scala

Material Description

Material descriptions are provided in the attached results. Scala data sheets denote material source as *unknown* and for interpretation and validation by the customer.

Results

Results are attached.

Photos can be downloaded from the following link:

<https://transfer.tonkinandtaylorgroup.com/message/Xc6rmYAByj7UchfgybeCVm>

This link will expire on 5 May 2019 after which we can provide the photos upon request.

Test Remarks

Material Logging

Material logging and descriptions in the field are in general accordance with the New Zealand Geotechnical Society Inc in 'Guideline for the Field Classification of Soil and Rock for Engineering Purposes' (December 2005), excluding geological information and are based on the observational behaviour of the recovered material.

As agreed with the customer, densities recorded in borehole logs are derived from Scala penetrometer test A on each lot. Scala penetrometer test B may alternatively be used for this purpose at the customer's discretion.

The logs represent our best assessment of the sub-surface conditions, but due to the subjective nature of material logging, we take no responsibility for any inaccuracies or misinterpretations.

Scala

The estimated CBR values are based on Figure 5.3, Correlation of Dynamic Cone Penetration and CBR AUSTROADS (2004) "Pavement Design - A Guide to the Structural Design of Road Pavements".

Our standard test procedure is to over-drill Scala penetrometer tests every 1m. As requested by you, we have not over-drilled these tests. This variation to the standard test method may increase friction with depth due to skin friction and the protrusion of the rod connectors. The results >1m deep may therefore not be accurate and are provided for your own interpretation and inference.

For ease of reference, Scala results from test "A" on each lot have been included in the borehole logs even though they were not performed adjacent to the boreholes.

For Lots 458 – 459, 472 – 482 and in Lot 391 A only, Aurecon conducted their own Scala penetrometer testing and this data has been included on applicable hand auger logs as agreed with the customer. These instances have been noted in the comments section of each log. Geotechnics takes no responsibility for data provided by the customer for inclusion in this report.

Hand Auger investigations that indicate the presence of Topsoil/Organic Silt may not have corresponding Scala Penetrometer data for that layer. This is because Scala penetrometer testing occasionally preceded Hand Auger investigations by a significant time interval. Topsoil/Organic Silt was placed on site/s between the testing and investigation activities. Scala data may therefore appear to commence at 0.0mbgl on Scala results pages but at a greater depth on logs.

Lots where this may be observed are: 448 – 450, 452 – 453, 462 – 471.
All other tests commenced at the same level.

General Remarks

This report has been prepared for the benefit of Aurecon New Zealand Ltd, with respect to the particular brief given to us and it cannot be relied upon in other contexts or for any other purpose without our prior review and agreement. The inherent uncertainties of site investigation work, mean the nature and continuity of subsoil away from the test location could vary from the data logged.

We provide the results and logs for your interpretation and inference.

Please reproduce this report in full when transmitting to others or including in internal reports.

If we can be of any further assistance, feel free to get in touch. Contact details are provided at the bottom of the letterhead page.

GEOTECHNICS LTD

Report prepared by:



Troy Robertson
Geotechnician

Authorised for Geotechnics by:

.....

Paul Burton
Project Director

Report checked by:



Ryan Milligan
Development Manager

11-Feb-19

t:\geotechnicsgroup\projects\1009168\workingmaterial\20190211 tasr terrace views stg 3 d&e rep01.docx



 GEOTECHNICS	LOCATION PLAN		Site Investigation						Locations are indicative only	
	15C Amber Crescent Judea Tauranga 3110	Site	Terrace Views - Stage 3 D & E	Our Ref	1009168.0.0.0/REP01	Drawn	TASR	Date	20/12/2018	
		Location	Papamoa	Your Ref	Terrace Views Stage 3 D & E	Checked	RWM	Date	20/12/2018	
		Project	Terrace Views, Papamoa - Stage 3 D & E: Site Investigation	Lab Ref	016/19 – 024/19	Scale	Not to Scale			



HAND AUGER LOG

HOLE ID: **Lot 391**

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI										LOCATION: Terrace View, Papamoa										JOB No.: 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Ground Floor, 247 Cameron Road
PO Box 2292, Tauranga
New Zealand
Tel: +64 7 578 6183
www.aurecongroup.com

Client: **Zariba Properties Limited**
Project: **Terrace Views**
Location: **Terrace Views, Papamoa - Stage 3D/3E**
Project Reference: **247593**

Lot 391

Sheet 1 of 1

DRILLING INFORMATION

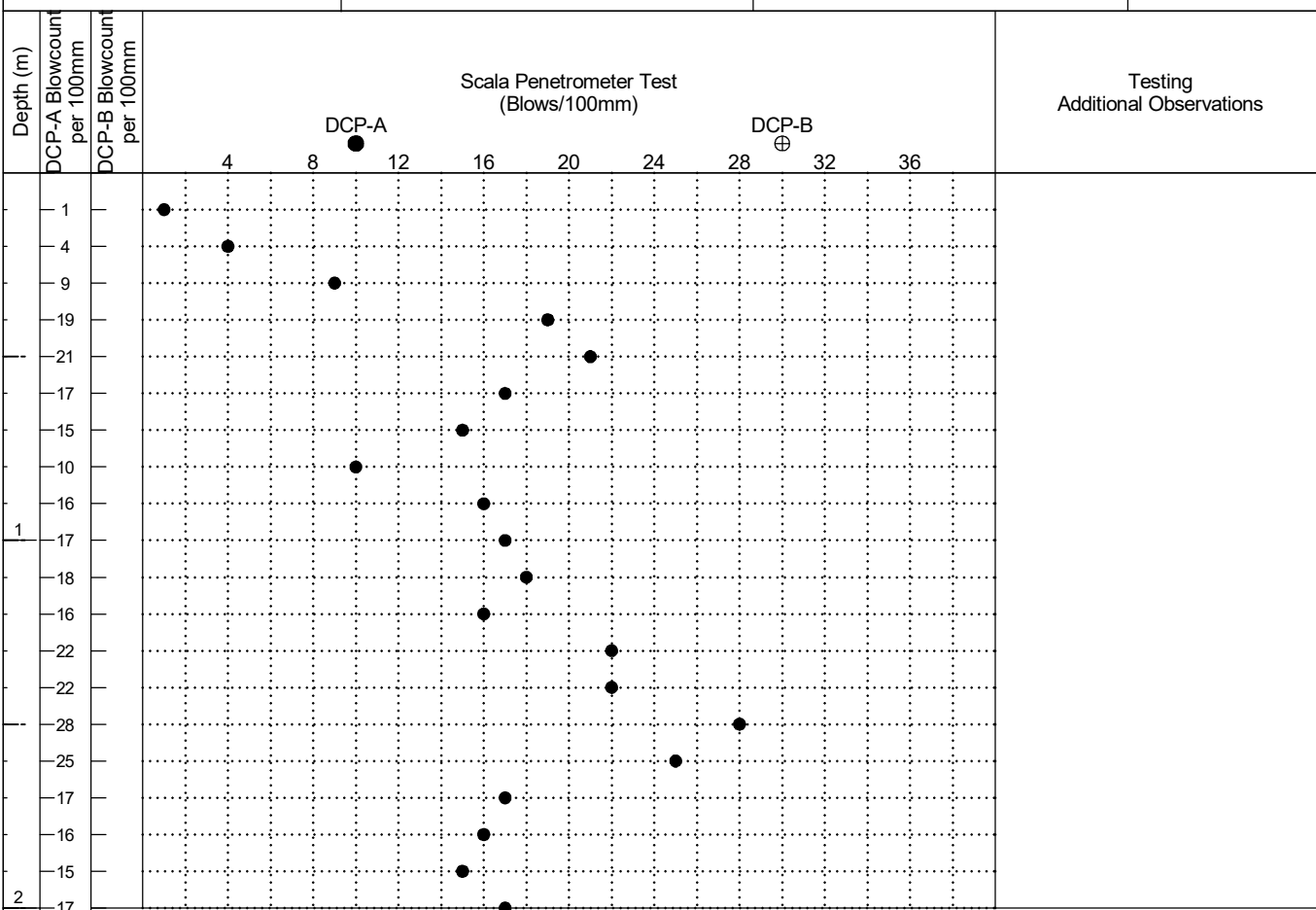
Method: Scala
Equipment: Scala
Contractor: Aurecon

CO-ORDINATES: N/A

Easting: N/A
Northing: N/A
Ground level: N/A

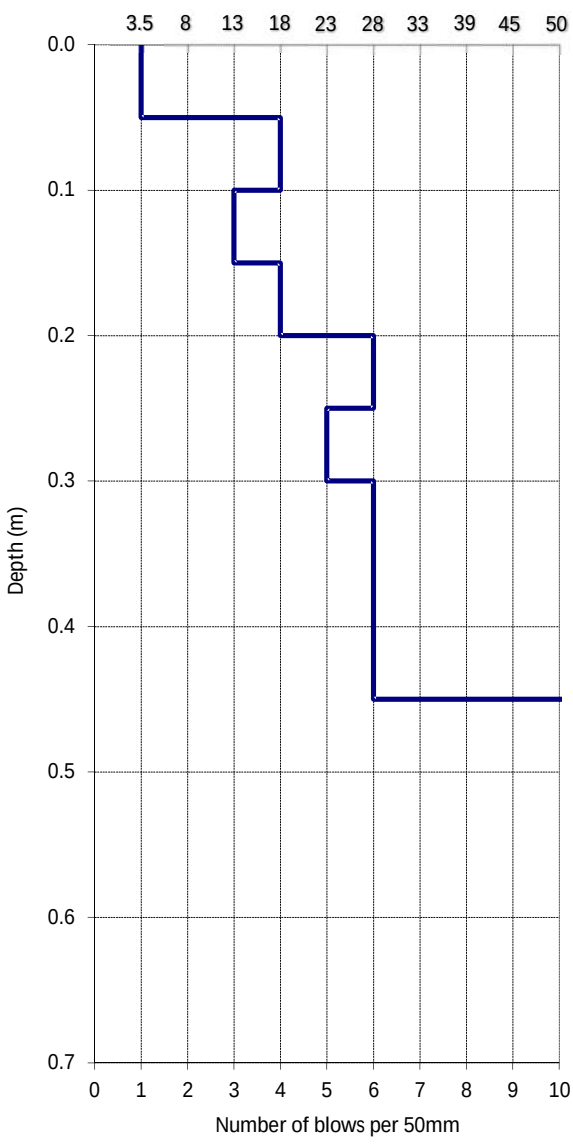
Date started: 19/12/2018
Date completed: 19/12/2018
Inclination: 90°
Orientation: N/A

Logged by: AY
Input by: AY
Checked by: MB
Verified by: RG



Remarks:
*Note: Scalas were offset from the Hand Auger conducted in the centre of the lot by ~5 metres on either side

Water Level Readings:
Date Time | Hole Depth | Water Level
No water level recorded

 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
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HAND AUGER LOG

HOLE Id: Lot 392

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI										LOCATION: Terrace View, Papamoa										JOB No.: 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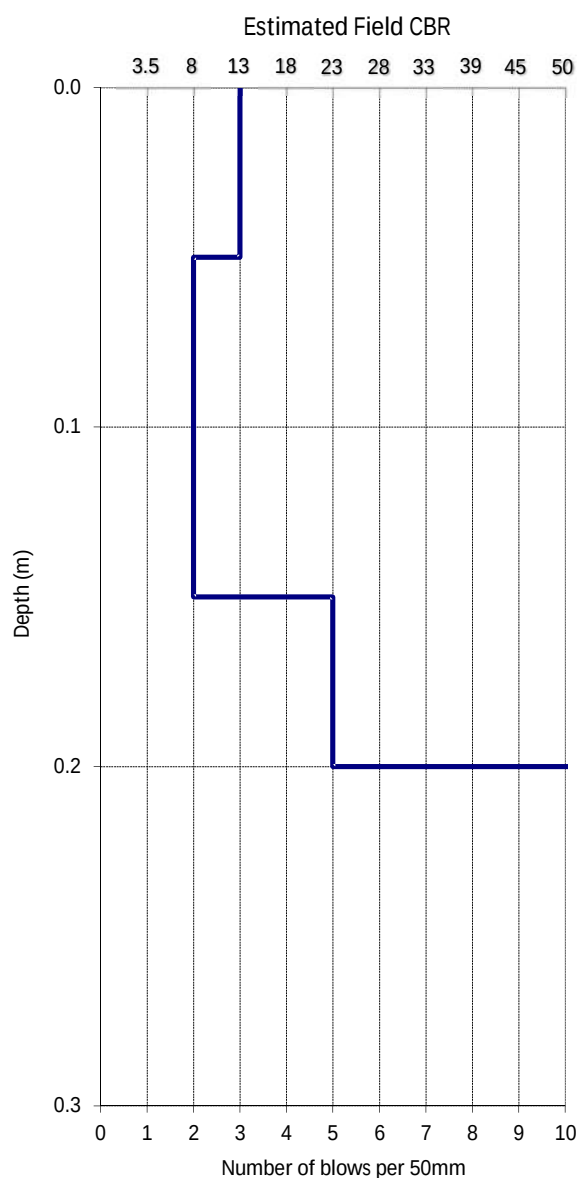
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Judea
Tauranga 3110
New Zealand
p. +64 7 571 0280

Lab Ref/URN

016/19

Project Name	TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID	1009168.0.0.0
Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851
Site Location	Papamoa		Material Source	Unknown
Material Description	See Lot 392 borehole log.		Test Series	Terrace Views/010219/SC391-400,439-443,483-487
Depth from ground surface to commencement of penetration (m)		0.0	Test Number	Lot 392 A

Chainage		Offset		Lane	
Vertical distance driven (mm)	Depth (mm)	Number of blows	Vertical distance driven (mm)	Depth (mm)	Number of blows
50	50	3	1050	1050	
100	100	2	1100	1100	
150	150	2	1150	1150	
200	200	5	1200	1200	
250	250	16	1250	1250	
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350	350		1350	1350	
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750	750		1750	1750	
800	800		1800	1800	
850	850		1850	1850	
900	900		1900	1900	
950	950		1950	1950	
1000	1000		2000	2000	



Test Remarks

Tested By	PIFA	Date	1/02/2019
Data Entry By	TASR	Date	1/02/2019
Checked By	RWM	Date	7/02/2019

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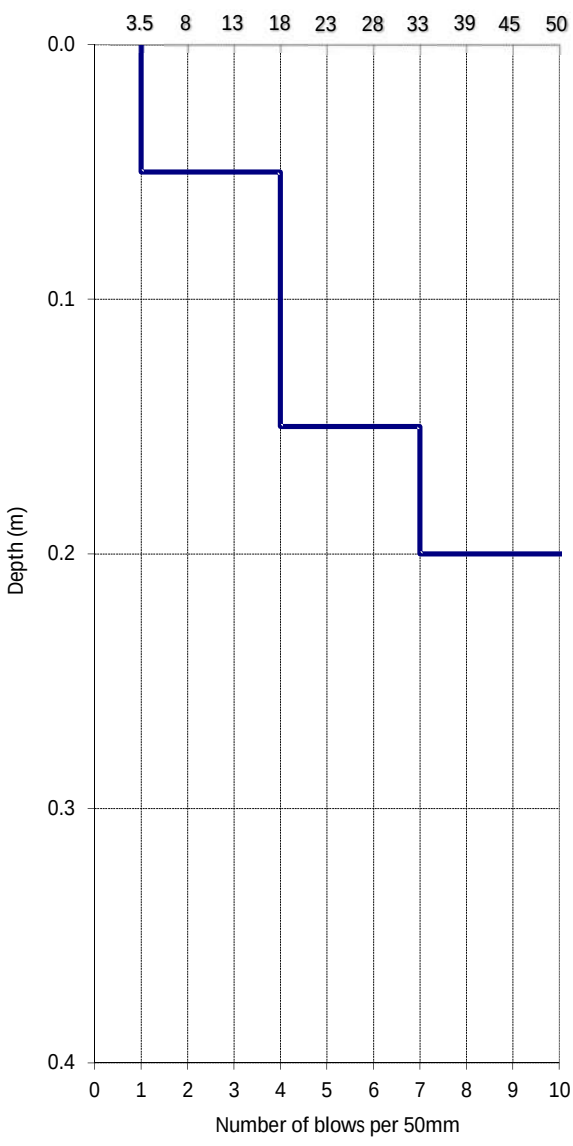
HAND AUGER LOG

HOLE Id: Lot 393

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI					LOCATION: Terrace View, Papamoa					JOB No.: 1009168.0000									
CO-ORDINATES: (NZTM2000)					DRILL TYPE: 50mm Hand Auger					HOLE STARTED: 10/12/2018 HOLE FINISHED: 10/12/2018 DRILLED BY: GEOTECHNICS LOGGED BY: PIFA CHECKED: RWM									
R.L.:					DRILL METHOD: HA														
DATUM:																			
GEOLOGICAL										ENGINEERING DESCRIPTION									
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.										Description and Additional Observations									
WATER CORE RECOVERY (%) METHOD SCALA PENETROMETER (Blows/50mm) 1 2 3 4 5 6 7 8 9										TESTS SAMPLES RL (m) DEPTH (m) GRAPHIC LOG MOISTURE / WEATHERING CONDITION STRENGTH / CLASSIFICATION SHEAR STRENGTH (kPa)									
1 4 4 7 12 >> 14 >> 16 >>										TS TS M VD M-W 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150 160 170 180 190 200									
DRY 10/12/2018 100 HA										Organic SILT, with some sand; dark brownish black. Stiff, dry to moist, non-plastic; sand, fine to coarse. Fine to coarse SAND, with trace silt and gravel; light orange brown. Very dense, moist, well graded; gravel, fine to medium, subangular to subrounded, pumiceous. 1.0m: Changes to orange brown; moist to wet. 1.25m: Changes to brown. 1.5m: Changes to greyish brown. End of borehole at 2.00mbgl. No standing groundwater encountered.									
COMMENTS:																			
Hole Depth 2m																			

 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
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HAND AUGER LOG

HOLE Id: Lot 394

Hole Location: See Location Plan

SHEET: 1 OF 1

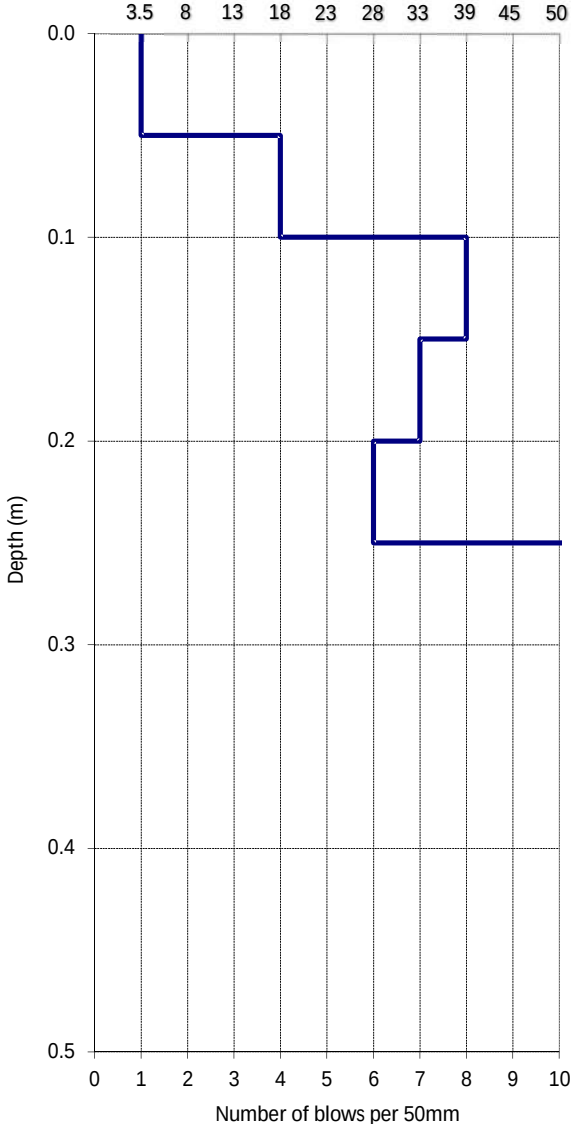
PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI						LOCATION: Terrace View, Papamoa				JOB No.: 1009168.0000							
CO-ORDINATES: (NZTM2000)						DRILL TYPE: 50mm Hand Auger				HOLE STARTED: 10/12/2018 HOLE FINISHED: 10/12/2018 DRILLED BY: GEOTECHNICS LOGGED BY: TASR CHECKED: RWM							
R.L.:						DRILL METHOD: HA											
DATUM:																	
GEOLOGICAL							ENGINEERING DESCRIPTION										
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.		WATER	CORE RECOVERY (%)	METHOD	SCALA PENETROMETER (Blows/50mm) 123456789				TESTS	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	MOISTURE / WEATHERING CONDITION	STRENGTH / PLASTICITY CLASSIFICATION	SHEAR STRENGTH (kPa) 102030405060708090100	Description and Additional Observations
			100	HA	1 4 8 7 6 11 >> 14 >> 15 >>								TS TS TS TS	D-M	St		Organic SILT, with some sand; dark brownish black. Stiff, dry to moist, non-plastic; sand, fine to coarse.
												0.5					Fine to coarse SAND, with minor gravel, trace silt; light pinkish brown. Very dense, moist to wet, well graded; gravel, fine, subangular to subrounded, pumiceous.
												1.0					0.9m: Changes to greyish brown.
												1.5					
												2.0					End of borehole at 2.00mbgl. No standing groundwater encountered.
COMMENTS:																	
Hole Depth 2m																	

 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1	
			Lab Ref/URN	
			016/19	

NZS 4402: 1998 Test 6.5.2 Dynamic Cone Penetrometer - Scala									
Project Name		TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID		1009168.0.0.0			
Customer Project ID		Terrace Views Stage 3 D & E		Equipment ID		TG0851			
Site Location		Papamoa		Material Source		Unknown			
Material Description		See Lot 394 borehole log.		Test Series		Terrace Views/010219/SC391-400,439-443,483-487			
Depth from ground surface to commencement of penetration (m)			0.0		Test Number			Lot 394 A	

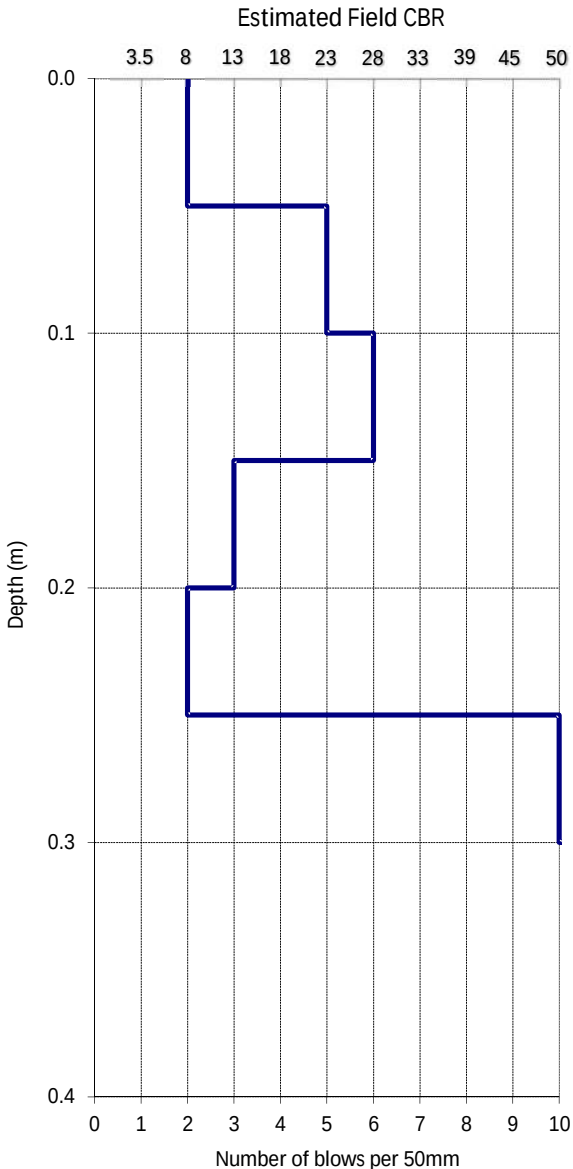
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Vertical distance driven (mm)	Depth (mm)	Number of blows	Vertical distance driven (mm)	Depth (mm)	Number of blows
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200	200	7	1200	1200	
250	250	6	1250	1250	
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350	350	14	1350	1350	
400	400	15	1400	1400	
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600	600		1600	1600	
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850	850		1850	1850	
900	900		1900	1900	
950	950		1950	1950	
1000	1000		2000	2000	

Estimated Field CBR



Test Remarks					

Tested By	PIFA	Date	1/02/2019
Data Entry By	TASR	Date	1/02/2019
Checked By	RWM	Date	7/02/2019

 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
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HAND AUGER LOG

HOLE Id: Lot 395

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI					LOCATION: Terrace View, Papamoa					JOB No.: 1009168.0000									
CO-ORDINATES: (NZTM2000)					DRILL TYPE: 50mm Hand Auger					HOLE STARTED: 10/12/2018 HOLE FINISHED: 10/12/2018 DRILLED BY: GEOTECHNICS LOGGED BY: EWDA CHECKED: RWM									
R.L.:					DRILL METHOD: HA														
DATUM:																			
GEOLOGICAL										ENGINEERING DESCRIPTION									
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.										Description and Additional Observations									
WATER CORE RECOVERY (%) METHOD SCALA PENETROMETER (Blows/50mm) 1 2 3 4 5 6 7 8 9										TESTS SAMPLES RL (m) DEPTH (m) GRAPHIC LOG MOISTURE / WEATHERING CONDITION STRENGTH / PLASTICITY CLASSIFICATION 10 20 30 40 50 60 70 80 90 100									
100 HA 2 4 5 5 5 10 12 >> 13 >>										TS D-M St VD M M-W 0.5 1.0 1.5 2.0 End of borehole at 2.00mbgl. No standing groundwater encountered.									
COMMENTS:																			
Hole Depth 2m																			

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HAND AUGER LOG

HOLE Id: Lot 396

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI				LOCATION: Terrace View, Papamoa				JOB No.: 1009168.0000							
CO-ORDINATES: (NZTM2000)				DRILL TYPE: 50mm Hand Auger				HOLE STARTED: 10/12/2018							
R.L.:				DRILL METHOD: HA				HOLE FINISHED: 10/12/2018							
DATUM:								DRILLED BY: GEOTECHNICS							
								LOGGED BY: PIFA							
								CHECKED: RWM							
GEOLOGICAL								ENGINEERING DESCRIPTION							
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION:								WATER CORE RECOVERY (%) METHOD SCALA PENETROMETER (Blows/50mm) 1 2 3 4 5 6 7 8 9 TESTS SAMPLES RL (m) DEPTH (m) GRAPHIC LOG MOISTURE CONDITION STRENGTH/PLASTICITY CLASSIFICATION SHEAR STRENGTH (kPa) 10 20 30 40 50 60 70 80 90 100 Description and Additional Observations							
								1.5m TS D-M St 10 20 30 40 50 60 70 80 90 100 Organic SILT, with some sand, trace gravel; dark brownish black. Stiff, dry to moist, non-plastic; sand, fine to coarse; gravel, fine to medium, subangular to subrounded, pumiceous. Fine to coarse SAND, with minor gravel, trace silt; light orange brown. Dense, moist, well graded; gravel, fine to medium, subangular to subrounded, pumiceous. 0.25m: Changes to very dense. 0.8m: Change to moist to wet. 1.0m: Change to orange brown. 1.6m: Changes to greyish brown; wet. End of borehole at 2.00mbgl. No standing groundwater encountered.							
COMMENTS:															
Hole Depth 2m															

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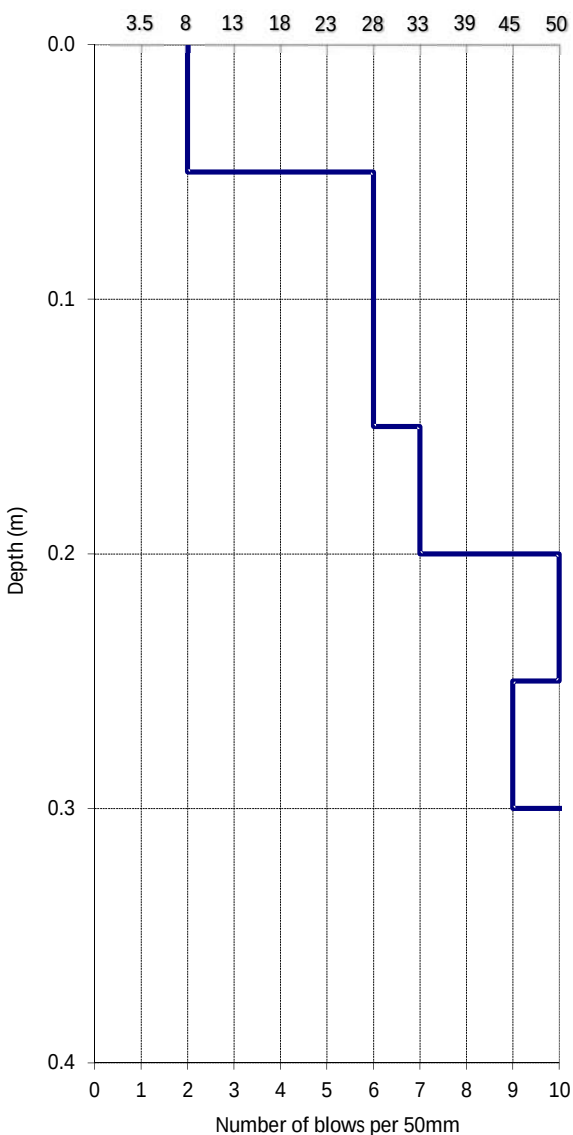
HAND AUGER LOG

HOLE Id: Lot 397

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI					LOCATION: Terrace View, Papamoa					JOB No.: 1009168.0000									
CO-ORDINATES: (NZTM2000)					DRILL TYPE: 50mm Hand Auger					HOLE STARTED: 10/12/2018 HOLE FINISHED: 10/12/2018 DRILLED BY: GEOTECHNICS LOGGED BY: DBRU CHECKED: RWM									
R.L.:					DRILL METHOD: HA														
DATUM:																			
GEOLOGICAL										ENGINEERING DESCRIPTION									
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.										Description and Additional Observations									
WATER CORE RECOVERY (%) METHOD SCALA PENETROMETER (Blows/50mm) 1 2 3 4 5 6 7 8 9										TESTS SAMPLES RL (m) DEPTH (m) GRAPHIC LOG MOISTURE / WEATHERING CONDITION STRENGTH / DENSITY CLASSIFICATION 10 20 30 40 50 60 70 80 90 100									
DRY 13/12/2018 100 HA 2 6 6 7 10 9 16 >>										TS TS M VD W M W 0.5 1.0 1.5 2.0 Organic SILT, with some sand; dark brownish black. Stiff, dry to moist, non-plastic; sand, fine to coarse. 0.7m: Changes to brown; wet. 1.2m: Changes to orange brown; moist. 1.6m: Changes to wet. End of borehole at 2.00mbgl. No standing groundwater encountered.									
COMMENTS:																			
Hole Depth 2m																			

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HAND AUGER LOG

HOLE Id: Lot 398

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI										LOCATION: Terrace View, Papamoa										JOB No.: 1009168.0000									
CO-ORDINATES: (NZTM2000)										DRILL TYPE: Window Sampler										HOLE STARTED: 10/12/2018 HOLE FINISHED: 10/12/2018 DRILLED BY: GEOTECHNICS LOGGED BY: EWDA CHECKED: RWM									
R.L.:										DRILL METHOD: PR																			
DATUM:																													
GEOLOGICAL															ENGINEERING DESCRIPTION														
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.															Description and Additional Observations														
WATER															CORE RECOVERY (%)														
METHOD															SCALA PENETROMETER (Blows/50mm)														
1															2														
1															2														
2															2														
4															7														
13 >>															13 >>														
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DRY 13/12/2018																													
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Hole Depth 2m																													

 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
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NZS 4402: 1998 Test 6.5.2 Dynamic Cone Penetrometer - Scala																																																																																																																																								
Project Name	TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID	1009168.0.0.0																																																																																																																																				
Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851																																																																																																																																				
Site Location	Papamoa		Material Source	Unknown																																																																																																																																				
Material Description	See Lot 398 borehole log.		Test Series	Terrace Views/010219/SC391-400,439-443,483-487																																																																																																																																				
Depth from ground surface to commencement of penetration (m)		0.0	Test Number	Lot 398 A																																																																																																																																				
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HAND AUGER LOG

HOLE Id: Lot 399

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI										LOCATION: Terrace View, Papamoa										JOB No.: 1009168.0000									
CO-ORDINATES: (NZTM2000)										DRILL TYPE: Window Sampler										HOLE STARTED: 10/12/2018 HOLE FINISHED: 10/12/2018 DRILLED BY: GEOTECHNICS LOGGED BY: EWDA CHECKED: RWM									
R.L.:										DRILL METHOD: PR																			
DATUM:																													
GEOLOGICAL															ENGINEERING DESCRIPTION														
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.															Description and Additional Observations														
WATER															CORE RECOVERY (%)														
METHOD															SCALA PENETROMETER (Blows/50mm)														
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Project Name	TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID	1009168.0.0.0																																																																																																																																				
Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851																																																																																																																																				
Site Location	Papamoa		Material Source	Unknown																																																																																																																																				
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HAND AUGER LOG

HOLE Id: Lot 400

Hole Location: See Location Plan

SHEET: 1 OF 1

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Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851																																																																																																																																				
Site Location	Papamoa		Material Source	Unknown																																																																																																																																				
Material Description	See Lot 400 borehole log.		Test Series	Terrace Views/010219/SC391-400,439-443,483-487																																																																																																																																				
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HAND AUGER LOG

HOLE Id: Lot 439

Hole Location: See Location Plan

SHEET: 1 OF 1

[illegible]

 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
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Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851																																																																																																																																				
Site Location	Papamoa		Material Source	Unknown																																																																																																																																				
Material Description	See Lot 439 borehole log.		Test Series	Terrace Views/010219/SC391-400,439-443,483-487																																																																																																																																				
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Material Description	See Lot 439 borehole log.		Test Series	Terrace Views/010219/SC391-400,439-443,483-487																																																																																																																																				
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Checked By	RWM	Date	7/02/2019																																																																																																																																					



HAND AUGER LOG

HOLE Id: **Lot 440**

Hole Location: See Location Plan

SHEET: 1 OF 1

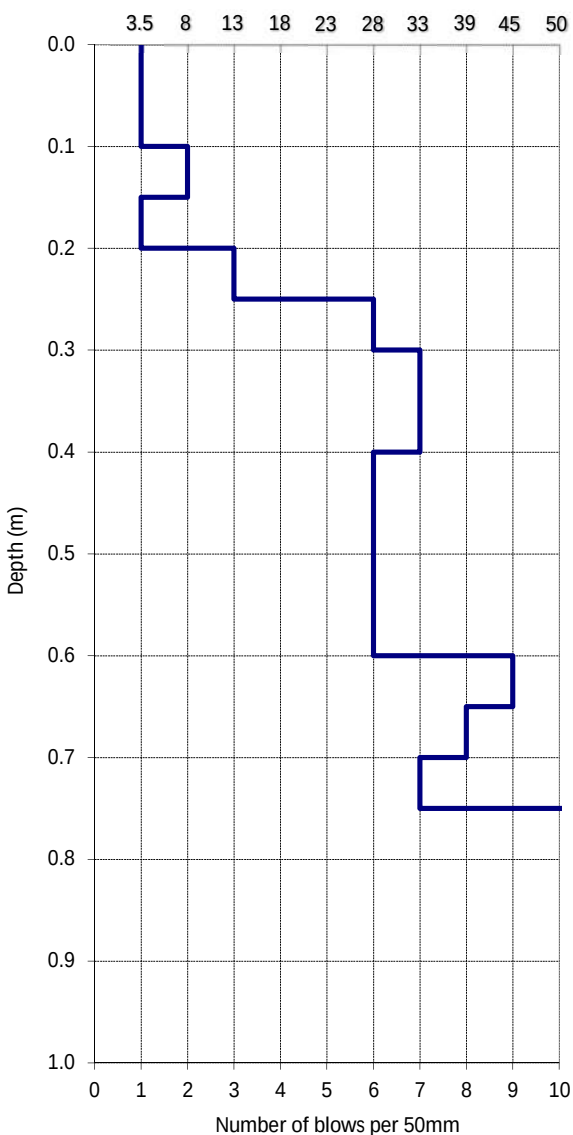
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 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1	
			Lab Ref/URN	
			016/19	

NZS 4402: 1998 Test 6.5.2 Dynamic Cone Penetrometer - Scala								
Project Name		TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID		1009168.0.0.0		
Customer Project ID		Terrace Views Stage 3 D & E		Equipment ID		TG0851		
Site Location		Papamoa		Material Source		Unknown		
Material Description		See Lot 440 borehole log.		Test Series		Terrace Views/010219/SC391-400,439-443,483-487		
Depth from ground surface to commencement of penetration (m)			0.0		Test Number		Lot 440 A	

Chainage		Offset		Lane	
Vertical distance driven (mm)	Depth (mm)	Number of blows	Vertical distance driven (mm)	Depth (mm)	Number of blows
50	50	1	1050	1050	
100	100	1	1100	1100	
150	150	2	1150	1150	
200	200	1	1200	1200	
250	250	3	1250	1250	
300	300	6	1300	1300	
350	350	7	1350	1350	
400	400	7	1400	1400	
450	450	6	1450	1450	
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550	550	6	1550	1550	
600	600	6	1600	1600	
650	650	9	1650	1650	
700	700	8	1700	1700	
750	750	7	1750	1750	
800	800	11	1800	1800	
850	850	11	1850	1850	
900	900	12	1900	1900	
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1000	1000		2000	2000	

Estimated Field CBR



Test Remarks					

Tested By	PIFA	Date	1/02/2019
Data Entry By	TASR	Date	1/02/2019
Checked By	RWM	Date	7/02/2019

 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
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HAND AUGER LOG

HOLE Id: Lot 441

Hole Location: See Location Plan

SHEET: 1 OF 1

[illegible]

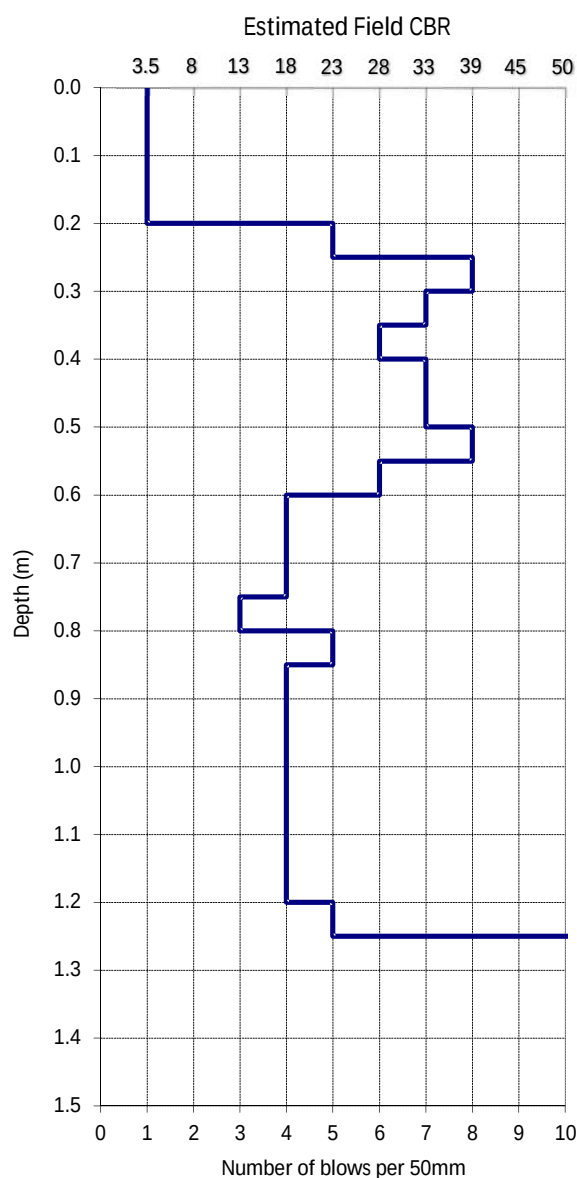
15C Amber Crescent
Judea
Tauranga 3110
New Zealand
p. +64 7 571 0280

Lab Ref/URN

016/19

Project Name	TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID	1009168.0.0.0
Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851
Site Location	Papamoa		Material Source	Unknown
Material Description	See Lot 441 borehole log.		Test Series	Terrace Views/010219/SC391-400,439-443,483-487
Depth from ground surface to commencement of penetration (m)		0.0	Test Number	Lot 441 A

Chainage		Offset		Lane	
Vertical distance driven (mm)	Depth (mm)	Number of blows	Vertical distance driven (mm)	Depth (mm)	Number of blows
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200	200	1	1200	1200	4
250	250	5	1250	1250	5
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350	350	7	1350	1350	14
400	400	6	1400	1400	13
450	450	7	1450	1450	
500	500	7	1500	1500	
550	550	8	1550	1550	
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850	850	5	1850	1850	
900	900	4	1900	1900	
950	950	4	1950	1950	
1000	1000	4	2000	2000	



Test Remarks

Tested By	PIFA	Date	1/02/2019
Data Entry By	TASR	Date	1/02/2019
Checked By	RWM	Date	7/02/2019

 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
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HAND AUGER LOG

HOLE ID: **Lot 442**

Hole Location: See Location Plan

SHEET: 1 OF 1

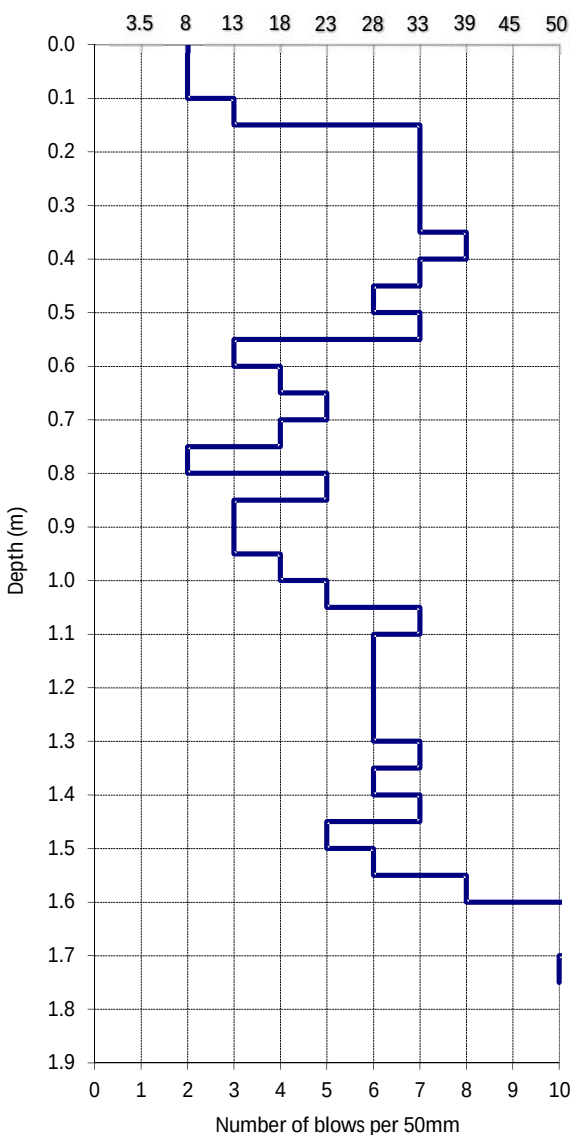
PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI										LOCATION: Terrace View, Papamoa										JOB No.: 1009168.0000									
CO-ORDINATES: (NZTM2000)										DRILL TYPE: 50mm Hand Auger										HOLE STARTED: 08/01/2019									
R.L.:										DRILL METHOD: HA										HOLE FINISHED: 08/01/2019									
DATUM:										LOGGED BY: PIFA										CHECKED: RWM									
GEOLOGICAL										ENGINEERING DESCRIPTION																			
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION										WATER CORE RECOVERY (%) METHOD SCALA PENETROMETER (Blows/50mm) TESTS SAMPLES RL (m) DEPTH (m) GRAPHIC LOG MOISTURE CONDITION WEATHERING STRENGTH/DENSITY CLASSIFICATION SHEAR STRENGTH (kPa)										Description and Additional Observations									
										100 HA										Organic SILT, with some sand; dark brownish black. stiff, moist to wet, non-plastic; sand, fine to medium.									
										0.5										Fine to coarse SAND, with minor gravel, trace silt, light yellowish brown. Dense, moist, well graded; gravel, fine, subrounded to subangular, pumiceous.									
										1.0										1.0m: Changes to dense.									
										1.5										1.4m: Changes to brown; wet; gravel, fine to medium.									
										2.0										1.6m: Changes to very dense.									
																				End of borehole at 2.00 mbgl.									
																				No standing groundwater encountered.									

COMMENTS:

 Hole Depth
 2m

Scale 1:13

Rev.: A

 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
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NZS 4402: 1998 Test 6.5.2 Dynamic Cone Penetrometer - Scala																																																																																																																																								
Project Name	TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID	1009168.0.0.0																																																																																																																																				
Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851																																																																																																																																				
Site Location	Papamoa		Material Source	Unknown																																																																																																																																				
Material Description	See Lot 442 borehole log.		Test Series	Terrace Views/010219/SC391-400,439-443,483-487																																																																																																																																				
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HAND AUGER LOG

HOLE Id: **Lot 443**

Hole Location: See Location Plan

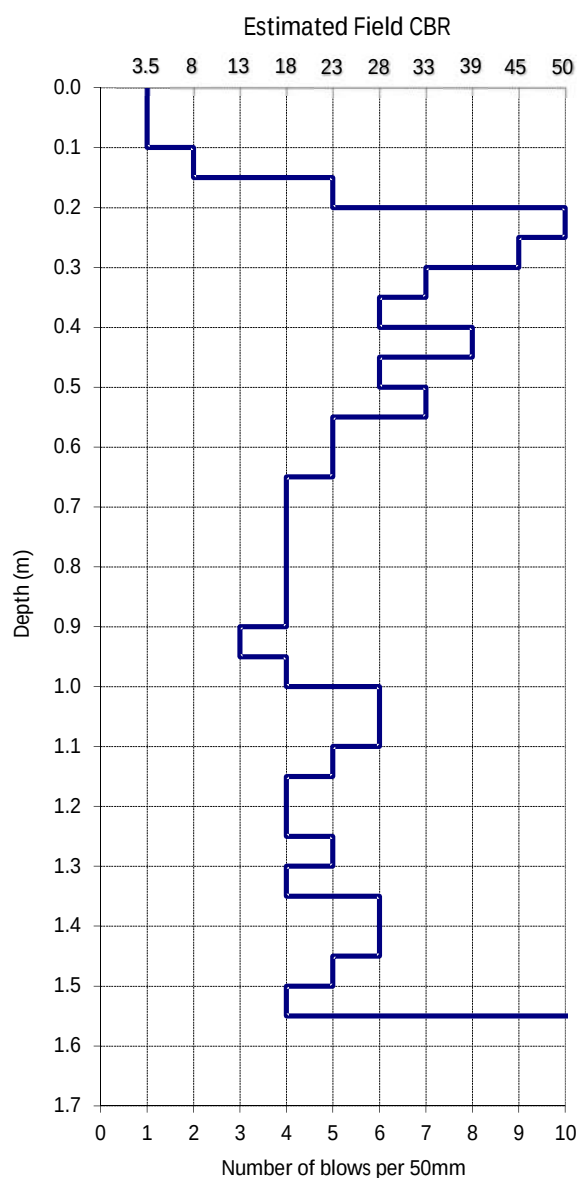
SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI					LOCATION: Terrace View, Papamoa					JOB No.: 1009168.0000									
CO-ORDINATES: (NZTM2000)					DRILL TYPE: 50mm Hand Auger					HOLE STARTED: 08/01/2019 HOLE FINISHED: 08/01/2019									
R.L.:					DRILL METHOD: HA					DRILLED BY: GEOTECHNICS									
DATUM:										LOGGED BY: PIFA		CHECKED: RWM							
GEOLOGICAL										ENGINEERING DESCRIPTION									
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.										Description and Additional Observations									
WATER CORE RECOVERY (%) METHOD SCALA PENETROMETER (Blows/50mm) 123456789										TESTS SAMPLES RL (m) DEPTH (m) GRAPHIC LOG MOISTURE / WEATHERING CONDITION STRENGTH/DENSITY CLASSIFICATION SHEAR STRENGTH (kPa)									
1 1 2 5 10 9 7 6 8 6 7 5 5 4 4 4 4 3 4 6 6 5 4 4 5 4 6 6 5 4 16 >>										TS TS M-W St M D W VD Organic SILT, with some sand; dark brownish black. Stiff, moist to wet, non-plastic; sand, fine to medium. Fine to coarse SAND, with minor silt and gravel; light greyish brown. Dense, moist, well graded; gravel, fine, subrounded to subangular, pumiceous. 1.4m: Changes to dark greyish brown; wet. 1.55m: Changes to very dense. 1.95m: Changes to pinkish brown. End of borehole at 2.0 mbgl. No standing groundwater encountered.									
COMMENTS:																			
Hole Depth 2m																			

016/19

Project Name	TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID	1009168.0.0.0
Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851
Site Location	Papamoa		Material Source	Unknown
Material Description	See Lot 443 borehole log.		Test Series	Terrace Views/010219/SC391-400,439-443,483-487
Depth from ground surface to commencement of penetration (m)		0.0	Test Number	Lot 443 A

Chainage		Offset		Lane	
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350	350	7	1350	1350	4
400	400	6	1400	1400	6
450	450	8	1450	1450	6
500	500	6	1500	1500	5
550	550	7	1550	1550	4
600	600	5	1600	1600	16
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800	800	4	1800	1800	
850	850	4	1850	1850	
900	900	4	1900	1900	
950	950	3	1950	1950	
1000	1000	4	2000	2000	



Tested By	PIFA	Date	1/02/2019
Data Entry By	TASR	Date	1/02/2019
Checked By	RWM	Date	7/02/2019

 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
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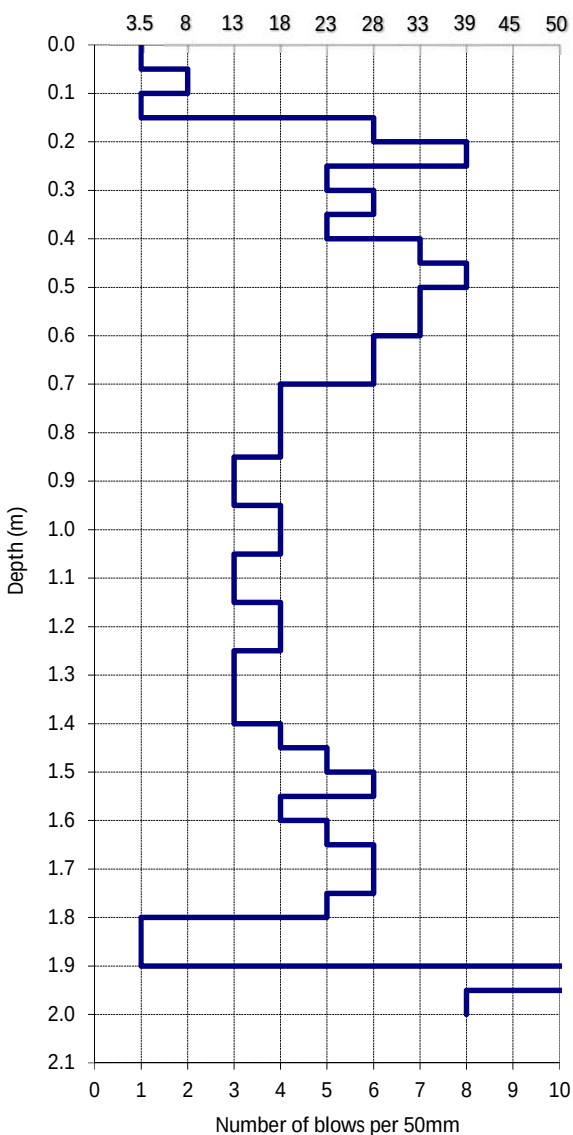
HAND AUGER LOG

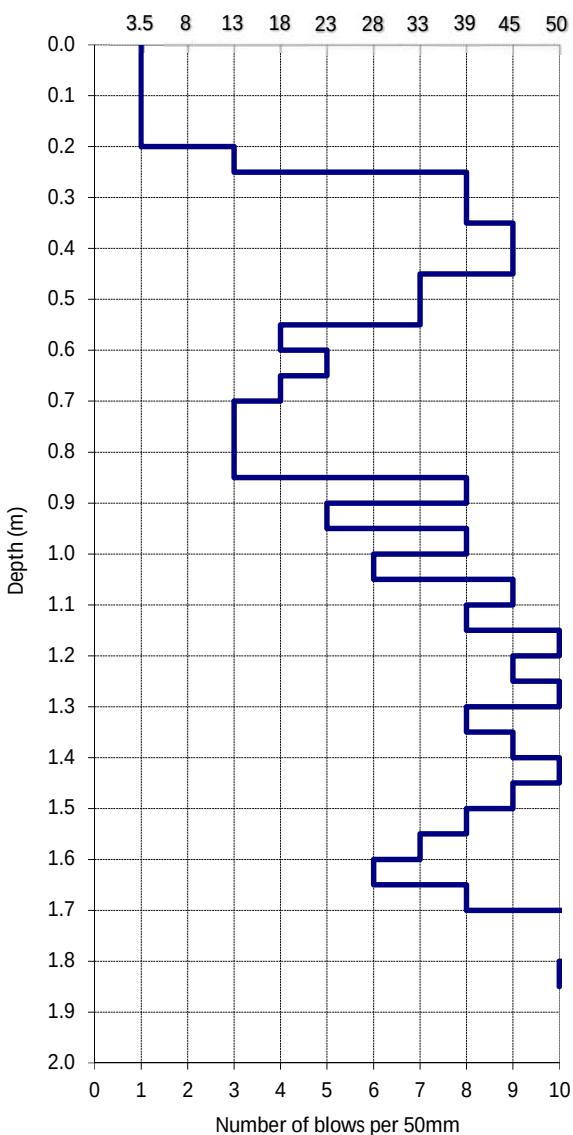
HOLE Id: **Lot 444**

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI										LOCATION: Terrace View, Papamoa										JOB No.: 1009168.0000																			
CO-ORDINATES: (NZTM2000)										DRILL TYPE: 50mm Hand Auger										HOLE STARTED: 08/01/2019																			
R.L.:										DRILL METHOD: HA										HOLE FINISHED: 08/01/2019																			
DATUM:																				DRILLED BY: GEOTECHNICS																			
																				LOGGED BY: DBRU										CHECKED: RWM									
GEOLOGICAL																				ENGINEERING DESCRIPTION																			
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.																				Description and Additional Observations																			
WATER																				MOISTURE / WEATHERING CONDITION																			
CORE RECOVERY (%)																				STRENGTH/CASUALTY CLASSIFICATION																			
METHOD																				SHEAR STRENGTH (kPa)																			
SCALA PENETROMETER (Blows/50mm)																																							
TESTS																																							
SAMPLES																																							
RL (m)																																							
DEPTH (m)																																							
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DRY 08/01/2019																																							
																				End of borehole at 2.00 mbgl.																			
																				No standing groundwater encountered.																			
COMMENTS:																																							
Hole Depth 2m																																							

 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
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NZS 4402: 1998 Test 6.5.2 Dynamic Cone Penetrometer - Scala																																																																																																																																								
Project Name	TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID	1009168.0.0.0																																																																																																																																				
Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851																																																																																																																																				
Site Location	Papamoa		Material Source	Unknown																																																																																																																																				
Material Description	See Lot 444 borehole log.		Test Series	Terrace Views/091218/SC444-446																																																																																																																																				
Depth from ground surface to commencement of penetration (m)		0.0	Test Number	Lot 444 A																																																																																																																																				
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Data Entry By	TASR		Date	1/02/2019																																																																																																																																				
Checked By	RWM		Date	7/02/2019																																																																																																																																				

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HAND AUGER LOG

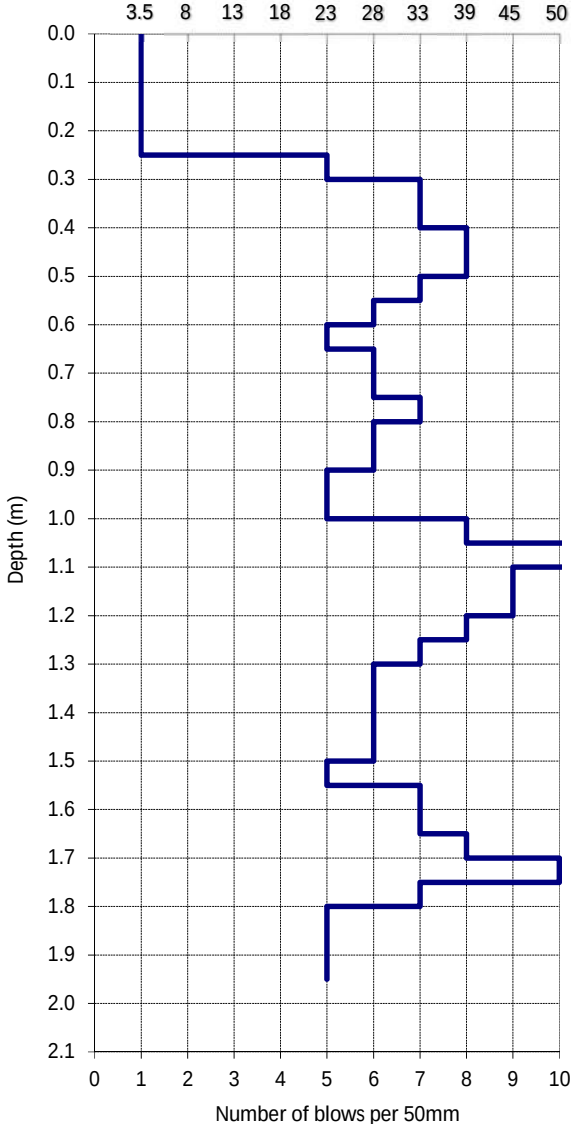
HOLE Id: **Lot 445**

Hole Location: See Location Plan

SHEET: 1 OF 1

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HAND AUGER LOG

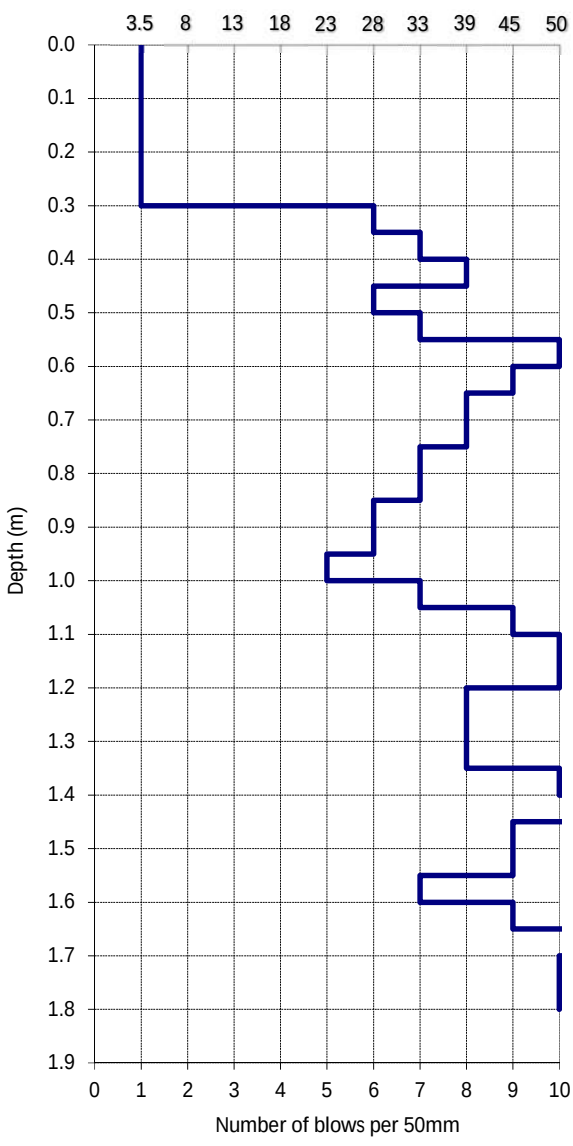
HOLE Id: **Lot 446**

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI										LOCATION: Terrace View, Papamoa										JOB No.: 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HAND AUGER LOG

HOLE Id: Lot 447

Hole Location: See Location Plan

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[illegible]

 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
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Project Name	TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID	1009168.0.0.0																																																																																																																																				
Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851																																																																																																																																				
Site Location	Papamoa		Material Source	Unknown																																																																																																																																				
Material Description	See Lot 448 borehole log.		Test Series	Terrace Views/111218/SC447-448,451-453,464-466,468-470																																																																																																																																				
Depth from ground surface to commencement of penetration (m)			Test Number	Lot 447 A																																																																																																																																				
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HAND AUGER LOG

HOLE ID: **Lot 448**

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI										LOCATION: Terrace View, Papamoa										JOB No.: 1009168.0000																			
CO-ORDINATES: (NZTM2000)										DRILL TYPE: Window Sampler										HOLE STARTED: 08/01/2019																			
R.L.:										DRILL METHOD: PR										HOLE FINISHED: 08/01/2019																			
DATUM:										LOGGED BY: EWDA										CHECKED: RWM																			
GEOLOGICAL										ENGINEERING DESCRIPTION																													
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION										WATER CORE RECOVERY (%) METHOD SCALA PENETROMETER (Blows/50mm) TESTS SAMPLES RL (m) DEPTH (m) GRAPHIC LOG MOISTURE CONDITION WEATHERING STRENGTH/DENSITY CLASSIFICATION SHEAR STRENGTH (kPa)										Description and Additional Observations																			
DRY 13/12/2018 100 WS										1 2 3 4 5 6 7 8 9 3 7 9 9 10 9 9 10 10 6 5 7 7 6 5 11 >> 11 >> 11 >>										TS TS TS TS M-W St 0.5 M VD 1.0 D 1.5 W 2.0										Organic SILT, with some sand; dark brownish black. Stiff, moist to wet, low plasticity; sand, fine to coarse. Fine to coarse SAND, with trace silt and gravel; light orange brown with trace pink mottling. Very dense, moist, well graded; gravel, fine to medium, subangular to subrounded, pumiceous. 0.85m: Changes to dense. 1.15m: Changes to very dense. 1.4m: Pink mottling absent. 1.6m: Changes to wet.									
																														End of borehole at 2.00mbgl.									
																														No standing groundwater encountered.									

COMMENTS:

 Hole Depth
 2m

Scale 1:13

Rev.: A

 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
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NZS 4402: 1998 Test 6.5.2 Dynamic Cone Penetrometer - Scala																																																																																																																																								
Project Name	TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID	1009168.0.0.0																																																																																																																																				
Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851																																																																																																																																				
Site Location	Papamoa		Material Source	Unknown																																																																																																																																				
Material Description	See Lot 448 borehole log.		Test Series	Terrace Views/111218/SC447-448,451-453,464-466,468-470																																																																																																																																				
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HAND AUGER LOG

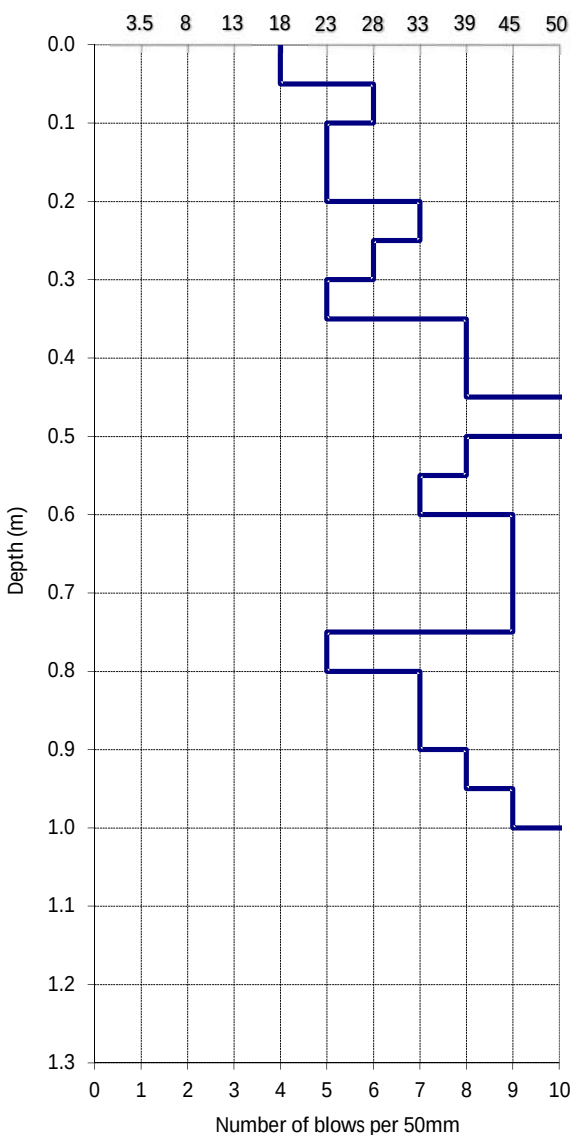
HOLE Id: Lot 449

Hole Location: See Location Plan

SHEET: 1 OF 1

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 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
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Material Description	See Lot 449 borehole log.		Test Series	Terrace Views/131218/SC449-450																																																																																																																																				
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HAND AUGER LOG

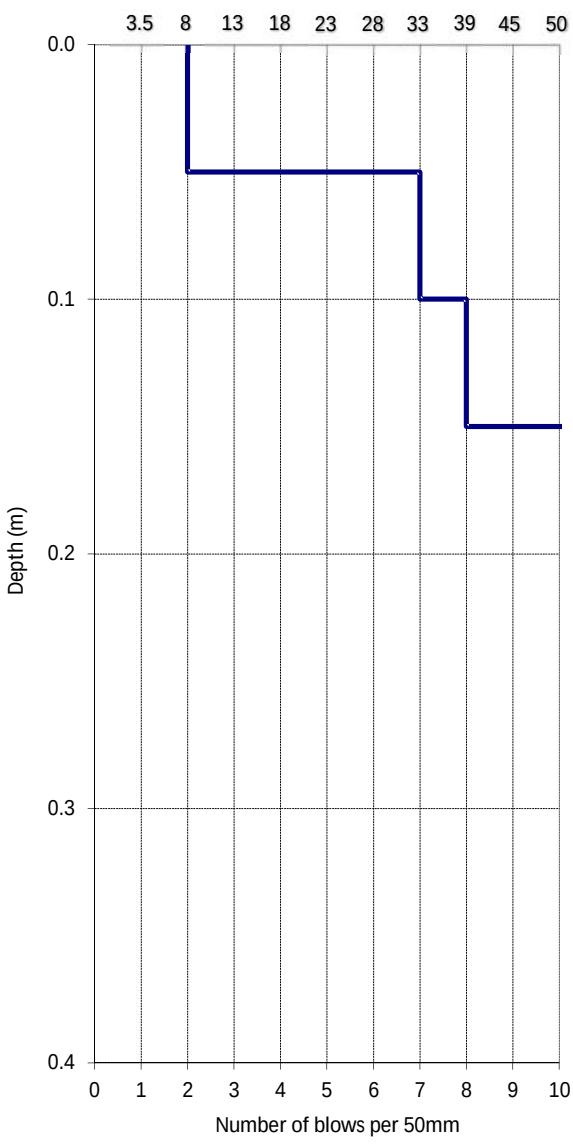
HOLE Id: Lot 450

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI						LOCATION: Terrace View, Papamoa							JOB No.: 1009168.0000																
CO-ORDINATES: (NZTM2000)						DRILL TYPE: Window Sampler							HOLE STARTED: 08/01/2019 HOLE FINISHED: 08/01/2019																
R.L.:						DRILL METHOD: PR							DRILLED BY: GEOTECHNICS																
DATUM:													LOGGED BY: EWDA CHECKED: RWM																
GEOLOGICAL										ENGINEERING DESCRIPTION																			
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.						WATER	CORE RECOVERY (%)	METHOD	SCALA PENETROMETER (Blows/50mm) 123456789									TESTS	SAMPLES	R_L (m)	DEPTH (m)	GRAPHIC LOG	MOISTURE / WEATHERING CONDITION	STRENGTH CLASSIFICATION	SHEAR STRENGTH (kPa) 10 20 30 40				Description and Additional Observations
							100	HA	3 12 >> 11 >> 12 >>													TS TS TS TS TS	M-W	St					Organic SILT, with some sand; dark brownish black. Stiff, moist to wet, low plasticity; sand, fine to coarse.
																							VD					Fine to coarse SAND, with trace silt and gravel; light orange brown. Very dense, moist to wet, well graded; gravel, fine to medium, subangular to subrounded, pumiceous.	
																											1.5m: Changes to orange brown. 1.65m: Changes to light greyish brown.		
																											End of borehole at 2.00mbgl. No standing groundwater encountered.		
COMMENTS:																													
Hole Depth 2m																													

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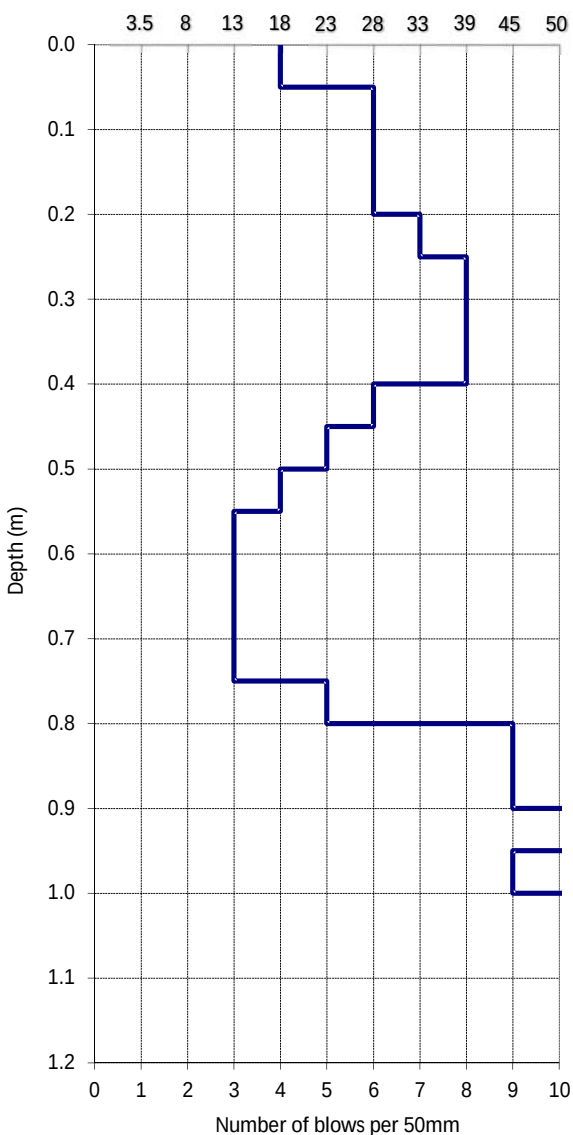
HAND AUGER LOG

HOLE Id: Lot 451

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI				LOCATION: Terrace View, Papamoa				JOB No.: 1009168.0000									
CO-ORDINATES: (NZTM2000)				DRILL TYPE: 50mm Hand Auger				HOLE STARTED: 10/12/2018									
R.L.:				DRILL METHOD: HA				HOLE FINISHED: 08/01/2019									
DATUM:								DRILLED BY: GEOTECHNICS									
								LOGGED BY: EWDA CHECKED: RWM									
GEOLOGICAL								ENGINEERING DESCRIPTION									
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.		WATER	CORE RECOVERY (%)	METHOD	SCALA PENETROMETER (Blows/50mm)				TESTS	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	MOISTURE / WEATHERING CONDITION	STRENGTH/DENSITY CLASSIFICATION	SHEAR STRENGTH (kPa)	Description and Additional Observations
					1	2	3	4	5	6	7	8	9				
								4		6				M-W	D		Fine to coarse SAND, with trace silt and gravel; light brown. Dense, moist to wet, well graded; gravel, subrounded to subangular, fine to medium, pumiceous.
										6							
										6							
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											7						
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												3			MD		0.55m: Changes to medium dense.
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															VD		0.8m: Changes to very dense.
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HAND AUGER LOG

HOLE Id: Lot 452

Hole Location: See Location Plan

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[illegible]

 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
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Project Name	TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID	1009168.0.0.0																																																																																																																																				
Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851																																																																																																																																				
Site Location	Papamoa		Material Source	Unknown																																																																																																																																				
Material Description	See Lot 452 borehole log.		Test Series	Terrace Views/111218/SC447-448,451-453,464-466,468-470																																																																																																																																				
Depth from ground surface to commencement of penetration (m)		0.0	Test Number	Lot 452 A																																																																																																																																				
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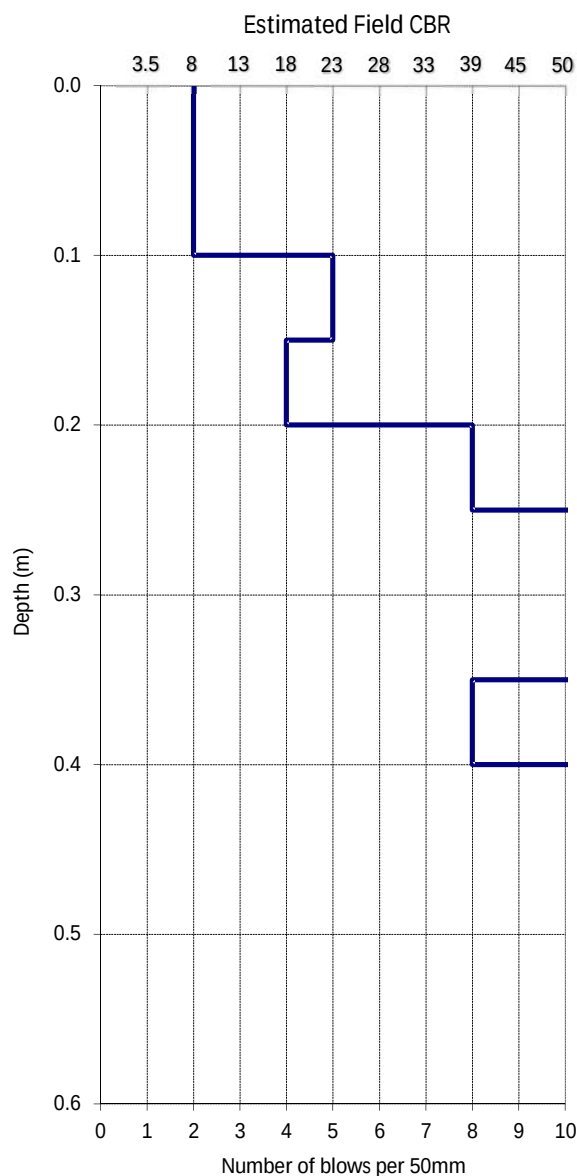
15C Amber Crescent
Judea
Tauranga 3110
New Zealand
p. +64 7 571 0280

Lab Ref/URN

019/19

Project Name	TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID	1009168.0.0.0
Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851
Site Location	Papamoa		Material Source	Unknown
Material Description	See Lot 452 borehole log.		Test Series	Terrace Views/111218/SC447-448,451-453,464-466,468-470
Depth from ground surface to commencement of penetration (m)		0.0	Test Number	Lot 452 B

Chainage		Offset		Lane	
Vertical distance driven (mm)	Depth (mm)	Number of blows	Vertical distance driven (mm)	Depth (mm)	Number of blows
50	50	2	1050	1050	
100	100	2	1100	1100	
150	150	5	1150	1150	
200	200	4	1200	1200	
250	250	8	1250	1250	
300	300	12	1300	1300	
350	350	11	1350	1350	
400	400	8	1400	1400	
450	450	12	1450	1450	
500	500	16	1500	1500	
550	550	13	1550	1550	
600	600		1600	1600	
650	650		1650	1650	
700	700		1700	1700	
750	750		1750	1750	
800	800		1800	1800	
850	850		1850	1850	
900	900		1900	1900	
950	950		1950	1950	
1000	1000		2000	2000	



Tested By	DBRU	Date	11/12/2018
Data Entry By	ALDE	Date	17/12/2018
Checked By	RWM	Date	7/02/2019



HAND AUGER LOG

HOLE Id: Lot 453

Hole Location: See Location Plan

SHEET: 1 OF 1

[illegible]

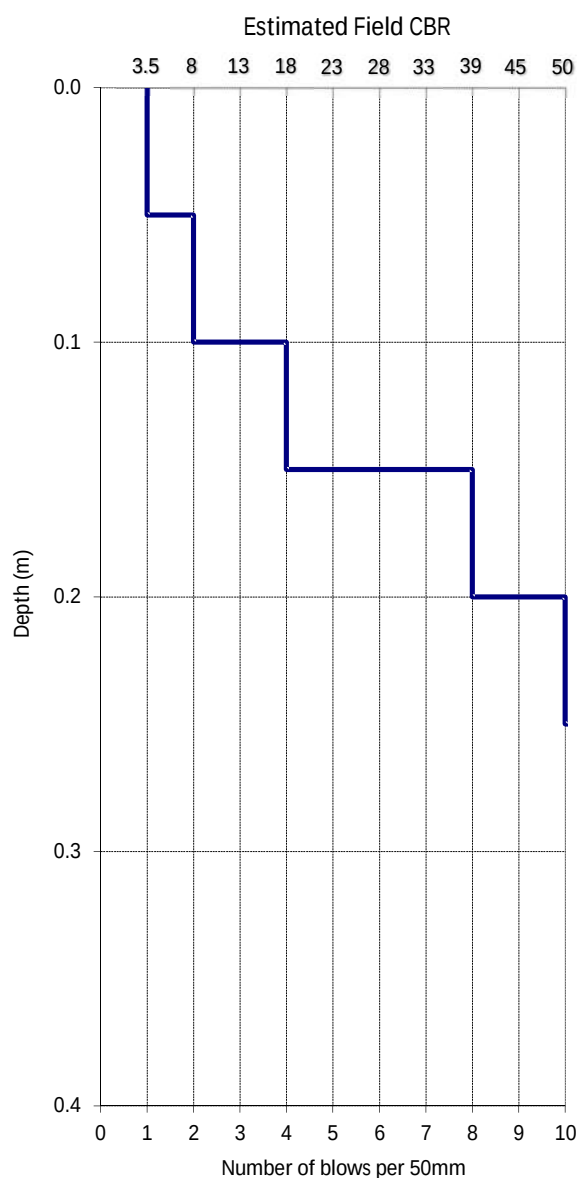
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Tauranga 3110
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p. +64 7 571 0280

Lab Ref/URN

019/19

Project Name	TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID	1009168.0.0.0
Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851
Site Location	Papamoa		Material Source	Unknown
Material Description	See Lot 453 borehole log.		Test Series	Terrace Views/111218/SC447-448,451-453,464-466,468-470
Depth from ground surface to commencement of penetration (m)		0.0	Test Number	Lot 453 A

Chainage		Offset		Lane	
Vertical distance driven (mm)	Depth (mm)	Number of blows	Vertical distance driven (mm)	Depth (mm)	Number of blows
50	50	1	1050	1050	
100	100	2	1100	1100	
150	150	4	1150	1150	
200	200	8	1200	1200	
250	250	10	1250	1250	
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700	700		1700	1700	
750	750		1750	1750	
800	800		1800	1800	
850	850		1850	1850	
900	900		1900	1900	
950	950		1950	1950	
1000	1000		2000	2000	



Test Remarks

Tested By	DBRU	Date	11/12/2018
Data Entry By	ALDE	Date	17/12/2018
Checked By	RWM	Date	7/02/2019

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Depth from ground surface to commencement of penetration (m)		0.0	Test Number	Lot 453 B																																																																																																																																				
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HAND AUGER LOG

HOLE Id: Lot 454

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI					LOCATION: Terrace View, Papamoa					JOB No.: 1009168.0000									
CO-ORDINATES: (NZTM2000)					DRILL TYPE: 50mm Hand Auger					HOLE STARTED: 10/12/2018									
R.L.:					DRILL METHOD: HA					HOLE FINISHED: 08/01/2019									
DATUM:										DRILLED BY: GEOTECHNICS									
										LOGGED BY: DBRU									
										CHECKED: RWM									
GEOLOGICAL										ENGINEERING DESCRIPTION									
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.										Description and Additional Observations									
WATER										MOISTURE / WEATHERING CONDITION									
CORE RECOVERY (%)										STRENGTH / STABILITY CLASSIFICATION									
METHOD										SHEAR STRENGTH (kPa)									
SCALA PENETROMETER (Blows/50mm)										GRAPHIC LOG									
1 2 3 4 5 6 7 8 9										10 20 30 40									
4										M									
9										VD									
10										10									
11 >>										1.45m: Changes to trace silt; wet.									
10										End of borehole at 2.00mbgl.									
										No standing groundwater encountered.									
COMMENTS:																			
Hole Depth 2m																			

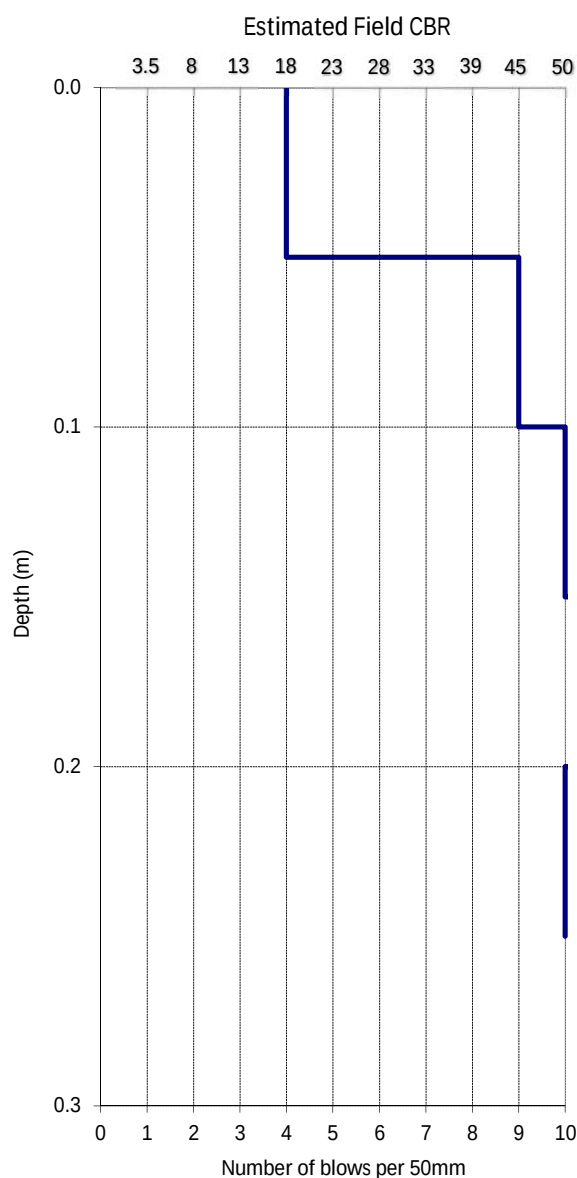
15C Amber Crescent
Judea
Tauranga 3110
New Zealand
p. +64 7 571 0280

Lab Ref/URN

018/19

Project Name	TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID	1009168.0.0.0
Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851
Site Location	Papamoa		Material Source	Unknown
Material Description	See Lot 454 borehole log.		Test Series	Terrace Views/101218/SC452,454-457
Depth from ground surface to commencement of penetration (m)		0.0	Test Number	Lot 454 A

Chainage		Offset		Lane	
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Tested By	PIFA	Date	10/12/2018
Data Entry By	ALDE	Date	17/12/2018
Checked By	RWM	Date	7/02/2019

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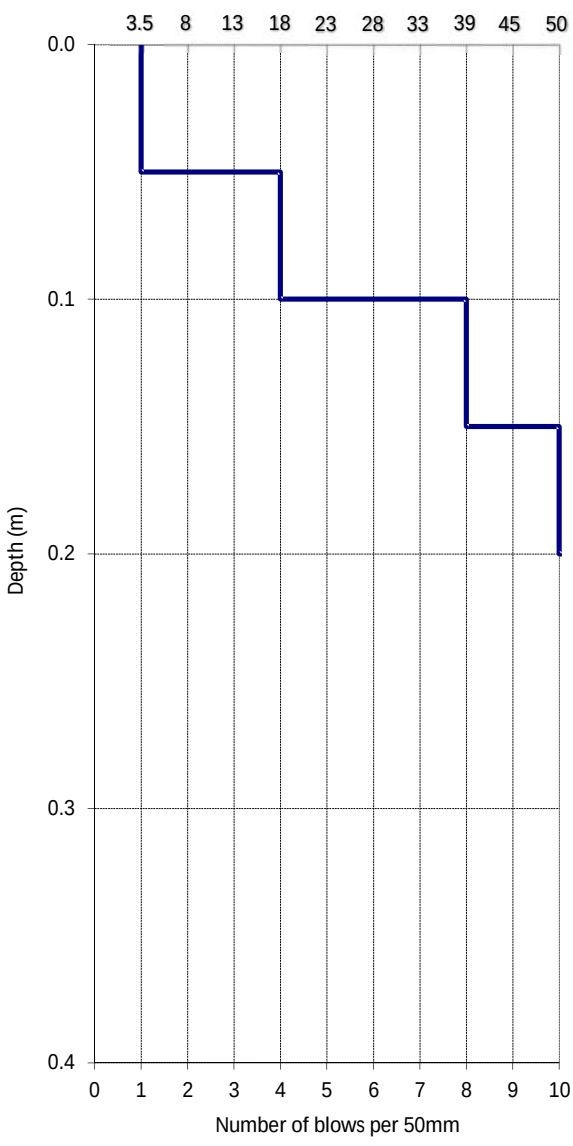
HOLE Id: Lot 455

Hole Location: See Location Plan

SHEET: 1 OF 1

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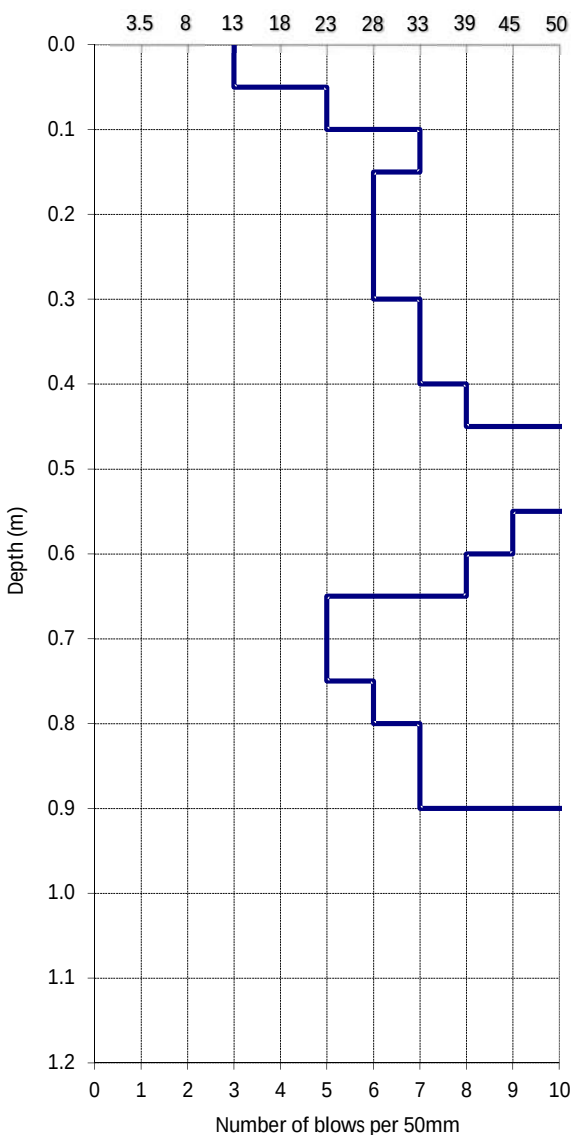
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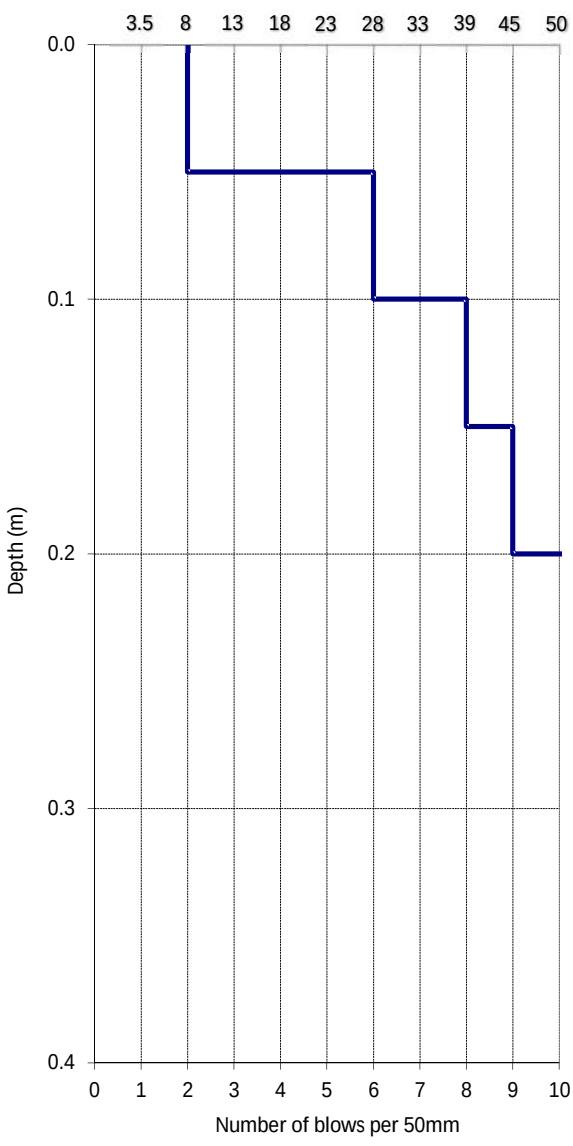
HOLE Id: **Lot 456**

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI				LOCATION: Terrace View, Papamoa				JOB No.: 1009168.0000											
CO-ORDINATES: (NZTM2000)				DRILL TYPE: 50mm Hand Auger				HOLE STARTED: 10/12/2018											
R.L.:				DRILL METHOD: HA				HOLE FINISHED: 08/01/2019											
DATUM:								DRILLED BY: GEOTECHNICS											
								LOGGED BY: EWDA											
								CHECKED: RWM											
GEOLOGICAL										ENGINEERING DESCRIPTION									
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION. WATER CORE RECOVERY (%) METHOD SCALA PENETROMETER (Blows/50mm) TESTS SAMPLES RL (m) DEPTH (m) GRAPHIC LOG MOISTURE / WEATHERING CONDITION STRENGTH / DENSITY CLASSIFICATION SHEAR STRENGTH (kPa)										Description and Additional Observations									
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HAND AUGER LOG

HOLE Id: Lot 457

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI				LOCATION: Terrace View, Papamoa				JOB No.: 1009168.0000										
CO-ORDINATES: (NZTM2000)				DRILL TYPE: 50mm Hand Auger				HOLE STARTED: 10/12/2018										
R.L.:				DRILL METHOD: HA				HOLE FINISHED: 08/01/2019										
DATUM:				DRILLED BY: GEOTECHNICS				LOGGED BY: PIFA										
				CHECKED: RWM														
GEOLOGICAL								ENGINEERING DESCRIPTION										
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.		WATER	CORE RECOVERY (%)	METHOD	SCALA PENETROMETER (Blows/50mm)				TESTS	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	MOISTURE WEATHERING CONDITION	STRENGTH DENSITY CLASSIFICATION	SHEAR STRENGTH (kPa)	Description and Additional Observations	
					1	2	3	4	5	6	7	8	9					
					1					8				D-M	D		Fine to coarse SAND, with trace silt and gravel; light orange brown. Dense, dry to moist, well graded; gravel, fine, subangular to subrounded, pumiceous.	
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Project Name	TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID	1009168.0.0.0																																																																																																																																				
Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851																																																																																																																																				
Site Location	Papamoa		Material Source	Unknown																																																																																																																																				
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Checked By	RWM	Date	7/02/2019																																																																																																																																					



HAND AUGER LOG

HOLE Id: **Lot 458**

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI				LOCATION: Terrace View, Papamoa				JOB No.: 1009168.0000																															
CO-ORDINATES: (NZTM2000)				DRILL TYPE: Window Sampler				HOLE STARTED: 14/12/2018																															
R.L.:				DRILL METHOD: PR				HOLE FINISHED: 08/01/2019																															
DATUM:								DRILLED BY: GEOTECHNICS																															
								LOGGED BY: TURI																															
								CHECKED: RWM																															
GEOLOGICAL										ENGINEERING DESCRIPTION																													
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.										DESCRIPTION AND ADDITIONAL OBSERVATIONS																													
WATER CORE RECOVERY (%) METHOD SCALA PENETROMETER (Blows/50mm) TESTS SAMPLES RL (m) DEPTH (m) GRAPHIC LOG MOISTURE / WEATHERING CONDITION STRENGTH / DENSITY CLASSIFICATION SHEAR STRENGTH (kPa)																																							
1 2 4 5 5 16 >> 18 >> 17 >> 15 >> 39 >> 34 >> 32 >> 16 >> 20 >> 15 >> 8 7 8 9 10										TS TS TS TS 0.5 1.0 1.5 2.0										M St MD VD D										Organic SILT, with some sand; dark brownish black. Stiff, moist, non-plastic; sand, fine to coarse. Fine to coarse SAND, with trace silt and gravel; light orange brown. Medium dense, moist, well graded; gravel, fine to medium, subangular to subrounded, pumiceous. 0.5m: Changes to very dense. 1.5m: Changes to dense. End of borehole at 2.00mbgl. No standing groundwater encountered.									
COMMENTS: Scala Penetrometer results obtained from																																							
Hole Depth 2m																																							



Ground Floor, 247 Cameron Road
PO Box 2292, Tauranga
New Zealand
Tel: +64 7 578 6183
www.aurecongroup.com

Client: **Zariba Properties Limited**
Project: **Terrace Views**
Location: **Terrace Views, Papamoa - Stage 3D/3E**
Project Reference: **247593**

Lot 458

Sheet 1 of 1

DRILLING INFORMATION

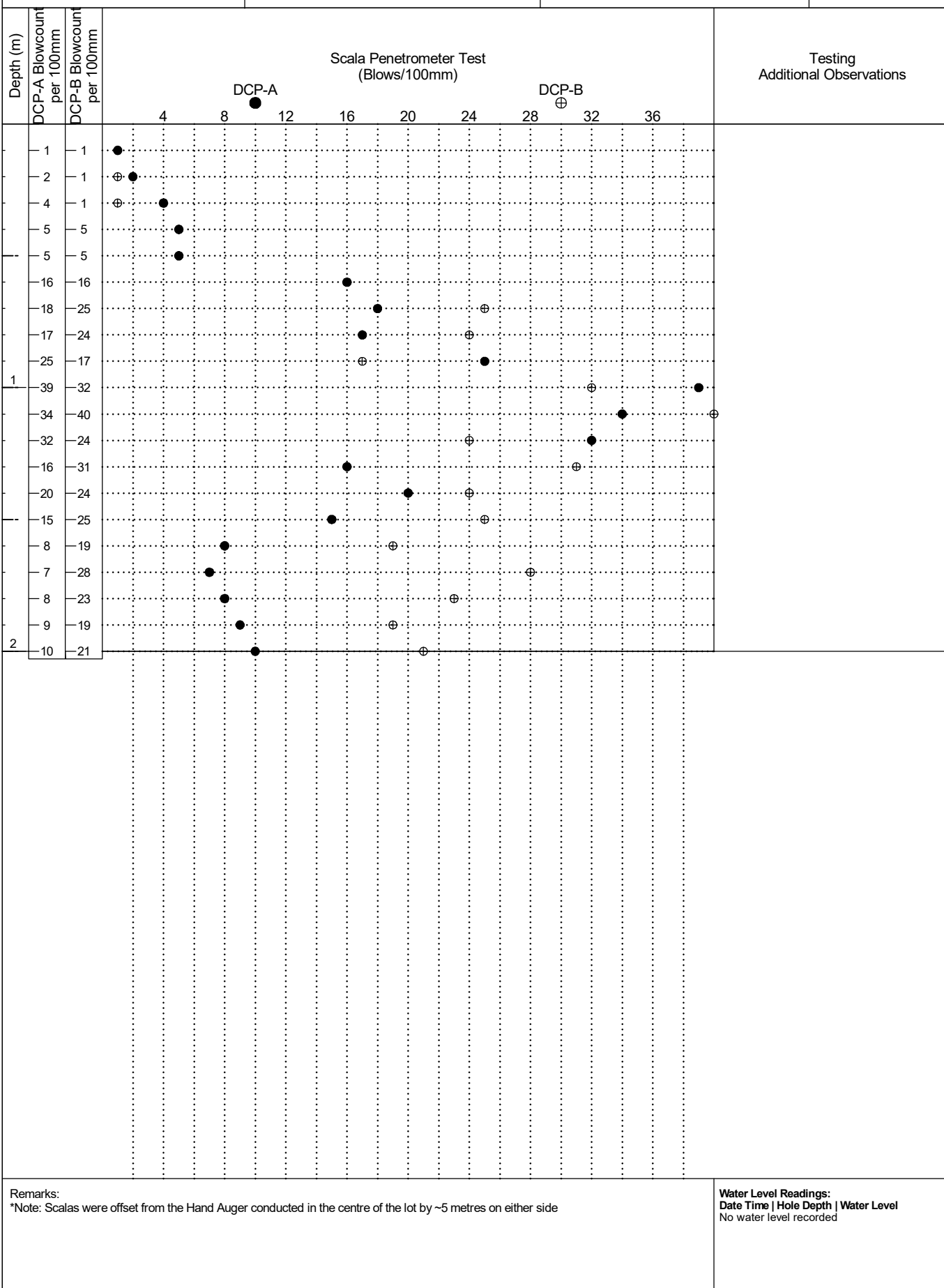
Method: Scala
Equipment: Scala
Contractor: Aurecon

CO-ORDINATES: N/A

Easting: N/A
Northing: N/A
Ground level: N/A

Date started: 19/12/2018
Date completed: 19/12/2018
Inclination: 90°
Orientation: N/A

Logged by: AY
Input by: AY
Checked by: MB
Verified by: RG





HAND AUGER LOG

HOLE Id: Lot 459

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI										LOCATION: Terrace View, Papamoa										JOB No.: 1009168.0000									
CO-ORDINATES: (NZTM2000)										DRILL TYPE: Window Sampler										HOLE STARTED: 14/12/2018									
R.L.:										DRILL METHOD: PR										HOLE FINISHED: 08/01/2019									
DATUM:										LOGGED BY: EWDA										CHECKED: RWM									
GEOLOGICAL															ENGINEERING DESCRIPTION														
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.															Description and Additional Observations														
WATER															MOISTURE CONDITION														
CORE RECOVERY (%)															WEATHERING														
METHOD															STRENGTH/CASUALTY CLASSIFICATION														
SCALA PENETROMETER (Blows/50mm)															SHEAR STRENGTH (kPa)														
TESTS																													
SAMPLES																													
RL (m)																													
DEPTH (m)																													
GRAPHIC LOG																													
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TS															MD														
TS															D														
0.5															0.5m: Changes to dense.														
1.0															1.5m: Changes to very dense, moist to wet.														
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Client: **Zariba Properties Limited**
Project: **Terrace Views**
Location: **Terrace Views, Papamoa - Stage 3D/3E**
Project Reference: **247593**

Lot 459

Sheet 1 of 1

DRILLING INFORMATION

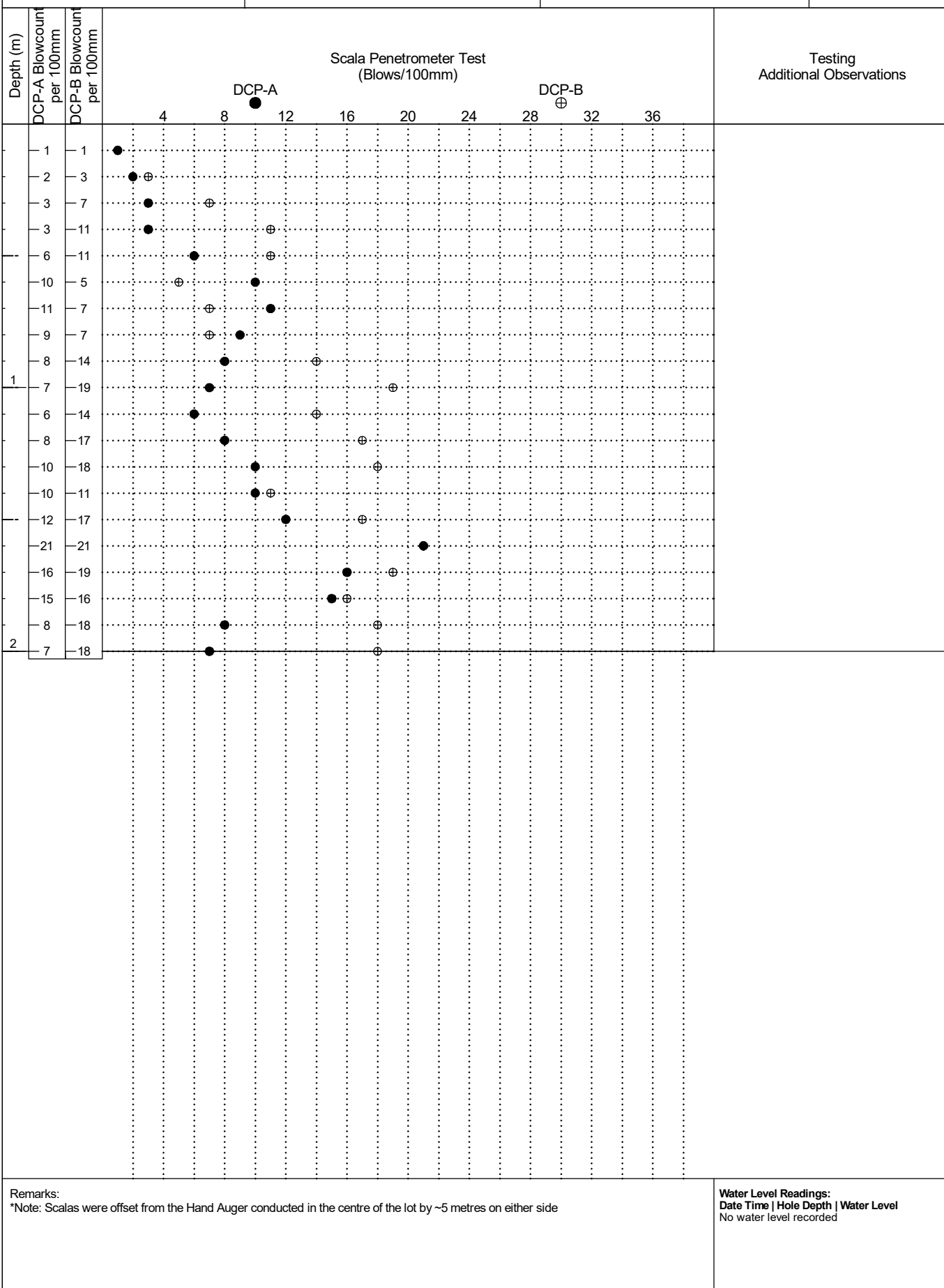
Method: Scala
Equipment: Scala
Contractor: Aurecon

CO-ORDINATES: N/A

Easting: N/A
Northing: N/A
Ground level: N/A

Date started: 19/12/2018
Date completed: 19/12/2018
Inclination: 90°
Orientation: N/A

Logged by: AY
Input by: AY
Checked by: MB
Verified by: RG





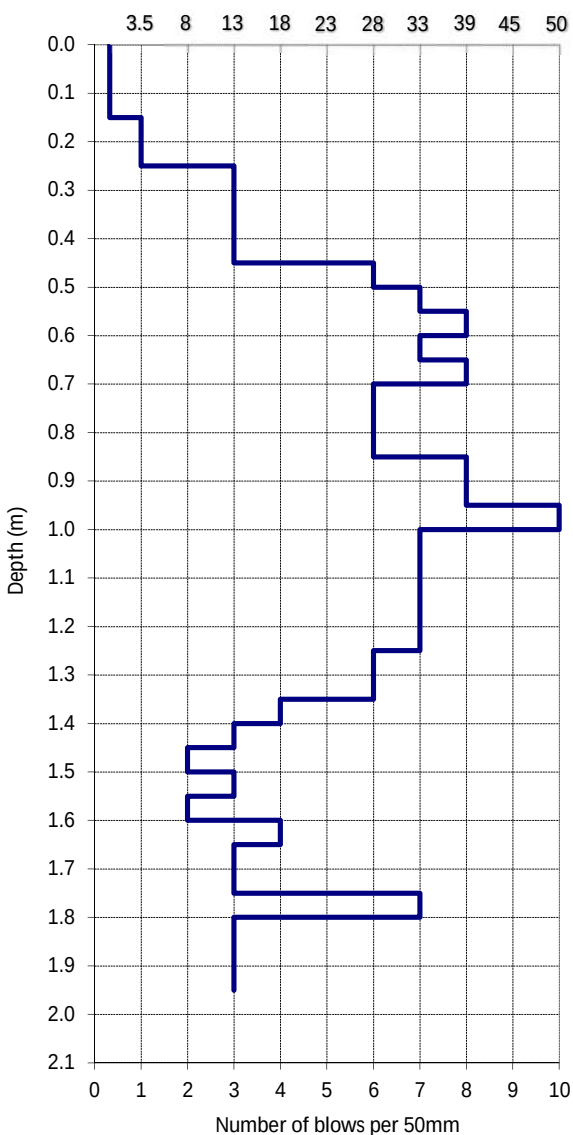
HAND AUGER LOG

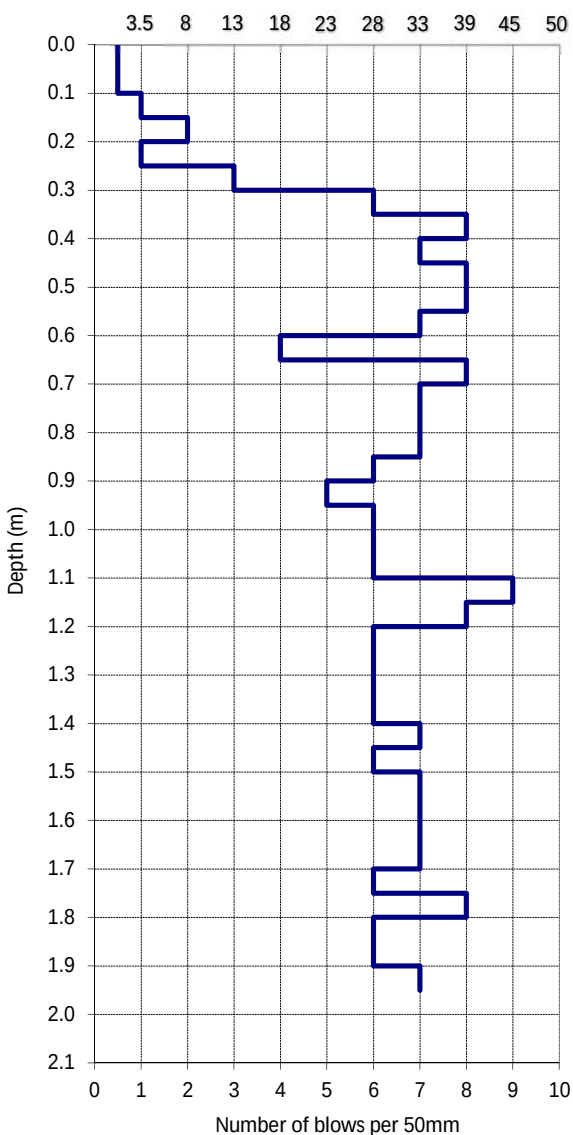
HOLE Id: Lot 460

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI				LOCATION: Terrace View, Papamoa				JOB No.: 1009168.0000									
CO-ORDINATES: (NZTM2000)				DRILL TYPE: Window Sampler				HOLE STARTED: 14/12/2018									
R.L.:				DRILL METHOD: PR				HOLE FINISHED: 08/01/2019									
DATUM:								DRILLED BY: GEOTECHNICS									
								LOGGED BY: EWDA CHECKED: RWM									
GEOLOGICAL								ENGINEERING DESCRIPTION									
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.		WATER	CORE RECOVERY (%)	METHOD	SCALA PENETROMETER (Blows/50mm)				TESTS	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	MOISTURE / WEATHERING CONDITION	STRENGTH / PLASTICITY CLASSIFICATION	SHEAR STRENGTH (kPa)	Description and Additional Observations
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																	No standing groundwater encountered.
COMMENTS:																	
Hole Depth 2m																	

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HAND AUGER LOG

HOLE Id: Lot 461

Hole Location: See Location Plan

SHEET: 1 OF 1

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HAND AUGER LOG

HOLE Id: Lot 462

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI						LOCATION: Terrace View, Papamoa							JOB No.: 1009168.0000																						
CO-ORDINATES: (NZTM2000)						DRILL TYPE: Window Sampler						HOLE STARTED: 14/12/2018																							
R.L.:						DRILL METHOD: PR						HOLE FINISHED: 08/01/2019																							
DATUM:												DRILLED BY: GEOTECHNICS				LOGGED BY: EWDA				CHECKED: RWM															
GEOLOGICAL											ENGINEERING DESCRIPTION																								
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION											WATER	CORE RECOVERY (%)	METHOD	SCALA PENETROMETER (Blows/50mm) 123456789									TESTS	SAMPLES	R.L. (m)	DEPTH (m)	GRAPHIC LOG	MOISTURE / WEATHERING CONDITION	STRENGTH/DENSITY CLASSIFICATION	SHEAR STRENGTH (kPa) 102550100200	Description and Additional Observations				
														2 8 10 11 >> 10											TS TS TS TS TS	M	St		Organic SILT, with some sand; dark brownish black. Stiff, moist, non-plastic; sand, fine to coarse.						
																										VD		Fine to coarse SAND, with trace silt and gravel; light orange brown. Very dense, moist, well graded; gravel, fine to medium, subangular to subrounded, pumiceous.							
																									0.5 1.0 1.5 2.0					End of borehole at 2.00mbgl. No standing groundwater encountered.					
COMMENTS:																																			
Hole Depth 2m																																			

 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
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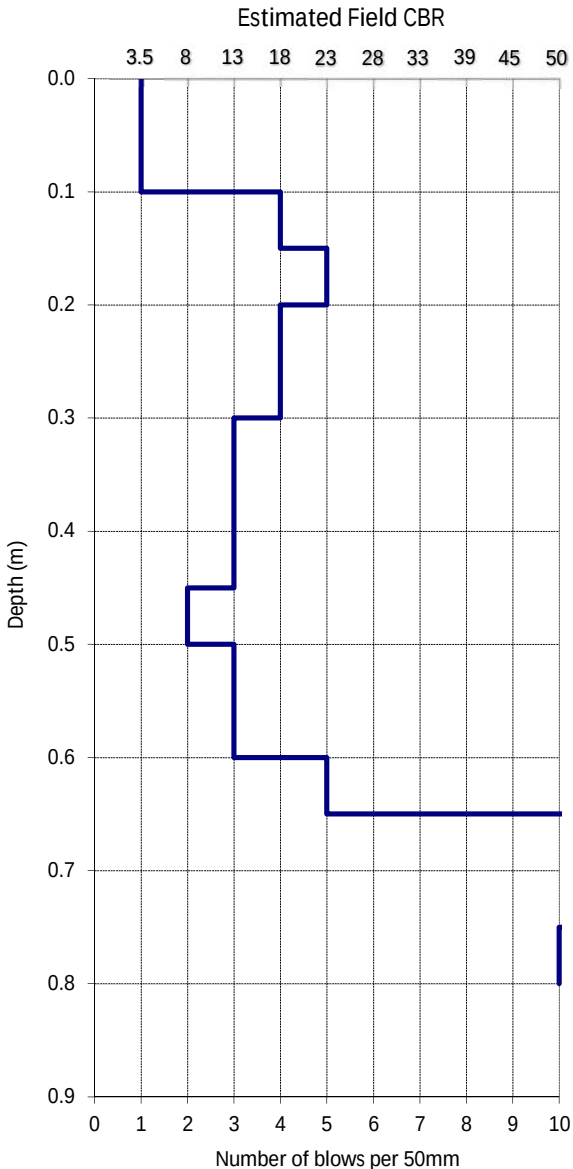
HOLE Id: Lot 463

Hole Location: See Location Plan

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Data Entry By	ALDE		Date	17/12/2018																																																																																																																																				
Checked By	RWM		Date	7/02/2019																																																																																																																																				



HAND AUGER LOG

HOLE Id: Lot 464

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI										LOCATION: Terrace View, Papamoa										JOB No.: 1009168.0000									
CO-ORDINATES: (NZTM2000)										DRILL TYPE: 50mm Hand Auger										HOLE STARTED: 10/12/2018 HOLE FINISHED: 08/01/2019									
R.L.:										DRILL METHOD: HA										DRILLED BY: GEOTECHNICS									
DATUM:																				LOGGED BY: TURI CHECKED: RWM									
GEOLOGICAL															ENGINEERING DESCRIPTION														
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.															Description and Additional Observations														
WATER															MOISTURE / WEATHERING														
CORE RECOVERY (%)															STRENGTH/DENSITY														
METHOD															CLASSIFICATION														
SCALA PENETROMETER (Blows/50mm)															SHEAR STRENGTH (kPa)														
TESTS																													
SAMPLES																													
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Project Name	TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID	1009168.0.0.0																																																																																																																																				
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Site Location	Papamoa		Material Source	Unknown																																																																																																																																				
Material Description	See Lot 464 borehole log.		Test Series	Terrace Views/111218/SC447-448,451-453,464-466,468-470																																																																																																																																				
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Checked By	RWM		Date	7/02/2019																																																																																																																																				



HAND AUGER LOG

HOLE Id: Lot 465

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI				LOCATION: Terrace View, Papamoa				JOB No.: 1009168.0000																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
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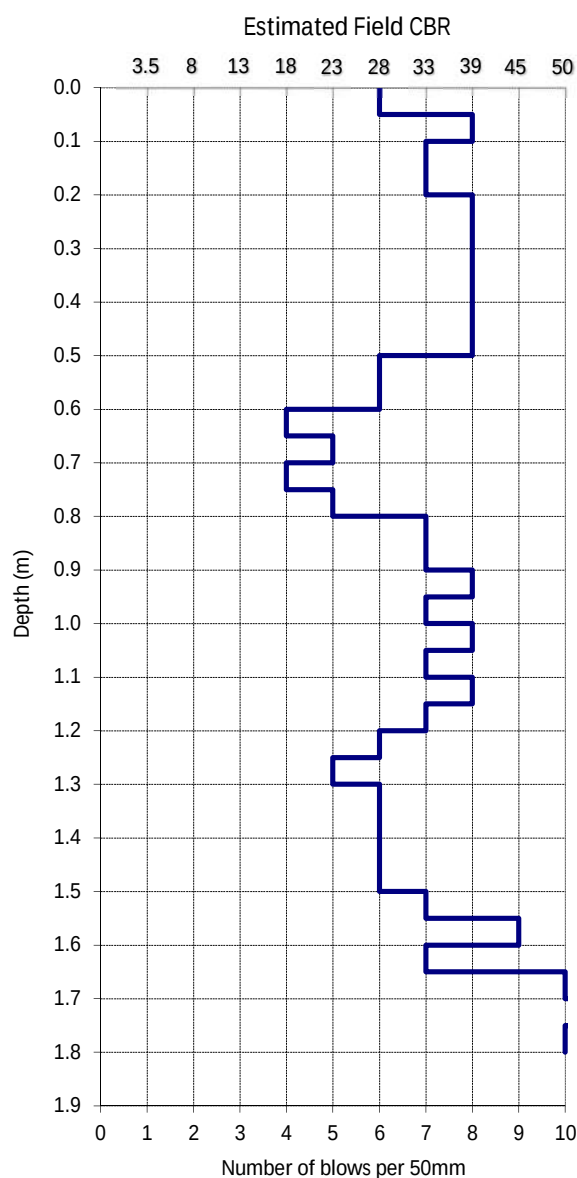
15C Amber Crescent
Judea
Tauranga 3110
New Zealand
p. +64 7 571 0280

Lab Ref/URN

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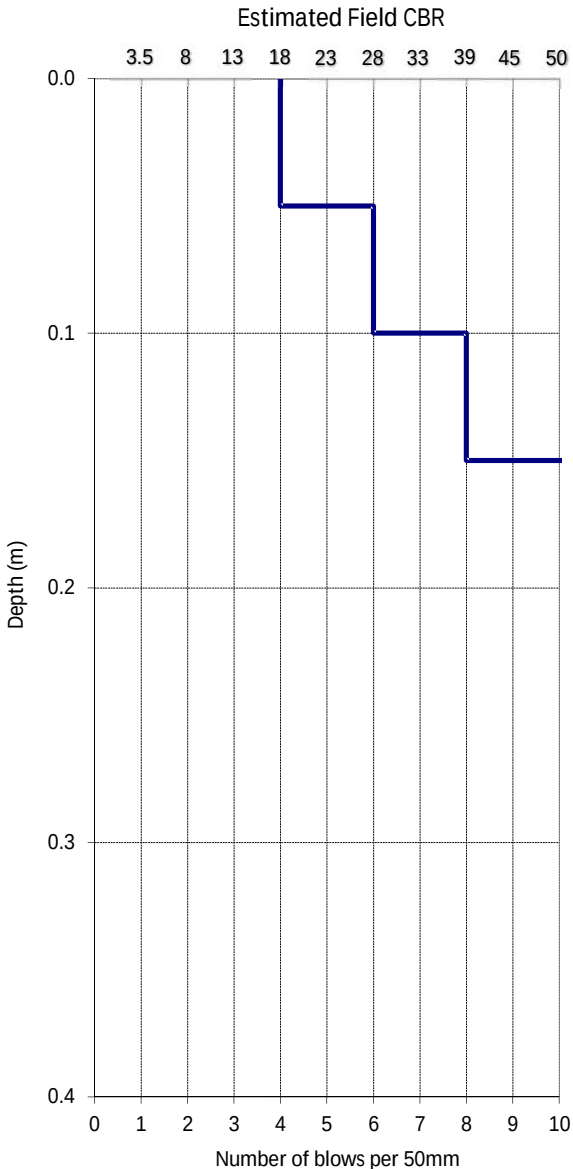
Project Name	TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID	1009168.0.0.0
Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851
Site Location	Papamoa		Material Source	Unknown
Material Description	See Lot 465 borehole log.		Test Series	Terrace Views/111218/SC447-448,451-453,464-466,468-470
Depth from ground surface to commencement of penetration (m)		0.0	Test Number	Lot 465 A

Chainage		Offset		Lane	
Vertical distance driven (mm)	Depth (mm)	Number of blows	Vertical distance driven (mm)	Depth (mm)	Number of blows
50	50	6	1050	1050	8
100	100	8	1100	1100	7
150	150	7	1150	1150	8
200	200	7	1200	1200	7
250	250	8	1250	1250	6
300	300	8	1300	1300	5
350	350	8	1350	1350	6
400	400	8	1400	1400	6
450	450	8	1450	1450	6
500	500	8	1500	1500	6
550	550	6	1550	1550	7
600	600	6	1600	1600	9
650	650	4	1650	1650	7
700	700	5	1700	1700	10
750	750	4	1750	1750	11
800	800	5	1800	1800	10
850	850	7	1850	1850	
900	900	7	1900	1900	
950	950	8	1950	1950	
1000	1000	7	2000	2000	



Test Remarks

Tested By	EWDA	Date	11/12/2018
Data Entry By	ALDE	Date	17/12/2018
Checked By	RWM	Date	7/02/2019

 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
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HAND AUGER LOG

HOLE Id: Lot 466

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI				LOCATION: Terrace View, Papamoa				JOB No.: 1009168.0000											
CO-ORDINATES: (NZTM2000)				DRILL TYPE: 50mm Hand Auger				HOLE STARTED: 10/12/2018											
R.L.:				DRILL METHOD: HA				HOLE FINISHED: 08/01/2019											
DATUM:								DRILLED BY: GEOTECHNICS											
								LOGGED BY: TURI											
								CHECKED: RWM											
GEOLOGICAL										ENGINEERING DESCRIPTION									
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION. WATER CORE RECOVERY (%) METHOD SCALA PENETROMETER (Blows/50mm) 1 2 3 4 5 6 7 8 9 TESTS SAMPLES RL (m) DEPTH (m) GRAPHIC LOG MOISTURE / WEATHERING CONDITION STRENGTH / DENSITY CLASSIFICATION 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 260 270 280 290 300 310 320 330 340 350 360 370 380 390 400 410 420 430 440 450 460 470 480 490 500 510 520 530 540 550 560 570 580 590 600 610 620 630 640 650 660 670 680 690 700 710 720 730 740 750 760 770 780 790 800 810 820 830 840 850 860 870 880 890 900 910 920 930 940 950 960 970 980 990 1000 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 260 270 280 290 300 310 320 330 340 350 360 370 380 390 400 410 420 430 440 450 460 470 480 490 500 510 520 530 540 550 560 570 580 590 600 610 620 630 640 650 660 670 680 690 700 710 720 730 740 750 760 770 780 790 800 810 820 830 840 850 860 870 880 890 900 910 920 930 940 950 960 970 980 990 1000 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 260 270 280 290 300 310 320 330 340 350 360 370 380 390 400 410 420 430 440 450 460 470 480 490 500 510 520 530 540 550 560 570 580 590 600 610 620 630 640 650 660 670 680 690 700 710 720 730 740 750 760 770 780 790 800 810 820 830 840 850 860 870 880 890 900 910 920 930 940 950 960 970 980 990 1000										Description and Additional Observations Sandy SILT; dark brownish black. Firm, moist, non-plastic; sand, fine to coarse. Fine to coarse SAND, with minor silt, trace gravel; light yellowish brown. Very dense, moist; gravel, fine, subangular to subrounded, pumiceous. 0.9m: Changes to yellowish brown; wet. 1.7m: Changes to brown. 1.9m: Changes to greyish brown. End of hole at 2.00mbgl. No standing groundwater encountered.									
COMMENTS:																			
Hole Depth 2m																			

 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
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Data Entry By	ALDE		Date	17/12/2018																																																																																																																																				
Checked By	RWM		Date	7/02/2019																																																																																																																																				



HAND AUGER LOG

HOLE Id: Lot 467

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI						LOCATION: Terrace View, Papamoa						JOB No.: 1009168.0000											
CO-ORDINATES: (NZTM2000)						DRILL TYPE: 50mm Hand Auger						HOLE STARTED: 10/12/2018 HOLE FINISHED: 08/01/2019											
R.L.:						DRILL METHOD: HA						DRILLED BY: GEOTECHNICS											
DATUM:												LOGGED BY: TURI CHECKED: RWM											
GEOLOGICAL												ENGINEERING DESCRIPTION											
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.												Description and Additional Observations											
WATER CORE RECOVERY (%) METHOD SCALA PENETROMETER (Blows/50mm) TESTS SAMPLES RL (m) DEPTH (m) GRAPHIC LOG MOISTURE / WEATHERING CONDITION STRENGTH / STABILITY CLASSIFICATION SHEAR STRENGTH (kPa)																							
1 2 3 4 5 6 7 8 9																							
5 5 9 12 >> 15 >> 12 >>												TS M-W St D VD W End of borehole at 2.00mbgl. No standing groundwater encountered											
DRY 08/01/2019																							
2.0																							
COMMENTS:																							
Hole Depth 2m																							

 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
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Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851																																																																																																																																				
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Material Description	See Lot 467 borehole log.		Test Series	Terrace Views/111218/SC447-448,451-453,464-466,468-470																																																																																																																																				
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HAND AUGER LOG

HOLE Id: **Lot 468**

Hole Location: See Location Plan

SHEET: 1 OF 1

[illegible]

15C Amber Crescent
Judea
Tauranga 3110
New Zealand
p. +64 7 571 0280

Page 1 of 1

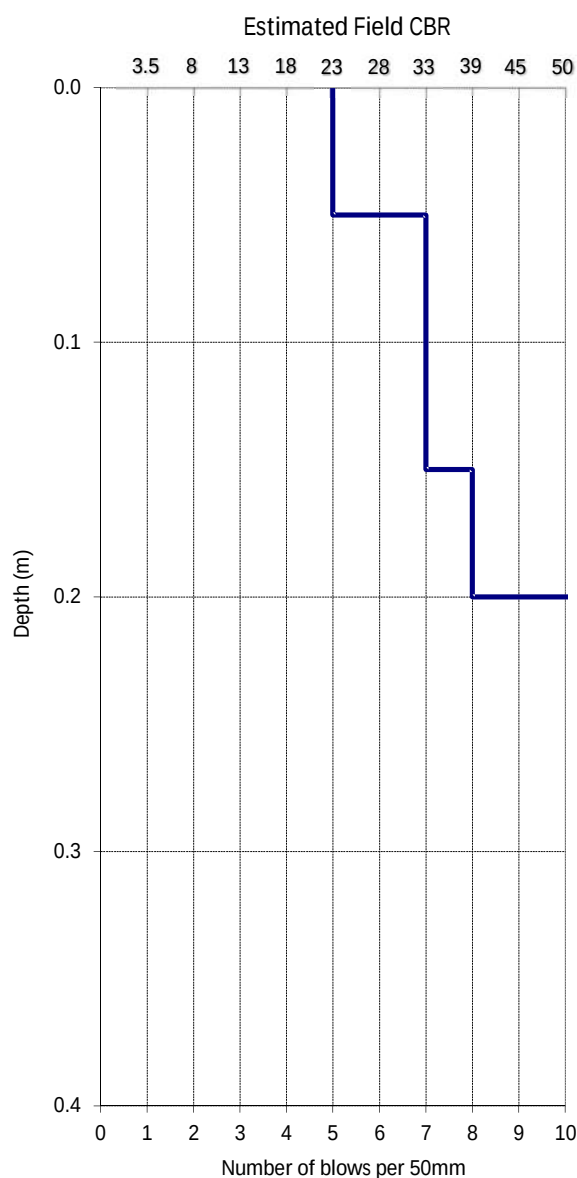
Lab Ref/URN

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NZS 4402: 1998 Test 6.5.2 Dynamic Cone Penetrometer - Scala

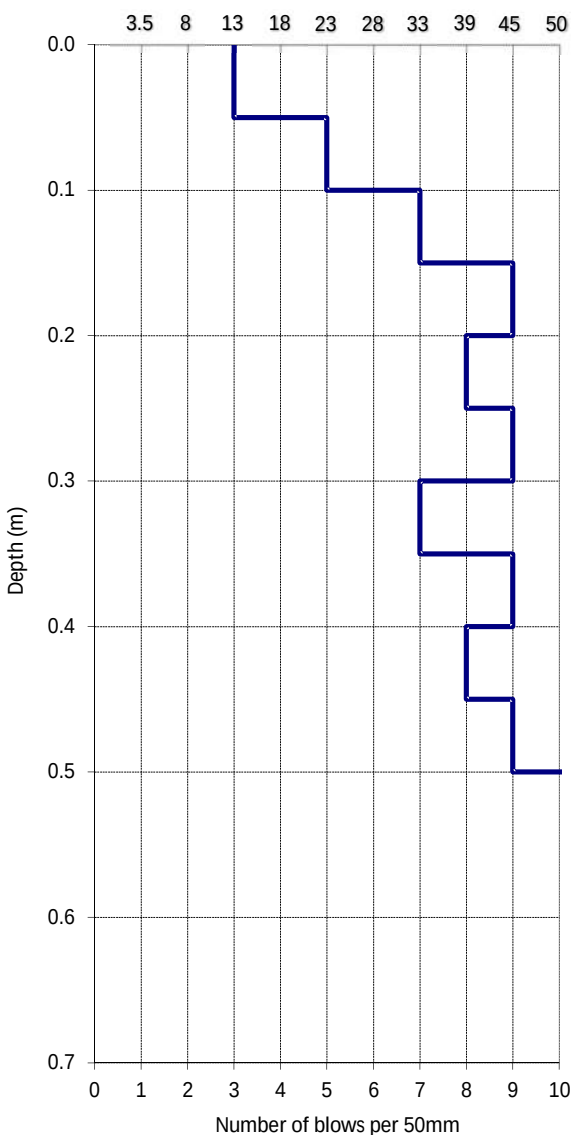
Project Name	TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID	1009168.0.0.0
Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851
Site Location	Papamoa		Material Source	Unknown
Material Description	See Lot 468 borehole log.		Test Series	Terrace Views/111218/SC447-448,451-453,464-466,468-470
Depth from ground surface to commencement of penetration (m)		0.0	Test Number	Lot 468 A

Chainage		Offset		Lane	
Vertical distance driven (mm)	Depth (mm)	Number of blows	Vertical distance driven (mm)	Depth (mm)	Number of blows
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200	200	8	1200	1200	
250	250	11	1250	1250	
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350	350	17	1350	1350	
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800	800		1800	1800	
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900	900		1900	1900	
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Test Remarks

Tested By	EWDA	Date	18/12/2018
Data Entry By	ALDE	Date	17/12/2018
Checked By	RWM	Date	7/02/2019

 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
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HAND AUGER LOG

HOLE Id: Lot 469

Hole Location: See Location Plan

SHEET: 1 OF 1

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 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
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HAND AUGER LOG

HOLE Id: Lot 470

Hole Location: See Location Plan

SHEET: 1 OF 1

[illegible]

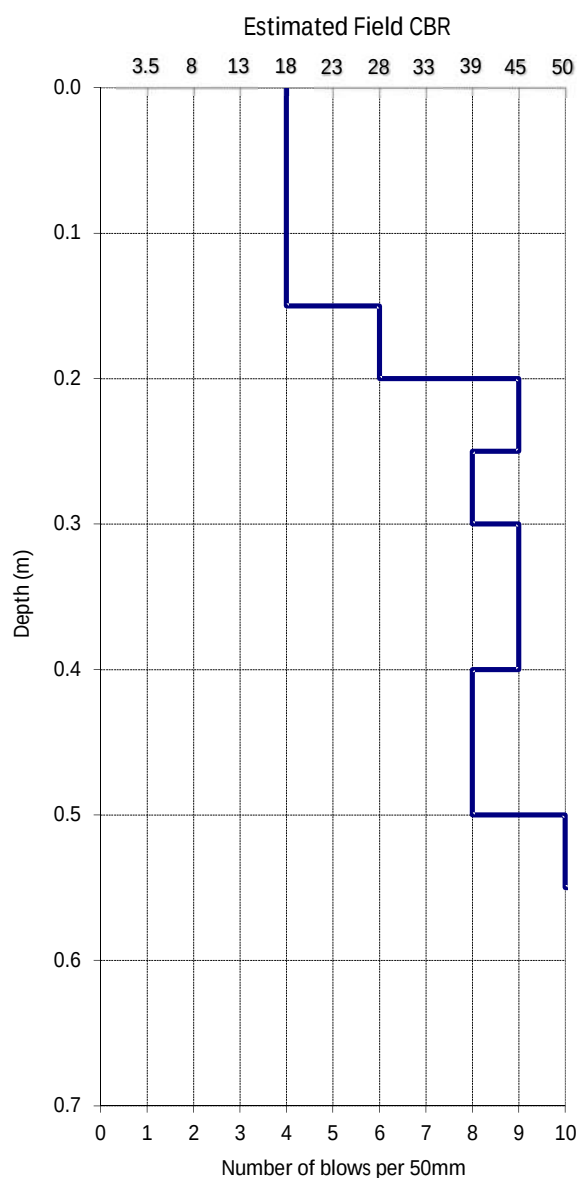
15C Amber Crescent
Judea
Tauranga 3110
New Zealand
p. +64 7 571 0280

Lab Ref/URN

019/19

Project Name	TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID	1009168.0.0.0
Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851
Site Location	Papamoa		Material Source	Unknown
Material Description	See Lot 470 borehole log.		Test Series	Terrace Views/111218/SC447-448,451-453,464-466,468-470
Depth from ground surface to commencement of penetration (m)		0.0	Test Number	Lot 470 A

Chainage		Offset		Lane	
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500	500	8	1500	1500	
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600	600	13	1600	1600	
650	650	13	1650	1650	
700	700		1700	1700	
750	750		1750	1750	
800	800		1800	1800	
850	850		1850	1850	
900	900		1900	1900	
950	950		1950	1950	
1000	1000		2000	2000	



Test Remarks

Tested By	PIFA	Date	11/12/2018
Data Entry By	ALDE	Date	17/12/2018
Checked By	RWM	Date	7/02/2019

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HAND AUGER LOG

HOLE Id: **Lot 471**

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI										LOCATION: Terrace View, Papamoa										JOB No.: 1009168.0000																			
CO-ORDINATES: (NZTM2000)										DRILL TYPE: 50mm Hand Auger										HOLE STARTED: 08/01/2019																			
R.L.:										DRILL METHOD: HA										HOLE FINISHED: 08/01/2019																			
DATUM:										LOGGED BY: PIFA										CHECKED: RWM																			
GEOLOGICAL										ENGINEERING DESCRIPTION																													
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION										WATER CORE RECOVERY (%) METHOD SCALA PENETROMETER (Blows/50mm) TESTS SAMPLES RL (m) DEPTH (m) GRAPHIC LOG MOISTURE CONDITION WEATHERING STRENGTH/DENSITY CLASSIFICATION SHEAR STRENGTH (kPa)										Description and Additional Observations																			
										100 HA DRY 08/01/2019										Organic SILT, with some sand; dark brownish black. Stiff, moist to wet, non-plastic; sand, fine to medium.																			
										3 7 6 6 6 8 9 7 8 5 4 3 4 6 8 9 9 13 >> 17 >> 14 >>										M-W St M D W VD										Fine to coarse SAND, with minor gravel, trace silt; light yellowish brown. Dense, moist, well graded; gravel, fine, subangular to subrounded, pumiceous. 0.7m: Changes to wet. 1.15m: Changes to very dense. 1.5m: Changes to dark greyish brown.									
										2.0										End of borehole at 2.00 mbgl. No standing groundwater encountered.																			

COMMENTS:

 Hole Depth
 2m

Scale 1:13

Rev.: A

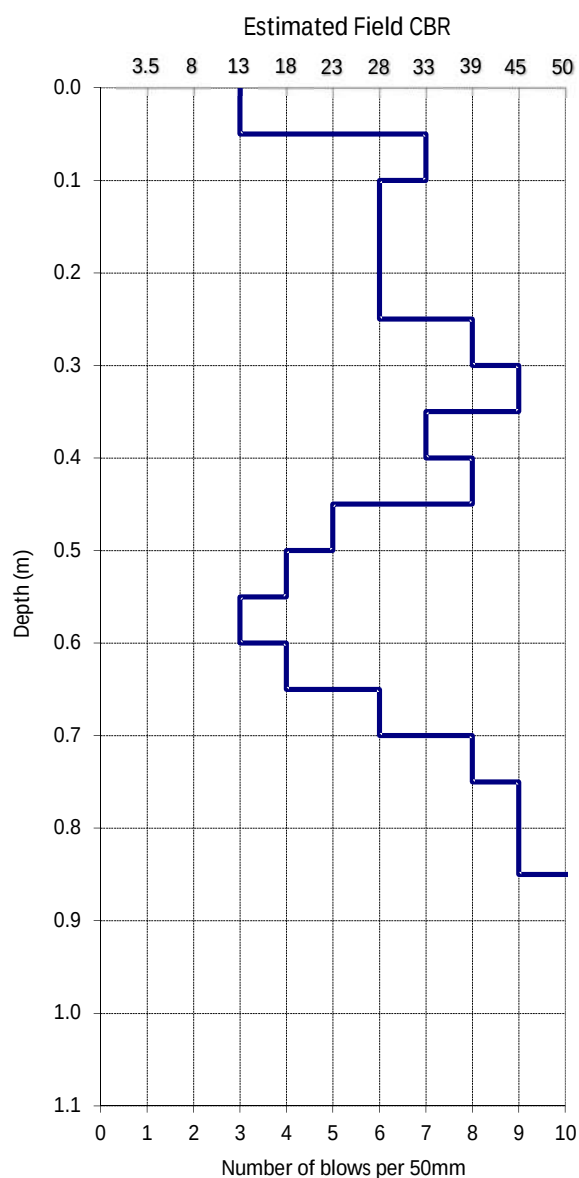
Page 1 of 1

Lab Ref/URN

019/19

Project Name	TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID	1009168.0.0.0
Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851
Site Location	Papamoa		Material Source	Unknown
Material Description	See Lot 471 borehole log.		Test Series	Terrace Views/111218/SC447-448,451-453,464-466,468-470
Depth from ground surface to commencement of penetration (m)		0.0	Test Number	Lot 471 A

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800	800	9	1800	1800	
850	850	9	1850	1850	
900	900	13	1900	1900	
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1000	1000	14	2000	2000	



Test Remarks

Tested By	PIFA	Date	4/12/2018
Data Entry By	ALDE	Date	17/12/2018
Checked By	RWM	Date	7/02/2019

 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
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100	100	5	1100	1100																																																																																																																																				
150	150	5	1150	1150																																																																																																																																				
200	200	4	1200	1200																																																																																																																																				
250	250	5	1250	1250																																																																																																																																				
300	300	4	1300	1300																																																																																																																																				
350	350	4	1350	1350																																																																																																																																				
400	400	4	1400	1400																																																																																																																																				



HAND AUGER LOG

HOLE Id: Lot 472

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI				LOCATION: Terrace View, Papamoa				JOB No.: 1009168.0000											
CO-ORDINATES: (NZTM2000)				DRILL TYPE: 50mm Hand Auger				HOLE STARTED: 08/01/2019											
R.L.:				DRILL METHOD: HA				HOLE FINISHED: 08/01/2019											
DATUM:								DRILLED BY: GEOTECHNICS											
								LOGGED BY: PIFA											
								CHECKED: RWM											
GEOLOGICAL										ENGINEERING DESCRIPTION									
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION:										WATER CORE RECOVERY (%) METHOD SCALA PENETROMETER (Blows/50mm) 1 2 3 4 5 6 7 8 9 TESTS SAMPLES RL (m) DEPTH (m) GRAPHIC LOG MOISTURE CONDITION STRENGTH/PLASTICITY CLASSIFICATION SHEAR STRENGTH (kPa) 10 20 30 40 Description and Additional Observations									
										TS TS TS TS D-M St									
										TS TS TS TS M D									
										0.5 1.0 1.5 2.0									
										M-W VD									
										1.3m: Changes to very dense, moist to wet. 1.5m: Changes to dark brown.									
										End of borehole at 2.00mbgl. No standing groundwater encountered.									
COMMENTS: Scala Penetrometer results obtained from Aurecon.																			
Hole Depth 2m																			



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Client: **Zariba Properties Limited**
Project: **Terrace Views**
Location: **Terrace Views, Papamoa - Stage 3D/3E**
Project Reference: **247593**

Lot 472

Sheet 1 of 1

DRILLING INFORMATION

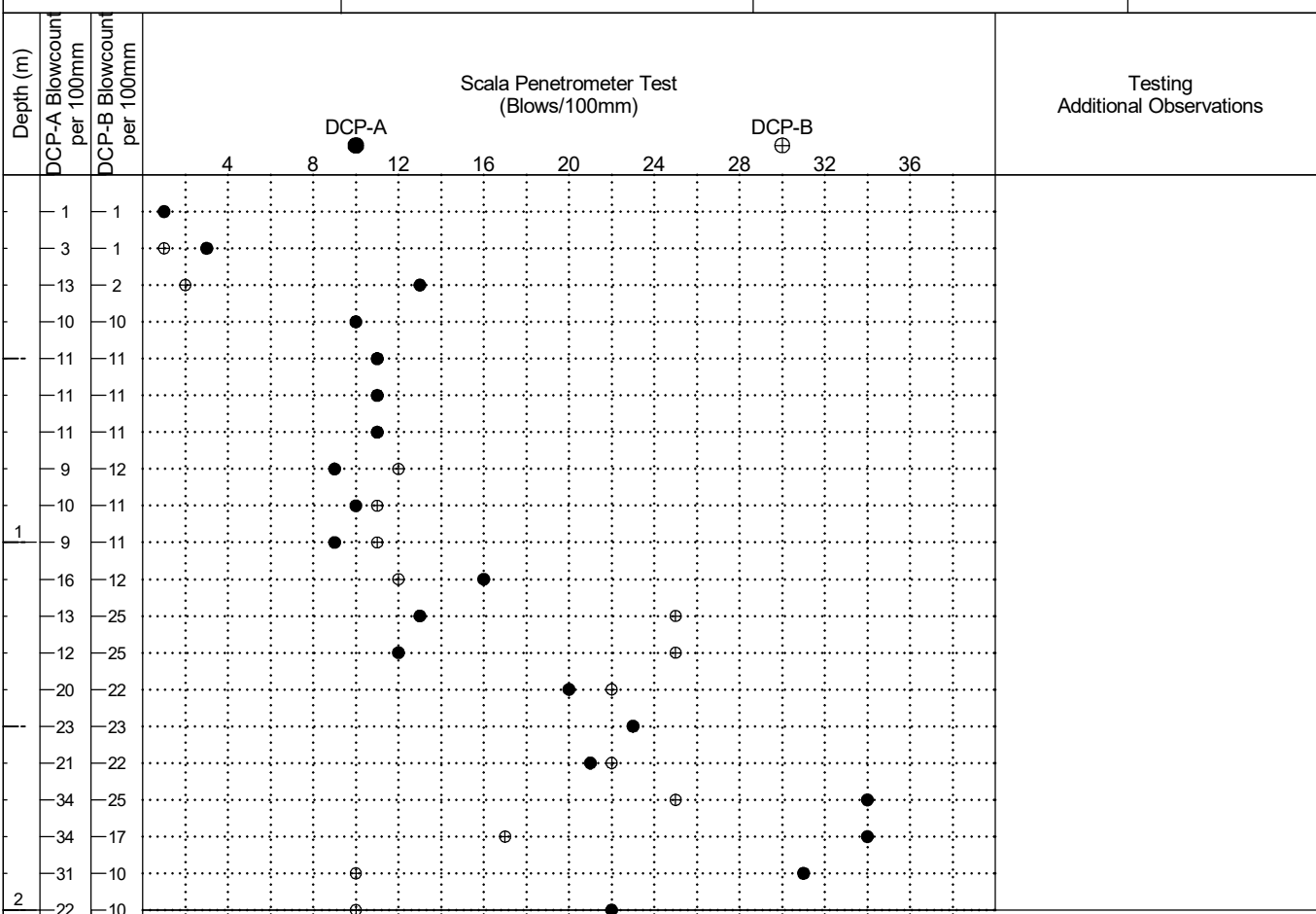
Method: Scala
Equipment: Scala
Contractor: Aurecon

CO-ORDINATES: N/A

Easting: N/A
Northing: N/A
Ground level: N/A

Date started: 19/12/2018
Date completed: 19/12/2018
Inclination: 90°
Orientation: N/A

Logged by: AY
Input by: AY
Checked by: MB
Verified by: RG



Remarks:
*Note: Scalas were offset from the Hand Auger conducted in the centre of the lot by ~5 metres on either side

Water Level Readings:
Date Time | Hole Depth | Water Level
No water level recorded



HAND AUGER LOG

HOLE Id: Lot 473

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI										LOCATION: Terrace View, Papamoa										JOB No.: 1009168.0000									
CO-ORDINATES: (NZTM2000)										DRILL TYPE: 50mm Hand Auger										HOLE STARTED: 08/01/2019 HOLE FINISHED: 08/01/2019									
R.L.:										DRILL METHOD: HA										DRILLED BY: GEOTECHNICS									
DATUM:																				LOGGED BY: PIFA CHECKED: RWM									
GEOLOGICAL															ENGINEERING DESCRIPTION														
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.															Description and Additional Observations														
WATER															MOISTURE / WEATHERING														
CORE RECOVERY (%)															STRENGTH/DENSITY														
METHOD															CLASSIFICATION														
SCALA PENETROMETER (Blows/50mm)															SHEAR STRENGTH (kPa)														
1 2 3 4 5 6 7 8 9															10 25 50 100 200														
TESTS																													
SAMPLES																													
RL (m)																													
DEPTH (m)																													
GRAPHIC LOG																													
D-M															St														
M															D														
VD																													
D																													
M-W																													
VD																													
W																													
2.0																													
DRY 08/01/2019																													
100																													
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5																													
15 >>																													
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18 >>																													
18 >>																													
23 >>																													
24 >>																													
															End of borehole at 2.00mbgl. No standing groundwater encountered.														
COMMENTS: Scala Penetrometer results obtained from Aurecon.																													
Hole Depth 2m																													



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Client: **Zariba Properties Limited**
Project: **Terrace Views**
Location: **Terrace Views, Papamoa - Stage 3D/3E**
Project Reference: **247593**

Lot 473

Sheet 1 of 1

DRILLING INFORMATION

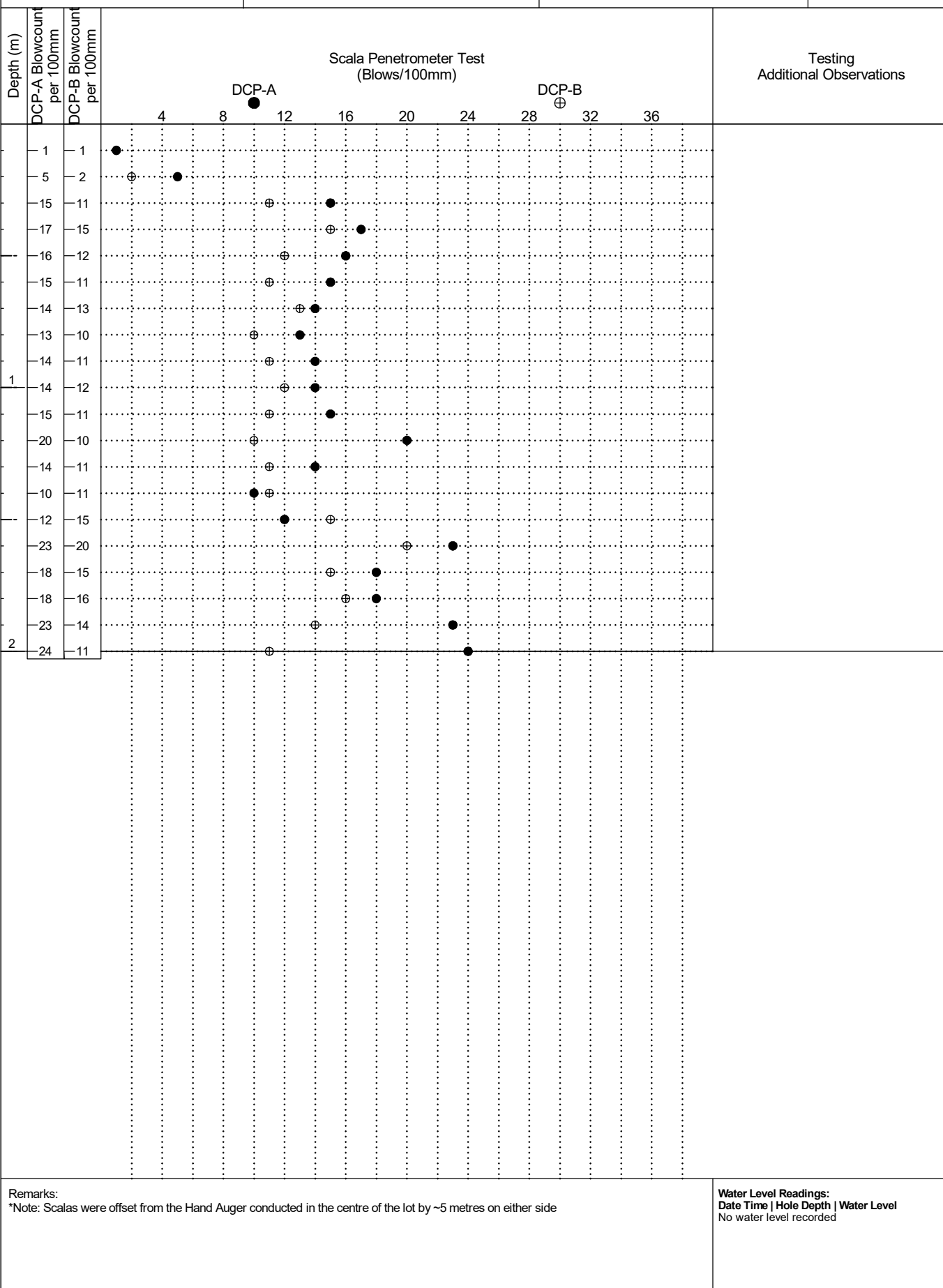
Method: Scala
Equipment: Scala
Contractor: Aurecon

CO-ORDINATES: N/A

Easting: N/A
Northing: N/A
Ground level: N/A

Date started: 19/12/2018
Date completed: 19/12/2018
Inclination: 90°
Orientation: N/A

Logged by: AY
Input by: AY
Checked by: MB
Verified by: RG





HAND AUGER LOG

HOLE Id: Lot 474

Hole Location: See Location Plan

SHEET: 1 OF 1

[illegible]



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Client: **Zariba Properties Limited**
Project: **Terrace Views**
Location: **Terrace Views, Papamoa - Stage 3D/3E**
Project Reference: **247593**

Lot 474

Sheet 1 of 1

DRILLING INFORMATION

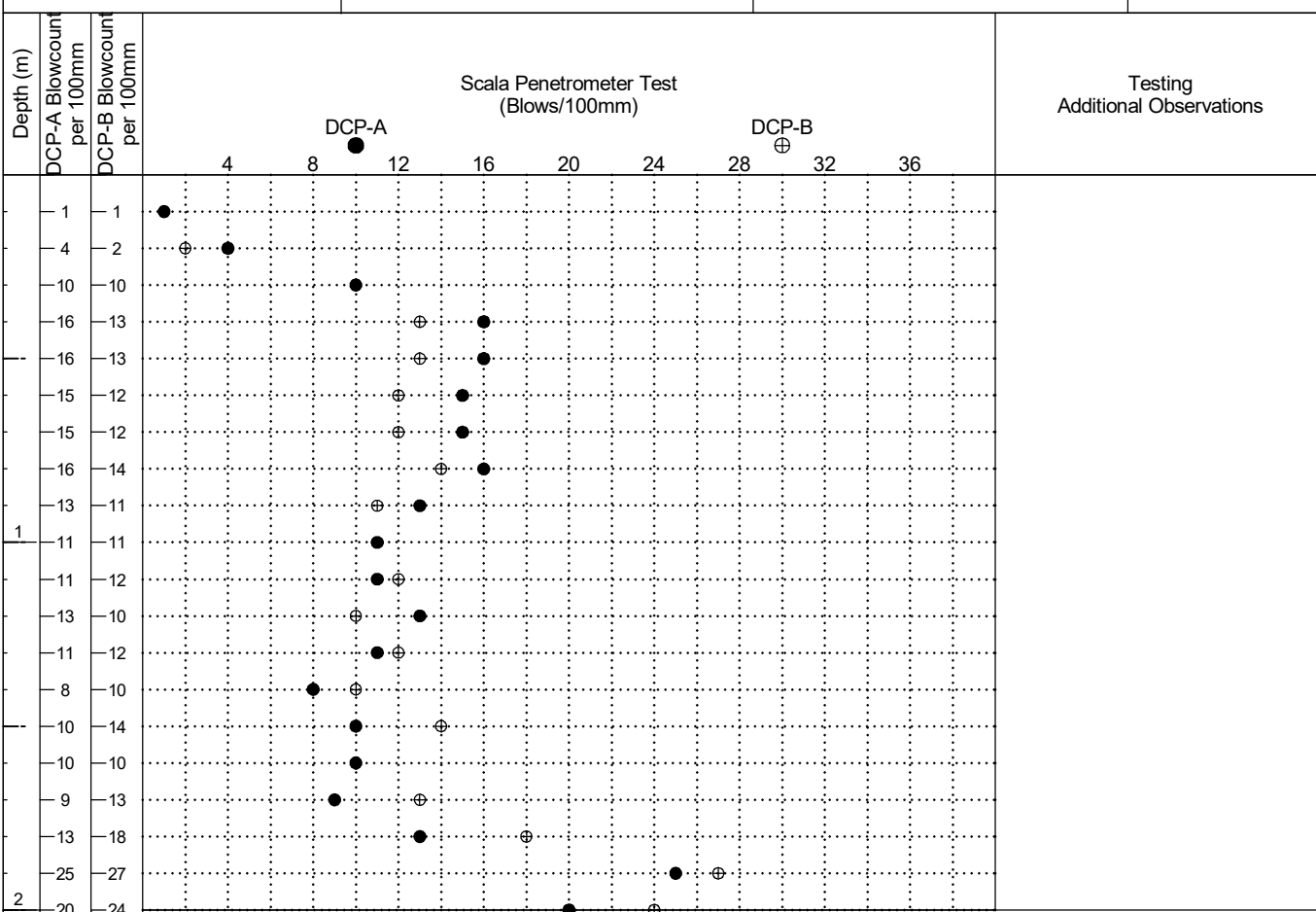
Method: Scala
Equipment: Scala
Contractor: Aurecon

CO-ORDINATES: N/A

Easting: N/A
Northing: N/A
Ground level: N/A

Date started: 19/12/2018
Date completed: 19/12/2018
Inclination: 90°
Orientation: N/A

Logged by: AY
Input by: AY
Checked by: MB
Verified by: RG



Remarks:
*Note: Scalas were offset from the Hand Auger conducted in the centre of the lot by ~5 metres on either side

Water Level Readings:
Date Time | Hole Depth | Water Level
No water level recorded



HAND AUGER LOG

HOLE Id: Lot 475

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI				LOCATION: Terrace View, Papamoa				JOB No.: 1009168.0000										
CO-ORDINATES: (NZTM2000)				DRILL TYPE: 50mm Hand Auger				HOLE STARTED: 08/01/2019										
R.L.:				DRILL METHOD: HA				HOLE FINISHED: 08/01/2019										
DATUM:								DRILLED BY: GEOTECHNICS										
								LOGGED BY: PIFA										
								CHECKED: RWM										
GEOLOGICAL								ENGINEERING DESCRIPTION										
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.		WATER	CORE RECOVERY (%)	METHOD	SCALA PENETROMETER (Blows/100mm)				TESTS	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	MOISTURE / WEATHERING CONDITION	STRENGTH / DENSITY CLASSIFICATION	SHEAR STRENGTH (kPa)	Description and Additional Observations	
					1	2	3	4	5	6	7	8	9					
					1	4											Organic SILT, with some sand; dark brownish black. Stiff, moist to wet, low plastic; sand, fine to medium.	
						4											Fine to coarse SAND, with trace silt and gravel; light greyish brown. Dense, moist, well graded; gravel, fine to medium, subangular to subrounded, pumiceous.	
									11		>>							
									17		>>							
									14		>>							
									9									
									7									
									12		>>							
									12		>>							
									10									
									12		>>							
									15		>>							
									16		>>							
									14		>>							1.4m: Changes to light brown.
									9									
									9									1.6m: Changes to light greyish brown.
									5									1.7m: Changes to medium dense.
									10									1.8m: Changes to dense.
									15		>>							
																		End of borehole at 2.00mbgl.
																		No standing groundwater encountered.
COMMENTS: Scala Penetrometer results obtained from Aurecon.																		
Hole Depth 2m																		



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Client: **Zariba Properties Limited**
Project: **Terrace Views**
Location: **Terrace Views, Papamoa - Stage 3D/3E**
Project Reference: **247593**

Lot 475

Sheet 1 of 1

DRILLING INFORMATION

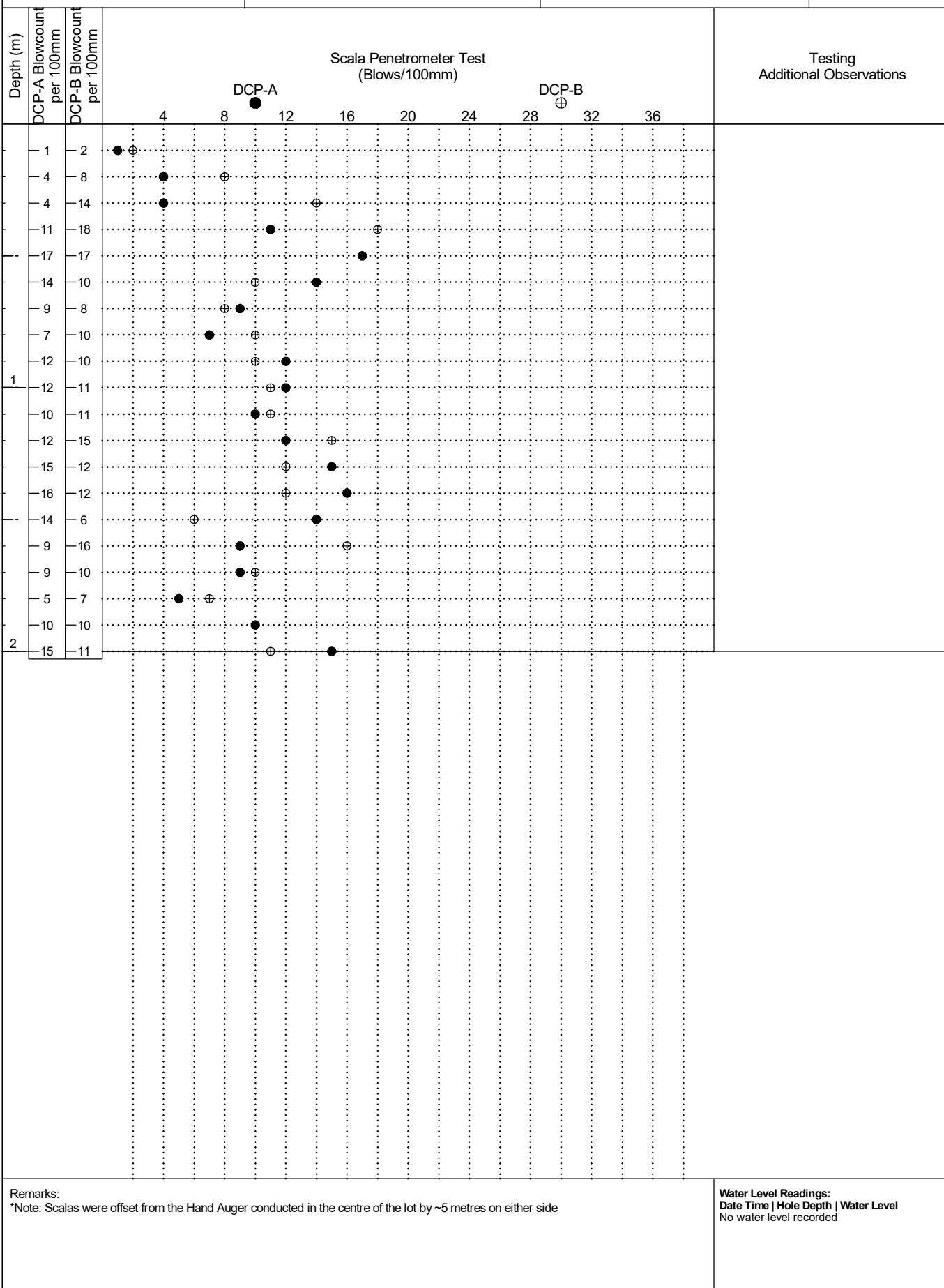
Method: Scala
Equipment: Scala
Contractor: Aurecon

CO-ORDINATES: N/A

Easting: N/A
Northing: N/A
Ground level: N/A

Date started: 18/01/2019
Date completed: 18/01/2019
Inclination: 90°
Orientation: N/A

Logged by: AC
Input by: AY
Checked by: MB
Verified by: RG





HAND AUGER LOG

HOLE Id: Lot 476

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI										LOCATION: Terrace View, Papamoa										JOB No.: 1009168.0000																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																													
CO-ORDINATES: (NZTM2000)										DRILL TYPE: 50mm Hand Auger										HOLE STARTED: 08/01/2019																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																													
R.L.:										DRILL METHOD: HA										HOLE FINISHED: 08/01/2019																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																													
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GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.															WATER	CORE RECOVERY (%)	METHOD	SCALA PENETROMETER (Blows/100mm)									TESTS	SAMPLES	RL (m)	DEPTH (m)	GRAPHIC LOG	MOISTURE / WEATHERING CONDITION	STRENGTH DENSITY CLASSIFICATION	SHEAR STRENGTH (kPa)				Description and Additional Observations																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																											
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Client: **Zariba Properties Limited**
Project: **Terrace Views**
Location: **Terrace Views, Papamoa - Stage 3D/3E**
Project Reference: **247593**

Lot 476

Sheet 1 of 1

DRILLING INFORMATION

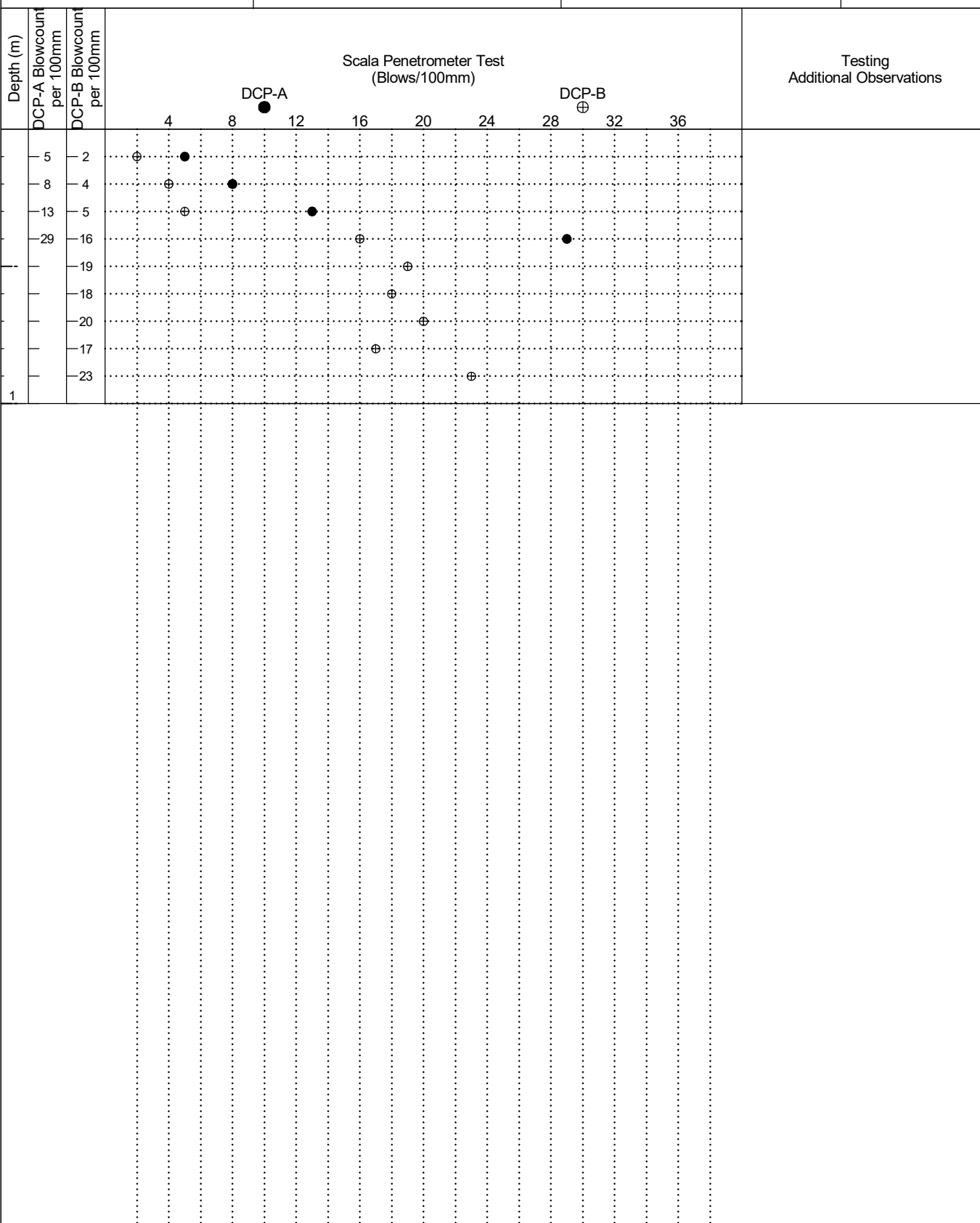
Method: Scala
Equipment: Scala
Contractor: Aurecon

CO-ORDINATES: N/A

Easting: N/A
Northing: N/A
Ground level: N/A

Date started: 18/01/2019
Date completed: 18/01/2019
Inclination: 90°
Orientation: N/A

Logged by: AC
Input by: AY
Checked by: MB
Verified by: RG



Remarks:

*Note: Scalas were offset from the Hand Auger conducted in the centre of the lot by ~5 metres on either side

Water Level Readings:

Date Time | Hole Depth | Water Level
No water level recorded



HAND AUGER LOG

HOLE Id: Lot 477

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI				LOCATION: Terrace View, Papamoa				JOB No.: 1009168.0000											
CO-ORDINATES: (NZTM2000)				DRILL TYPE: 50mm Hand Auger				HOLE STARTED: 08/01/2019											
R.L.:				DRILL METHOD: HA				HOLE FINISHED: 08/01/2019											
DATUM:								DRILLED BY: GEOTECHNICS											
								LOGGED BY: PIFA											
								CHECKED: RWM											
GEOLOGICAL										ENGINEERING DESCRIPTION									
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION: WATER CORE RECOVERY (%) METHOD SCALA PENETROMETER (Blows/100mm) 1 2 3 4 5 6 7 8 9 TESTS SAMPLES RL (m) DEPTH (m) GRAPHIC LOG MOISTURE CONDITION STRENGTH/PLASTICITY CLASSIFICATION 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150 160 170 180 190 200 210 220 230 240 250 260 270 280 290 300 310 320 330 340 350 360 370 380 390 400 410 420 430 440 450 460 470 480 490 500 510 520 530 540 550 560 570 580 590 600 610 620 630 640 650 660 670 680 690 700 710 720 730 740 750 760 770 780 790 800 810 820 830 840 850 860 870 880 890 900 910 920 930 940 950 960 970 980 990 1000 1010 1020 1030 1040 1050 1060 1070 1080 1090 1100 1110 1120 1130 1140 1150 1160 1170 1180 1190 1200 1210 1220 1230 1240 1250 1260 1270 1280 1290 1300 1310 1320 1330 1340 1350 1360 1370 1380 1390 1400 1410 1420 1430 1440 1450 1460 1470 1480 1490 1500 1510 1520 1530 1540 1550 1560 1570 1580 1590 1600 1610 1620 1630 1640 1650 1660 1670 1680 1690 1700 1710 1720 1730 1740 1750 1760 1770 1780 1790 1800 1810 1820 1830 1840 1850 1860 1870 1880 1890 1900 1910 1920 1930 1940 1950 1960 1970 1980 1990 2000 2010 2020 2030 2040 2050 2060 2070 2080 2090 2100 2110 2120 2130 2140 2150 2160 2170 2180 2190 2200 2210 2220 2230 2240 2250 2260 2270 2280 2290 2300 2310 2320 2330 2340 2350 2360 2370 2380 2390 2400 2410 2420 2430 2440 2450 2460 2470 2480 2490 2500 2510 2520 2530 2540 2550 2560 2570 2580 2590 2600 2610 2620 2630 2640 2650 2660 2670 2680 2690 2700 2710 2720 2730 2740 2750 2760 2770 2780 2790 2800 2810 2820 2830 2840 2850 2860 2870 2880 2890 2900 2910 2920 2930 2940 2950 2960 2970 2980 2990 3000 3010 3020 3030 3040 3050 3060 3070 3080 3090 3100 3110 3120 3130 3140 3150 3160 3170 3180 3190 3200 3210 3220 3230 3240 3250 3260 3270 3280 3290 3300 3310 3320 3330 3340 3350 3360 3370 3380 3390 3400 3410 3420 3430 3440 3450 3460 3470 3480 3490 3500 3510 3520 3530 3540 3550 3560 3570 3580 3590 3600 3610 3620 3630 3640 3650 3660 3670 3680 3690 3700 3710 3720 3730 3740 3750 3760 3770 3780 3790 3800 3810 3820 3830 3840 3850 3860 3870 3880 3890 3900 3910 3920 3930 3940 3950 3960 3970 3980 3990 4000 4010 4020 4030 4040 4050 4060 4070 4080 4090</																			



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Client: **Zariba Properties Limited**
Project: **Terrace Views**
Location: **Terrace Views, Papamoa - Stage 3D/3E**
Project Reference: **247593**

Lot 477

Sheet 1 of 1

DRILLING INFORMATION

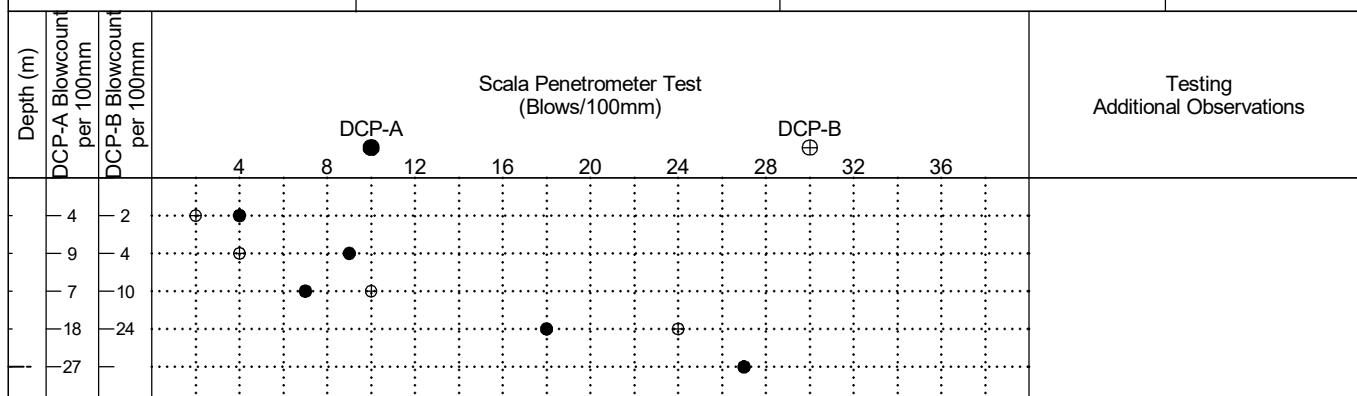
Method: Scala
Equipment: Scala
Contractor: Aurecon

CO-ORDINATES: N/A

Easting: N/A
Northing: N/A
Ground level: N/A

Date started: 18/01/2019
Date completed: 18/01/2019
Inclination: 90°
Orientation: N/A

Logged by: AC
Input by: AY
Checked by: MB
Verified by: RG



Remarks:
*Note: Scalas were offset from the Hand Auger conducted in the centre of the lot by ~5 metres on either side

Water Level Readings:
Date Time | Hole Depth | Water Level
No water level recorded



HAND AUGER LOG

HOLE Id: Lot 478

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI										LOCATION: Terrace View, Papamoa										JOB No.: 1009168.0000									
CO-ORDINATES: (NZTM2000)										DRILL TYPE: 50mm Hand Auger										HOLE STARTED: 08/01/2019 HOLE FINISHED: 08/01/2019 DRILLED BY: GEOTECHNICS LOGGED BY: PIFA CHECKED: RWM									
R.L.:										DRILL METHOD: HA																			
DATUM:																													
GEOLOGICAL															ENGINEERING DESCRIPTION														
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.															Description and Additional Observations														
WATER															CORE RECOVERY (%)														
METHOD															SCALA PENETROMETER (Blows/100mm)														
1															2														
2															3														
3															4														
4															5														
5															6														
6															7														
7															8														
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27 >>																													
</																													



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Client: **Zariba Properties Limited**
Project: **Terrace Views**
Location: **Terrace Views, Papamoa - Stage 3D/3E**
Project Reference: **247593**

Lot 478

Sheet 1 of 1

DRILLING INFORMATION

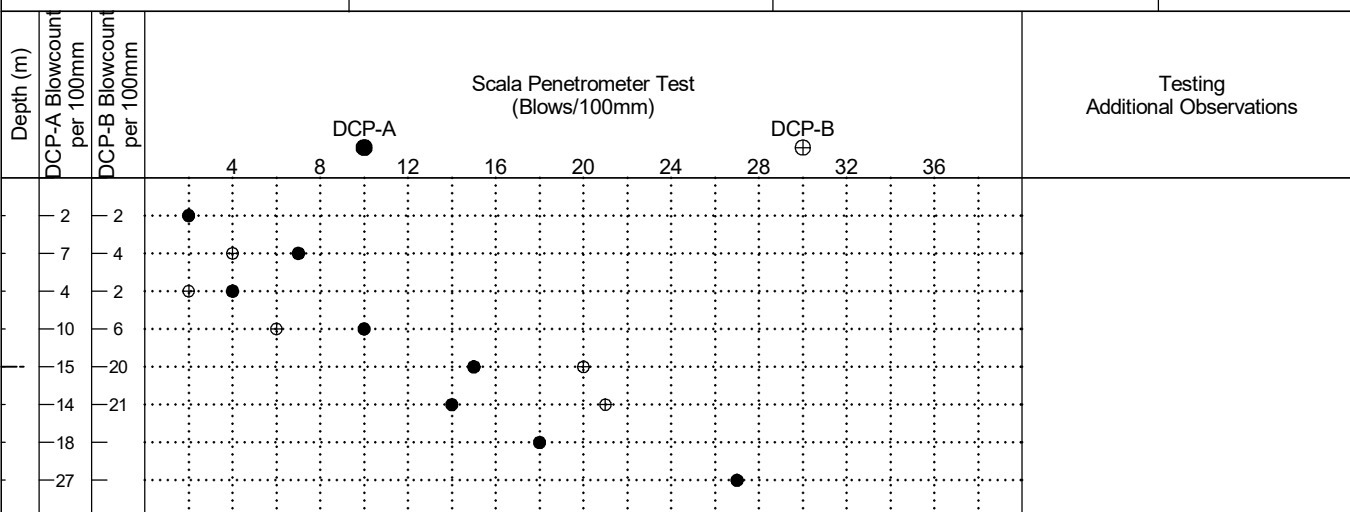
Method: Scala
Equipment: Scala
Contractor: Aurecon

CO-ORDINATES: N/A

Easting: N/A
Northing: N/A
Ground level: N/A

Date started: 18/01/2019
Date completed: 18/01/2019
Inclination: 90°
Orientation: N/A

Logged by: AC
Input by: AY
Checked by: MB
Verified by: RG



Remarks:
*Note: Scalas were offset from the Hand Auger conducted in the centre of the lot by ~5 metres on either side

Water Level Readings:
Date Time | Hole Depth | Water Level
No water level recorded



HAND AUGER LOG

HOLE Id: Lot 479

Hole Location: See Location Plan

SHEET: 1 OF 1

[illegible]



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Client: **Zariba Properties Limited**
Project: **Terrace Views**
Location: **Terrace Views, Papamoa - Stage 3D/3E**
Project Reference: **247593**

Lot 479

Sheet 1 of 1

DRILLING INFORMATION

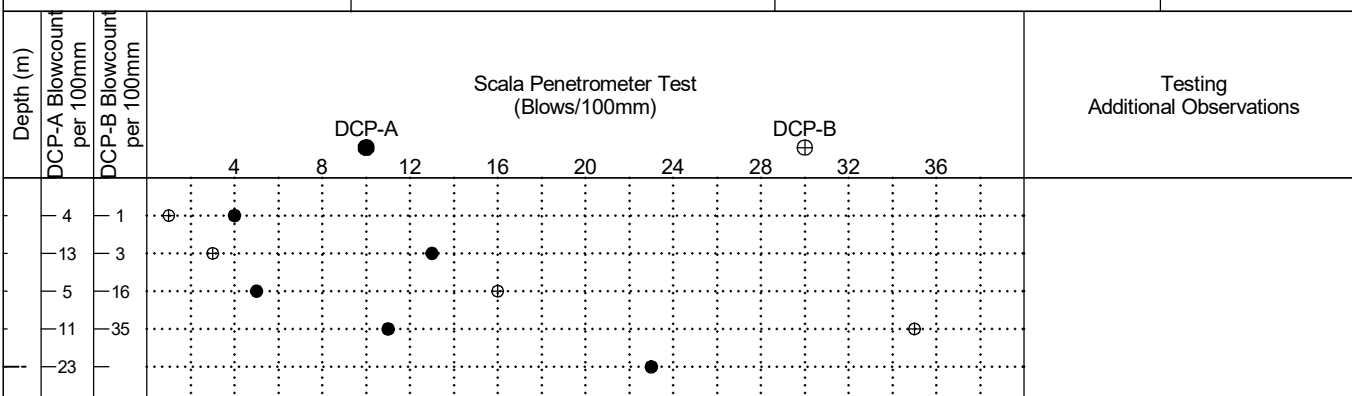
Method: Scala
Equipment: Scala
Contractor: Aurecon

CO-ORDINATES: N/A

Easting: N/A
Northing: N/A
Ground level: N/A

Date started: 18/01/2019
Date completed: 18/01/2019
Inclination: 90°
Orientation: N/A

Logged by: AC
Input by: AY
Checked by: MB
Verified by: RG



Remarks:
*Note: Scalas were offset from the Hand Auger conducted in the centre of the lot by ~5 metres on either side

Water Level Readings:
Date Time | Hole Depth | Water Level
No water level recorded



HAND AUGER LOG

HOLE Id: Lot 480

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI										LOCATION: Terrace View, Papamoa										JOB No.: 1009168.0000									
CO-ORDINATES: (NZTM2000)										DRILL TYPE: 50mm Hand Auger										HOLE STARTED: 08/01/2019 HOLE FINISHED: 08/01/2019									
R.L.:										DRILL METHOD: HA										DRILLED BY: GEOTECHNICS									
DATUM:																				LOGGED BY: PIFA CHECKED: RWM									
GEOLOGICAL															ENGINEERING DESCRIPTION														
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.															Description and Additional Observations														
WATER															SAMPLES														
CORE RECOVERY (%)															RL (m)														
METHOD															DEPTH (m)														
SCALA PENETROMETER (Blows/100mm)															GRAPHIC LOG														
1 2 3 4 5 6 7 8 9															MOISTURE / WEATHERING CONDITION														
															STRENGTH CLASSIFICATION														
															10 20 30 40 SHEAR STRENGTH (kPa)														
3															Organic SILT, with some sand, trace gravel; dark brownish black. Stiff, moist, non-plastic; sand, fine to medium; gravel, fine, subangular to subrounded, pumiceous.														
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7																													
12 >>																													
14 >>																													
24 >>															M-W VD														



Ground Floor, 247 Cameron Road
PO Box 2292, Tauranga
New Zealand
Tel: +64 7 578 6183
www.aurecongroup.com

Client: **Zariba Properties Limited**
Project: **Terrace Views**
Location: **Terrace Views, Papamoa - Stage 3D/3E**
Project Reference: **247593**

Lot 480

Sheet 1 of 1

DRILLING INFORMATION

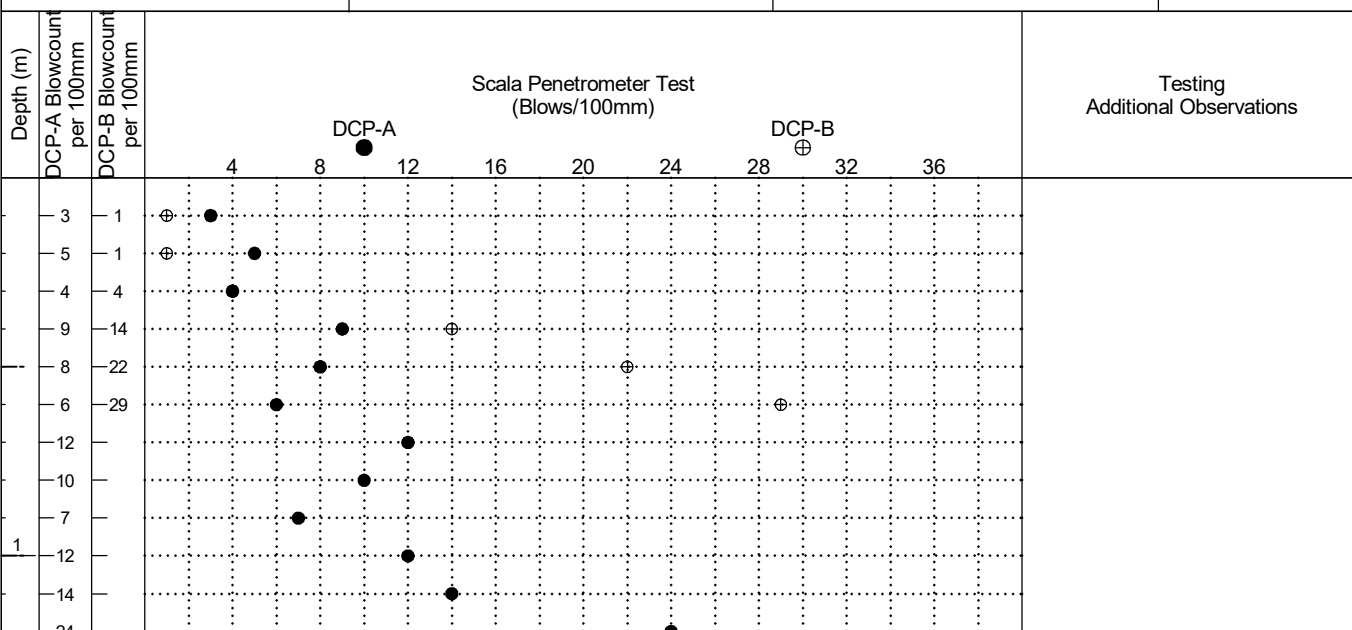
Method: Scala
Equipment: Scala
Contractor: Aurecon

CO-ORDINATES: N/A

Easting: N/A
Northing: N/A
Ground level: N/A

Date started: 18/01/2019
Date completed: 18/01/2019
Inclination: 90°
Orientation: N/A

Logged by: AC
Input by: MB
Checked by: MB
Verified by: RG



Remarks:
*Note: Scalas were offset from the Hand Auger conducted in the centre of the lot by ~5 metres on either side

Water Level Readings:
Date Time | Hole Depth | Water Level
No water level recorded



HAND AUGER LOG

HOLE Id: Lot 481

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI										LOCATION: Terrace View, Papamoa										JOB No.: 1009168.0000																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																		
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Project Reference: **247593**

Lot 481

Sheet 1 of 1

DRILLING INFORMATION

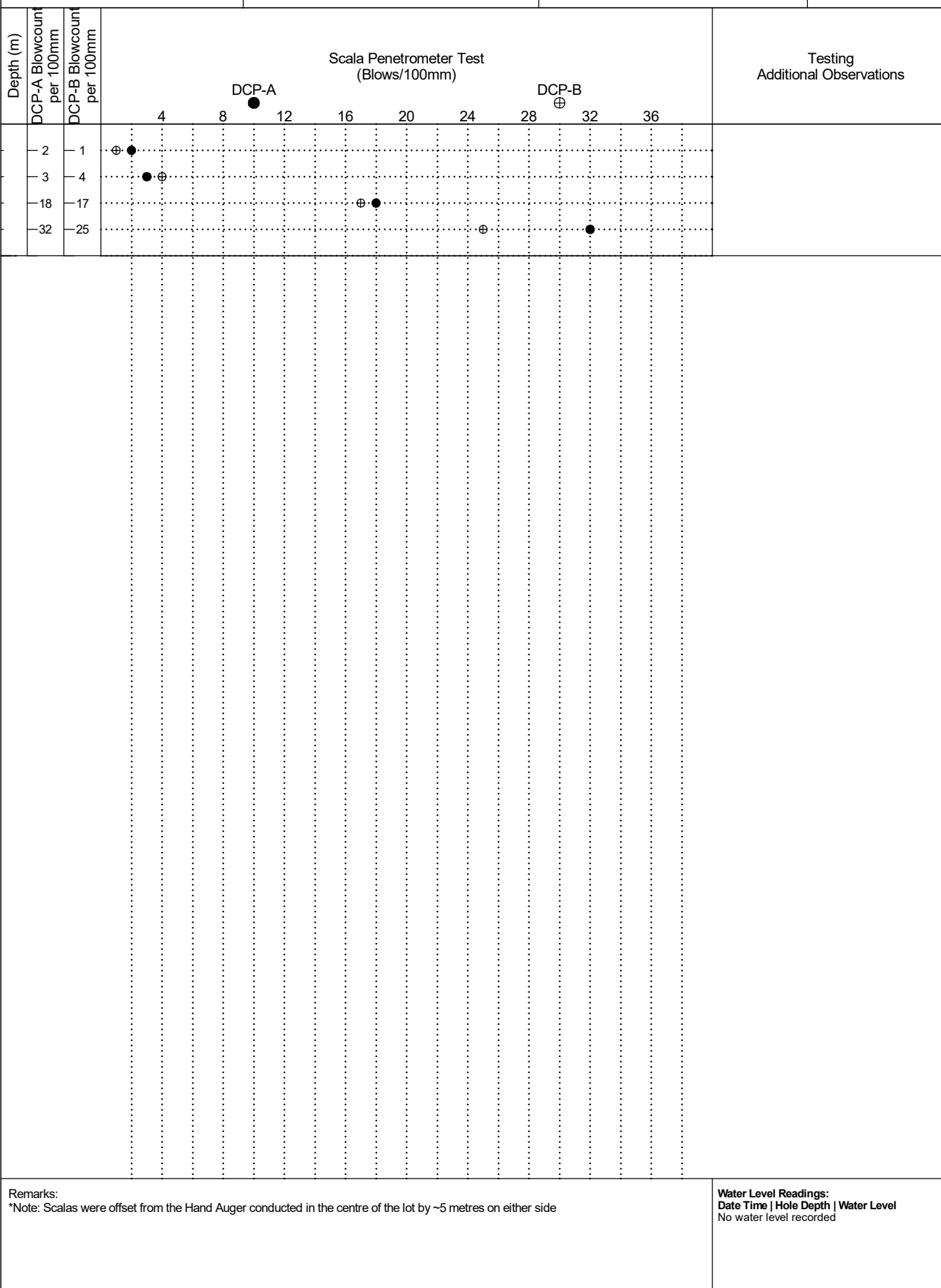
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Contractor: Aurecon

CO-ORDINATES: N/A

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Inclination: 90°
Orientation: N/A

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HAND AUGER LOG

HOLE Id: Lot 482

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI										LOCATION: Terrace View, Papamoa										JOB No.: 1009168.0000																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																											
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Client: **Zariba Properties Limited**
Project: **Terrace Views**
Location: **Terrace Views, Papamoa - Stage 3D/3E**
Project Reference: **247593**

Lot 482

Sheet 1 of 1

DRILLING INFORMATION

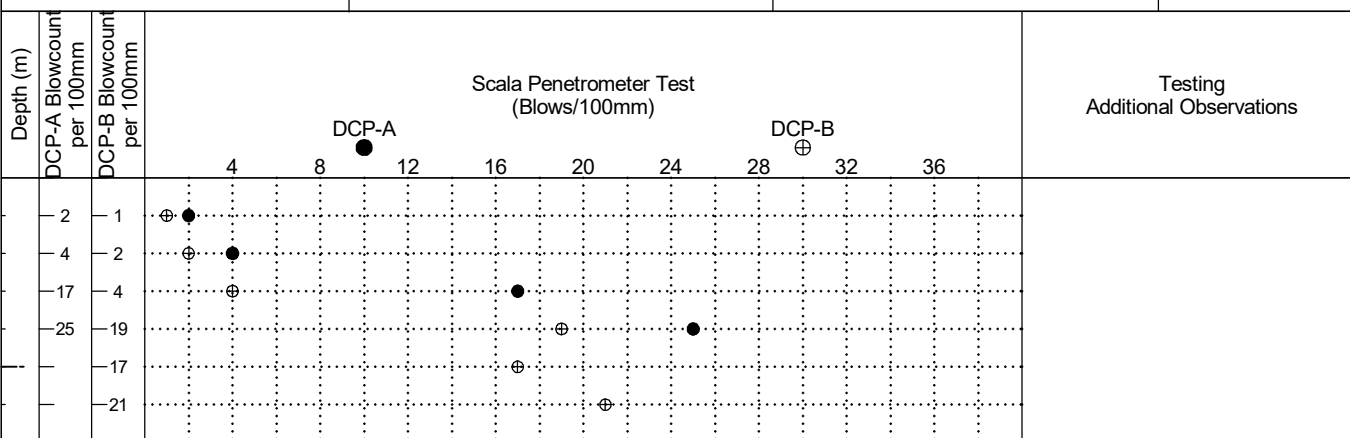
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Contractor: Aurecon

CO-ORDINATES: N/A

Easting: N/A
Northing: N/A
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Date started: 18/01/2019
Date completed: 18/01/2019
Inclination: 90°
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Logged by: AC
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Remarks:
*Note: Scalas were offset from the Hand Auger conducted in the centre of the lot by ~5 metres on either side

Water Level Readings:
Date Time | Hole Depth | Water Level
No water level recorded



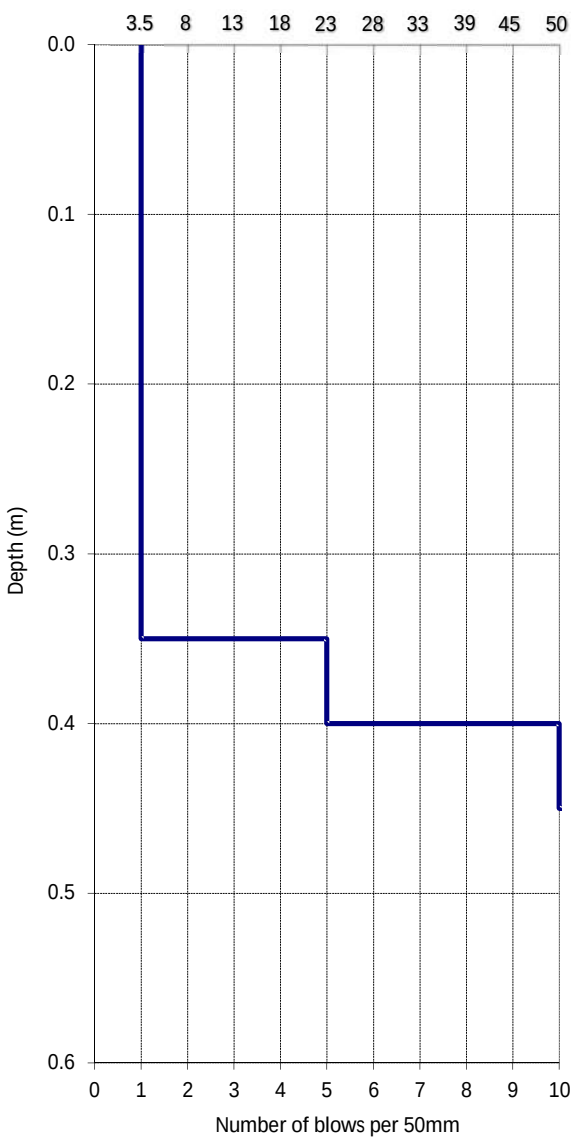
HAND AUGER LOG

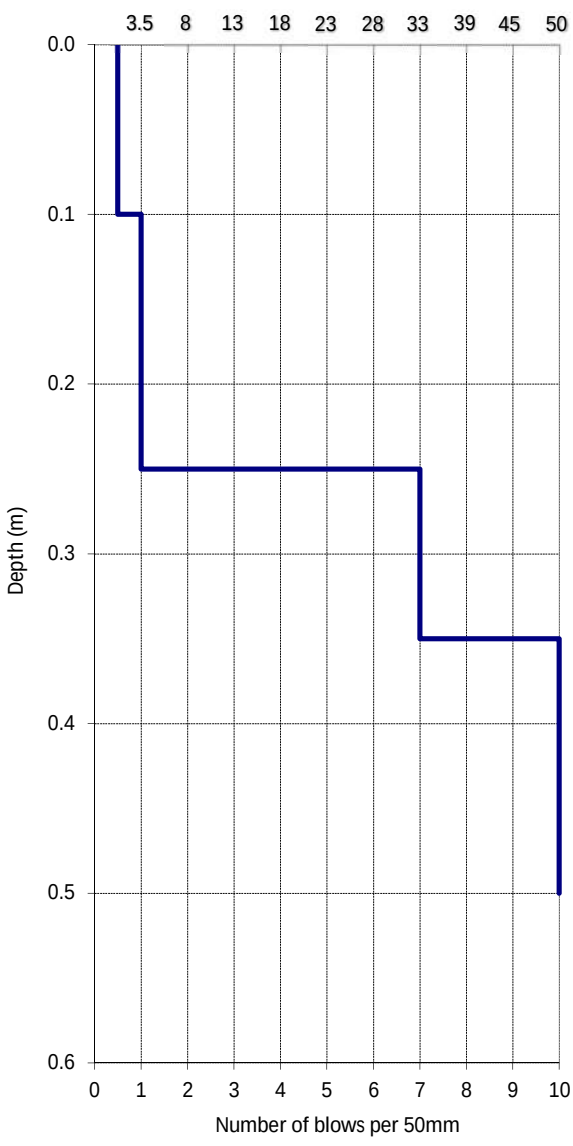
HOLE Id: Lot 483

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI						LOCATION: Terrace View, Papamoa						JOB No.: 1009168.0000											
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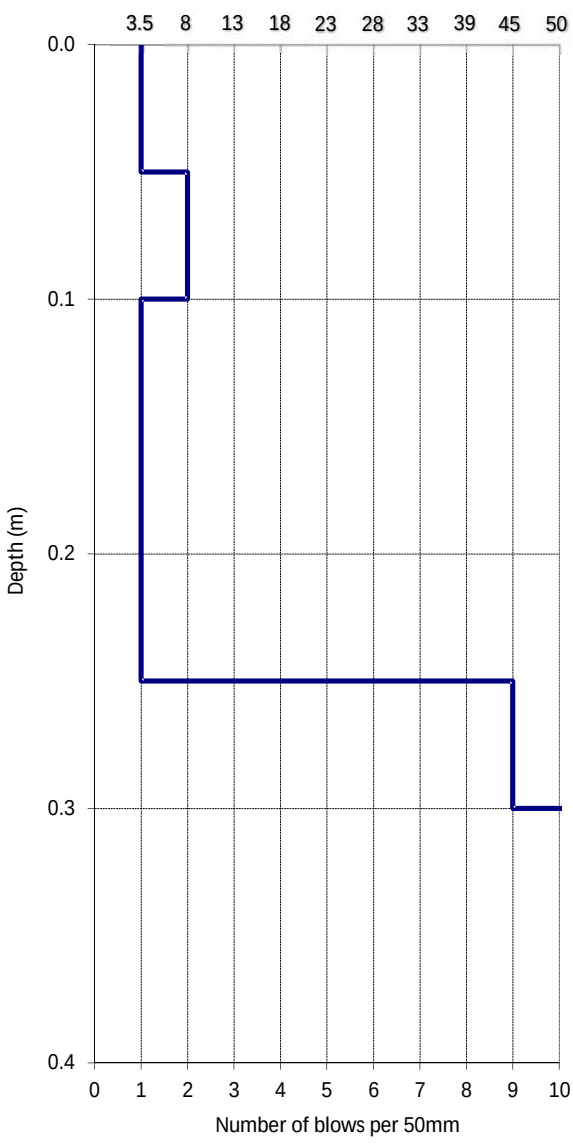
HAND AUGER LOG

HOLE Id: Lot 484

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI										LOCATION: Terrace View, Papamoa										JOB No.: 1009168.0000																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																							
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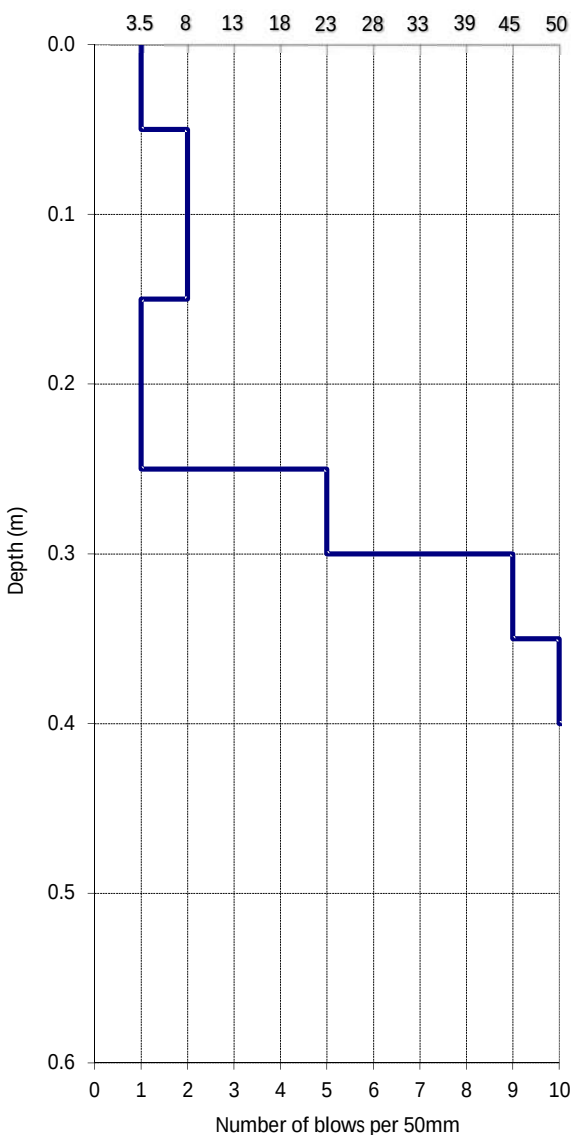
HAND AUGER LOG

HOLE Id: **Lot 485**

Hole Location: See Location Plan

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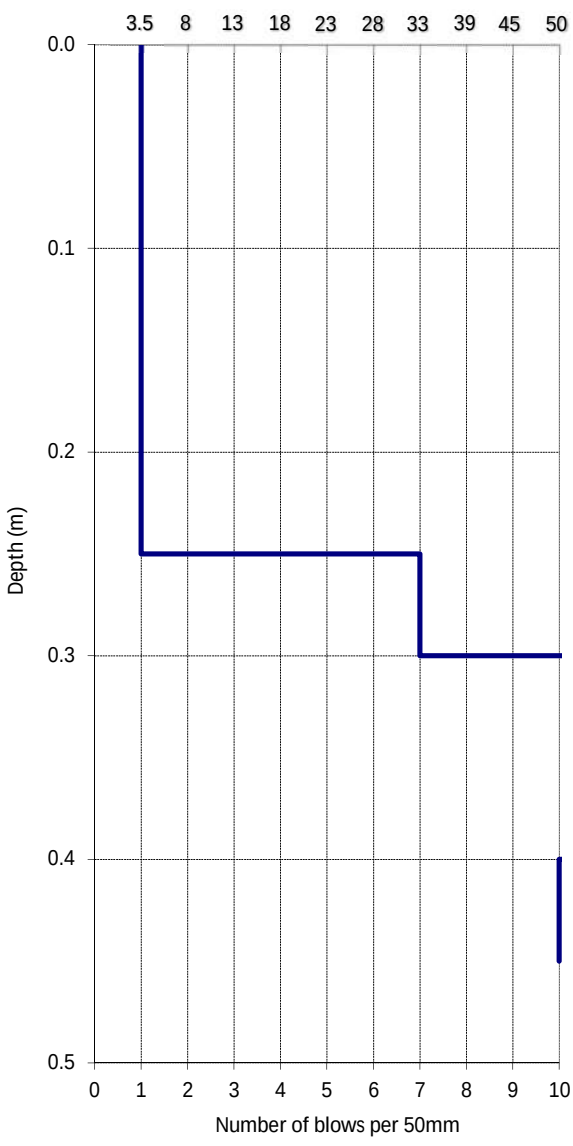
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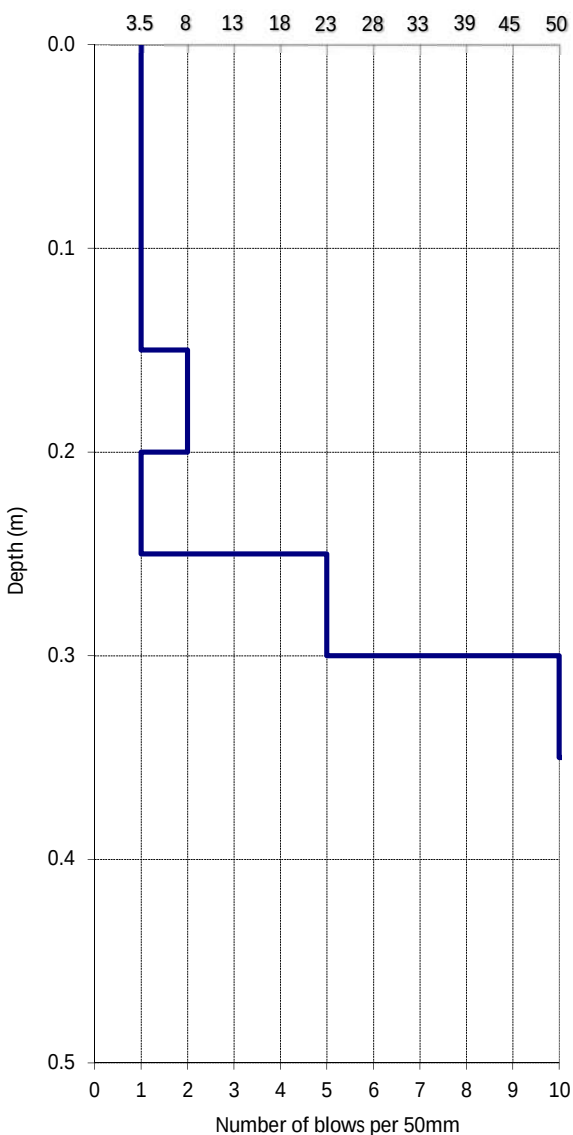
HOLE Id: Lot 486

Hole Location: See Location Plan

SHEET: 1 OF 1

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Site Location		Papamoa		Material Source	Unknown																																																																																																																																				
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HAND AUGER LOG

HOLE Id: **Lot 487**

Hole Location: See Location Plan

SHEET: 1 OF 1

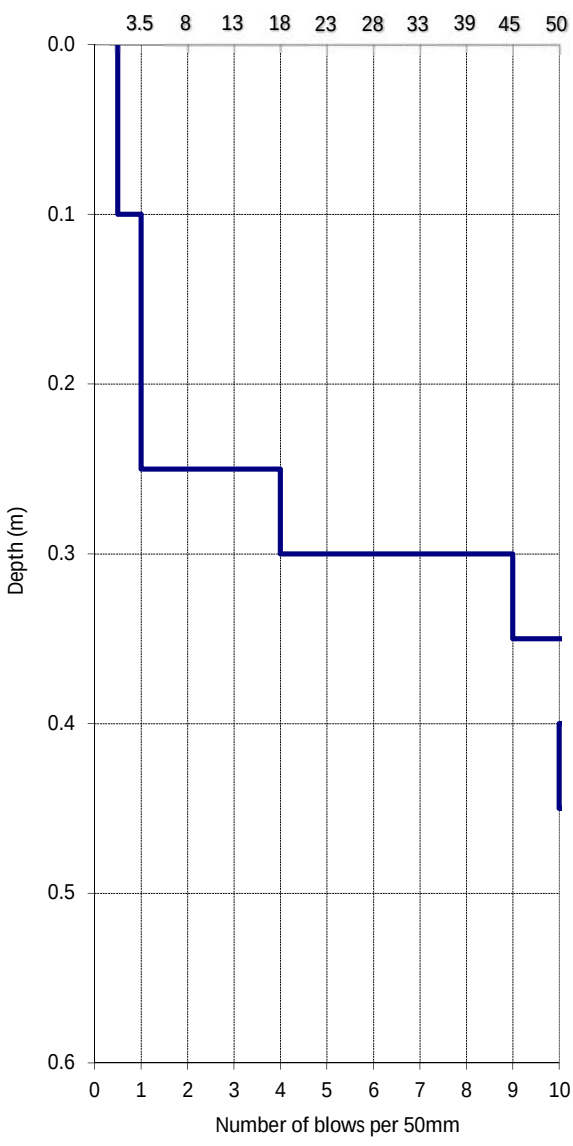
PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI										LOCATION: Terrace View, Papamoa										JOB No.: 1009168.0000																			
CO-ORDINATES: (NZTM2000)										DRILL TYPE: 50mm Hand Auger										HOLE STARTED: 08/01/2019																			
R.L.:										DRILL METHOD: HA										HOLE FINISHED: 08/01/2019																			
DATUM:										LOGGED BY: PIFA										CHECKED: RWM																			
GEOLOGICAL										ENGINEERING DESCRIPTION																													
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION										WATER CORE RECOVERY (%) METHOD SCALA PENETROMETER (Blows/50mm) TESTS SAMPLES RL (m) DEPTH (m) GRAPHIC LOG MOISTURE CONDITION WEATHERING STRENGTH/DENSITY CLASSIFICATION SHEAR STRENGTH (kPa)										Description and Additional Observations																			
DRY 31/01/2019 100 HA										1 1 1 1 4 9 11 >> 10 11 >>										TS TS M VD M-W W										Organic SILT, with some sand; dark brownish black. Stiff, wet, non-plastic; sand, fine to medium. Fine to coarse SAND, with trace silt and gravel; light brownish grey. Dense, moist, well graded; gravel, fine, subangular to subrounded, pumiceous. 0.3m: Changes to very dense. 0.75m: Changes to moist to wet. 1.15m: Changes to minor silt; light brown. 1.8m: Changes to wet.									
																														End of borehole at 2.00mbgl.									
																														No standing groundwater encountered.									

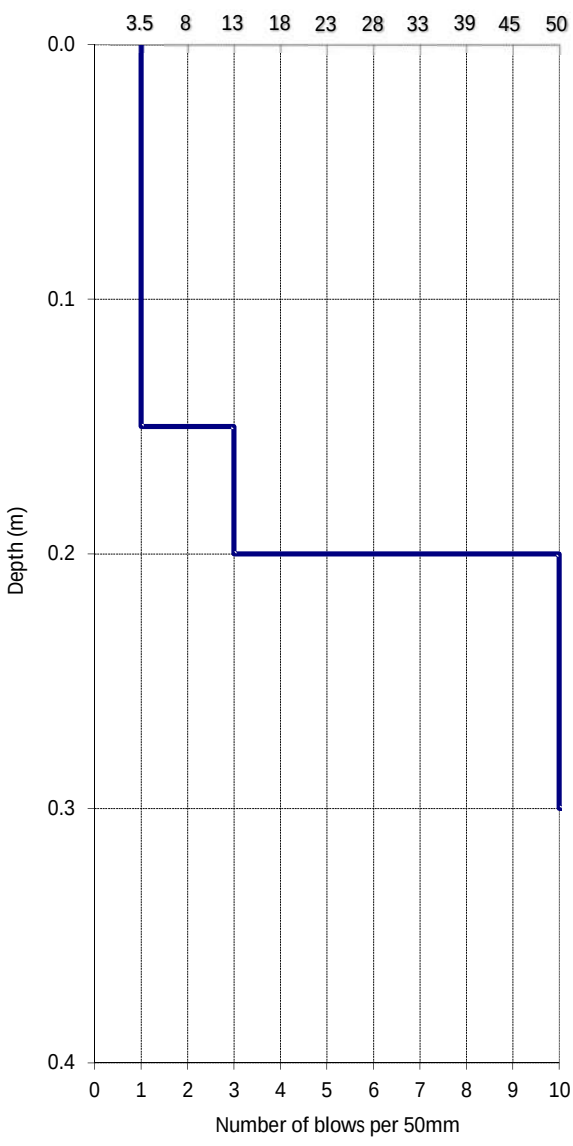
COMMENTS:

 Hole Depth
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Scale 1:13

Rev.: A

 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
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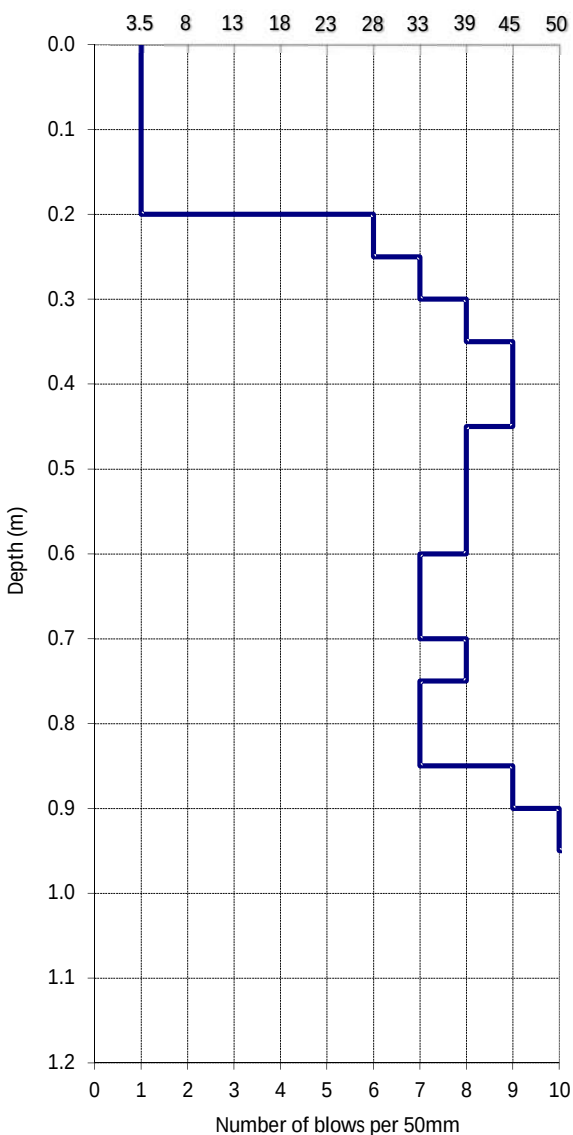
HAND AUGER LOG

HOLE Id: **Lot 488**

Hole Location: See Location Plan

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HAND AUGER LOG

HOLE Id: Lot 489

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI										LOCATION: Terrace View, Papamoa										JOB No.: 1009168.0000									
CO-ORDINATES: (NZTM2000)										DRILL TYPE: 50mm Hand Auger										HOLE STARTED: 08/01/2019 HOLE FINISHED: 08/01/2019 DRILLED BY: GEOTECHNICS LOGGED BY: PIFA CHECKED: RWM									
R.L.:										DRILL METHOD: HA																			
DATUM:																													
GEOLOGICAL															ENGINEERING DESCRIPTION														
GEOLOGICAL UNIT, GENERIC NAME, ORIGIN, MATERIAL COMPOSITION.															Description and Additional Observations														
WATER															CORE RECOVERY (%)														
METHOD															SCALA PENETROMETER (Blows/50mm)														
1															2														
2															3														
3															4														
4															5														
5															6														
6															7														
7															8														
8															9														
9																													
TESTS															SAMPLES														
RL (m)															DEPTH (m)														
GRAPHIC LOG															MOISTURE CONDITION														
WEATHERING															STRENGTH/CASUALTY CLASSIFICATION														
10															SHEAR STRENGTH (kPa)														
20															100														
300															500														
1000																													
															Organic SILT, minor sand; dark brownish black. Stiff, moist, non-plastic; sand, fine to medium.														
															Fine to coarse SAND, with trace silt and gravel; light brownish grey. Dense, moist, well graded; gravel, fine to medium, subangular to subrounded, pumiceous.														
															1.05m: Changes to light yellowish brown; very dense.														
															1.2m: Changes to light brownish grey; moist to wet.														
															1.5m: Changes to light greyish brown.														
															End of borehole at 2.00mbgl. No standing groundwater encountered.														
COMMENTS:																													
Hole Depth 2m																													

 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
			Lab Ref/URN																																																																																																																																					
			023/19																																																																																																																																					
NZS 4402: 1998 Test 6.5.2 Dynamic Cone Penetrometer - Scala																																																																																																																																								
Project Name	TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID	1009168.0.0.0																																																																																																																																				
Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851																																																																																																																																				
Site Location	Papamoa		Material Source	Unknown																																																																																																																																				
Material Description	See Lot 489 borehole log.		Test Series	Terrace Views/160119/SC489-492																																																																																																																																				
Depth from ground surface to commencement of penetration (m)		0.0	Test Number	Lot 489 A																																																																																																																																				
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HAND AUGER LOG

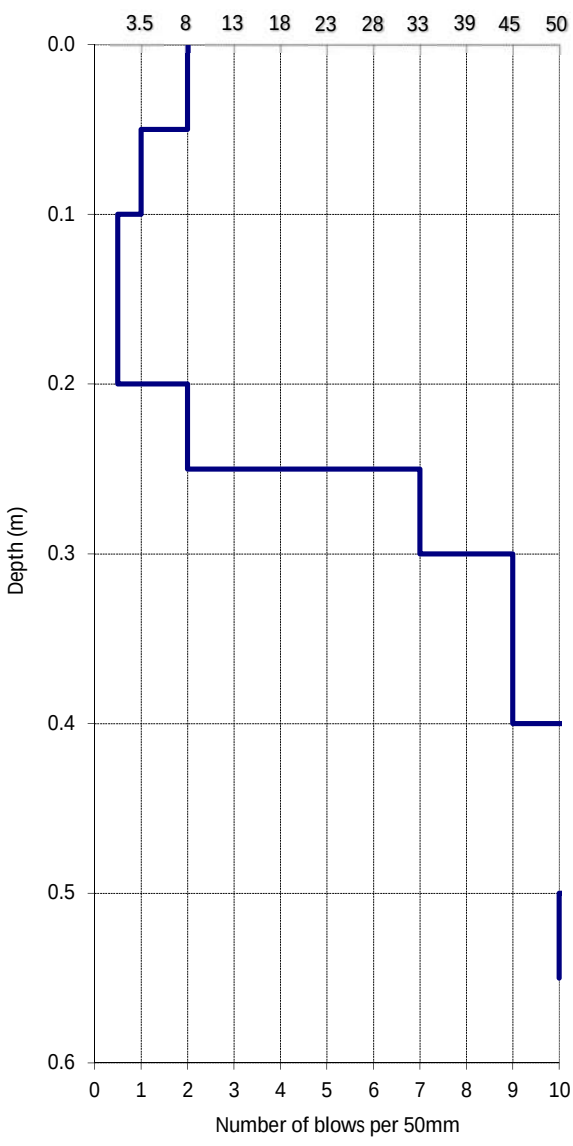
HOLE Id: Lot 490

Hole Location: See Location Plan

SHEET: 1 OF 1

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 GEOTECHNICS	15C Amber Crescent Judea Tauranga 3110 New Zealand p. +64 7 571 0280		Page 1 of 1																																																																																																																																					
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NZS 4402: 1998 Test 6.5.2 Dynamic Cone Penetrometer - Scala																																																																																																																																								
Project Name	TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID	1009168.0.0.0																																																																																																																																				
Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851																																																																																																																																				
Site Location	Papamoa		Material Source	Unknown																																																																																																																																				
Material Description	See Lot 490 borehole log.		Test Series	Terrace Views/160119/SC489-492																																																																																																																																				
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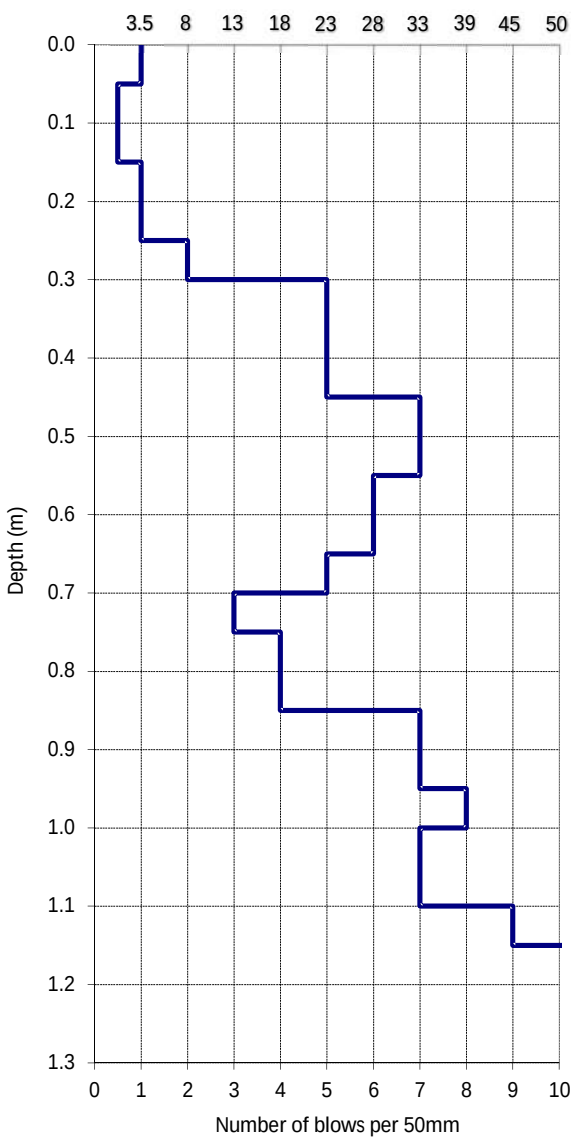
HAND AUGER LOG

HOLE Id: Lot 491

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI										LOCATION: Terrace View, Papamoa										JOB No.: 1009168.0000																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																		
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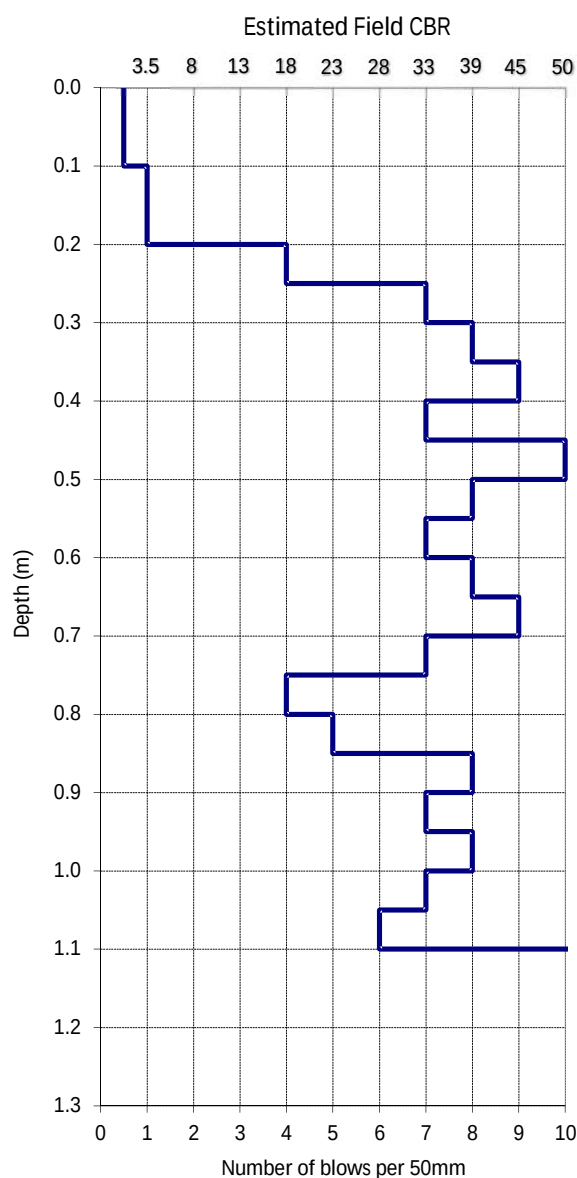
15C Amber Crescent
Judea
Tauranga 3110
New Zealand
p. +64 7 571 0280

Lab Ref/URN

023/19

Project Name	TERRACE VIEWS STAGE 3 D & E - SITE INVESTIGATION		Project ID	1009168.0.0.0
Customer Project ID	Terrace Views Stage 3 D & E		Equipment ID	TG0851
Site Location	Papamoa		Material Source	Unknown
Material Description	See Lot 491 borehole log.		Test Series	Terrace Views/160119/SC489-492
Depth from ground surface to commencement of penetration (m)		0.0	Test Number	Lot 491 B

Chainage		Offset		Lane	
Vertical distance driven (mm)	Depth (mm)	Number of blows	Vertical distance driven (mm)	Depth (mm)	Number of blows
50	50	0.5	1050	1050	7
100	100	0.5	1100	1100	6
150	150	1	1150	1150	11
200	200	1	1200	1200	18
250	250	4	1250	1250	
300	300	7	1300	1300	
350	350	8	1350	1350	
400	400	9	1400	1400	
450	450	7	1450	1450	
500	500	10	1500	1500	
550	550	8	1550	1550	
600	600	7	1600	1600	
650	650	8	1650	1650	
700	700	9	1700	1700	
750	750	7	1750	1750	
800	800	4	1800	1800	
850	850	5	1850	1850	
900	900	8	1900	1900	
950	950	7	1950	1950	
1000	1000	8	2000	2000	



Test Remarks

Tested By	EWDA	Date	16/01/2019
Data Entry By	TASR	Date	1/02/2019
Checked By	RWM	Date	7/02/2019



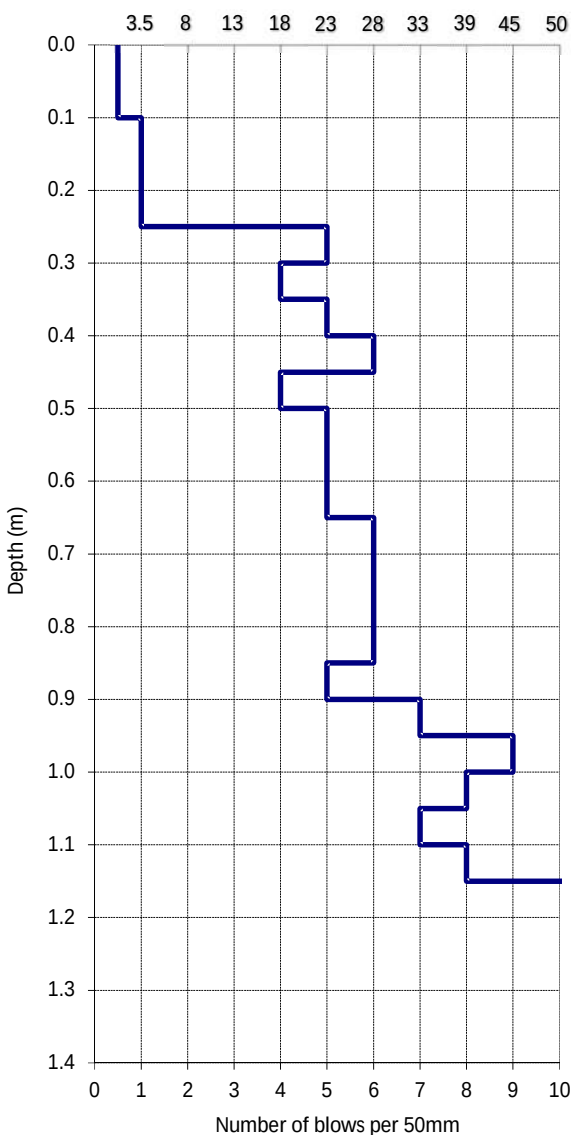
HAND AUGER LOG

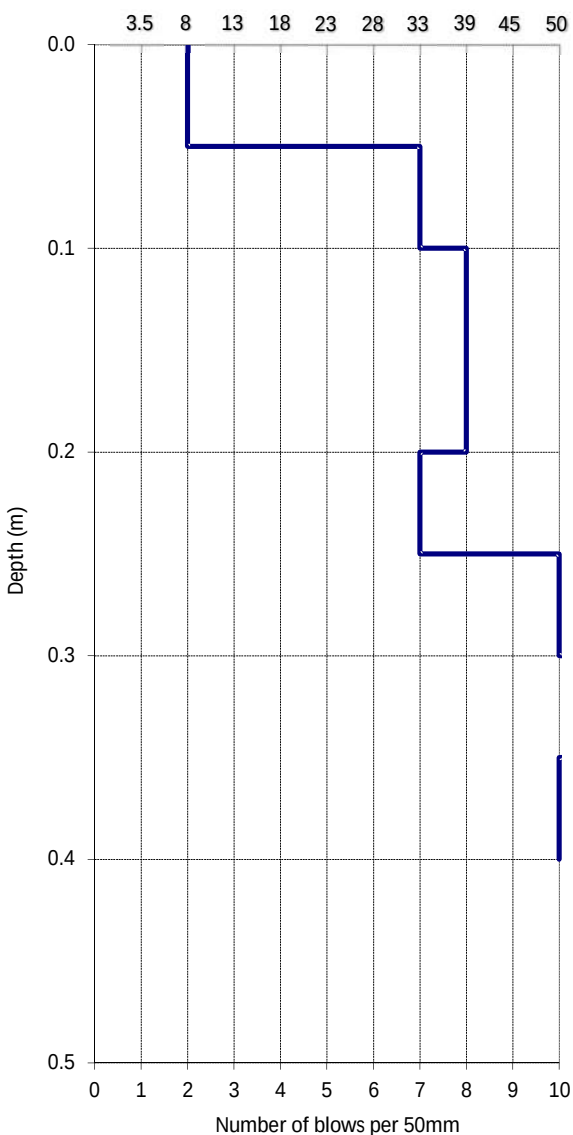
HOLE Id: Lot 492

Hole Location: See Location Plan

SHEET: 1 OF 1

PROJECT: GTG AURECON TERRACE VIEWS STG 3 D&E - SI										LOCATION: Terrace View, Papamoa										JOB No.: 1009168.0000																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																									
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Document prepared by

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Swaziland, Tanzania, Thailand, Uganda,
United Arab Emirates, Vietnam.