

## 171 TAYLOR STREET, CAMBRIDGE

PRELIMINARY GEOTECHNICAL ASSESSMENT

> PROJECT NO: HD1934 KOTARE CONSULTANTS REFERENCE: PGR 09 APRIL 2021

## **Executive summary**

## Introduction

Errol Hall has engaged us to undertake a preliminary geotechnical assessment for the site located at 171 Taylor Street, Cambridge. They propose to subdivide the site to create 1 additional lot. This report is intended to be submitted to the Waipa District Council in support of the subdivision consent application.

## Our scope included

- a desktop study of the site to review existing information, including historical aerial images, geology maps, contour maps, and the NZ Geotechnical Database (NZGD)
- a hand auger investigation, including 1 hand auger with strength testing and 2 soakage tests
- a natural hazards assessment, including a qualitative liquefaction assessment
- preliminary stormwater recommendations
- preliminary foundation recommendations

## Our key findings were

- ground conditions were consistent with the mapped Hinuera Formation
- the degree of liquefaction induced ground damage is likely to be 'none to minor'
- disposal of stormwater via soakage is not viable for the proposed lot and we recommend a detention type system
- the strength requirements for 'good ground' in accordance with NZS3604:2011 are not achieved due to low strength soils encountered
- deepened foundations are suitable for a suspended timber floor structure
- standard foundations (according to NZS3604:2011) with ground improvements to approx. 1.3 m below ground level or a raft type foundation designed for low bearing capacity are suitable for a concrete floor building

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## Introduction

Errol Hall proposes to subdivide the property located at 171 Taylor Street, Cambridge to create 1 new lot. We have been engaged to undertake a preliminary geotechnical assessment for the new lot. This report presents the results of our investigation and assessment for the proposed lot (Lot 2). A site plan showing the subdivision scheme is included in Appendix A.

This report is intended to be submitted to the Waipa District Council in support of an application for subdivision consent. Further testing and assessment will be needed at building consent stage.

## Scope

The scope of our assessment included a:

- desktop study of the site to review existing information, including historical aerial images, geology maps, contour maps, and the NZ Geotechnical Database (NZGD)
- hand auger investigation, including 1 hand auger with strength testing and 2 soakage tests
- natural hazards assessment, including a qualitative liquefaction assessment
- preliminary stormwater recommendations
- preliminary foundation recommendations

## Site description

The site is located at 171 Taylor Street, Cambridge. It is bounded by Taylor Street to the north and residential houses to all other boundaries. The site is flat and approximately 68 m above local datum. An existing dwelling is located on the northern side of the property within the proposed Lot 1 and a shed which is to be removed on the southern side within the proposed Lot 2. There is a gully and stream system located approximately 650 m to the south at approximately 22 m above local datum<sup>1</sup>.

## Desk study

## Geological setting

The geology map of the area<sup>2</sup> indicates that the site is underlain by the Hinuera Formation, which is described as cross-bedded pumice sand, silt and gravel with interbedded peat.

## Aerial photography

We have sourced historic photos of the site from Retrolens<sup>3</sup> and Google Earth<sup>4</sup>. Clear photos were available from 1939. The photos show that no significant changes were present until between 1953 and 1967 when the existing house on the property was built. No significant changes were seen on the site after 1967.

<sup>&</sup>lt;sup>1</sup> Waikato Regional Council Contours, Local Maps, <a href="https://waikatomaps.waikatoregion.govt.nz/Viewer/?map=8d6d6fda779b4e59951953ae97d0ec4a">https://waikatomaps.waikatoregion.govt.nz/Viewer/?map=8d6d6fda779b4e59951953ae97d0ec4a</a>, Accessed 23/03/2021.

<sup>&</sup>lt;sup>2</sup> 1:250,000 Geological Map of New Zealand. New Zealand Geology Web Map. GNS, 2013. http://data.gns.cri.nz/. Accessed 23/03/2021.

<sup>3</sup> http://retrolens.nz/

<sup>&</sup>lt;sup>4</sup> Google Earth Pro

## **NZGD**

We have reviewed the NZ Geotechnical Database (NZGD) in the area of the site. The database has 2 sites within 200m with 8 hand augers in total. Ground conditions on these sites confirmed the geology mapped and encountered.

## Site investigation

Our site investigation included a site walkover, 1 hand augers and 2 soakage tests. Soil logs and site plan can be seen in Appendix B.

## **Ground conditions**

The materials we encountered on site were consistent with the mapped Hinuera Formation. Ground conditions on site consisted of:

- topsoil to 0.3 m below ground level (bgl)
- stiff clay with some silt to 0.7 m bgl
- loose silty sand to 0.9 m bgl
- stiff silty clay to 1.2 m bgl
- medium dense to dense sand to 3.0 m bgl

## Groundwater

Groundwater was not encountered during this investigation up to a depth of 3.0 m bgl. Groundwater is expected to be at a depth greater than 4 m bgl.

## Geotechnical assessment

## Natural hazards

- Earthquake: The site subsoil class is D 'Deep or soft soils'. For Earthquake induced liquefaction see liquefaction section below.
- Tsunami, volcanic, geothermal, or sedimentation activity: The site is not near any known sources of these risks.
- Landslips: The site is level with no anticipated instability
- Erosion: No indications of erosion were observed during the site investigation and we consider the site to be at low risk of damage due to erosion.
- Subsidence: Risk of the site to general subsidence is low.

## Liquefaction

We have undertaken a qualitative liquefaction assessment for the site using the latest guidelines. This assessment qualifies as a Level B (calibrated desk study) in accordance with the 2017 planning guidelines<sup>5</sup>. Class D soils have been assumed for this assessment.

A desktop study, conducted by Tonkin and Taylor<sup>6</sup>, was carried out for the Hamilton City Council to help define the liquefaction risk within Hamilton City. The site is in the same geology as the HCC desk study. The sites 1/500 year event peak ground acceleration was calculated to be 0.25g<sup>7</sup>. The 1/1000

<sup>&</sup>lt;sup>5</sup> Earthquake Commission (EQC) / Ministry for the Environment and Ministry of Business, Innovation and Employment. 'Planning and engineering guidance for potentially liquefaction prone land' Rev 01. Dated September 2017.

<sup>&</sup>lt;sup>6</sup> 'Liquefaction Desktop Study' prepared by Tonkin & Taylor for Hamilton City Council, Ref No. 1007144.v1.1, dated February 2019

 $<sup>^{7}</sup>$  New Zealand Transport Agency (October 2018). Bridge Manual (SP/M/022). Third edition, Amendment 3

year peak ground acceleration used in the HCC desk study is 0.28g. The HCC desk study 1/1000 year event can be used for a conservative assessment. The HCC outputs used can be seen in Appendix C.

As discussed above, the groundwater at the site is expected to be at depths greater than 4 m bgl. Using the 1/1000 year analysis in the desk study, for an expected groundwater of 4m, the degree of liquefaction induced ground damage is likely to be none to minor.

As the liquefaction effects are likely to be none to minor, no further consideration of liquefaction is needed.

## Preliminary stormwater assessment

We undertook a falling head permeability test in general accordance with NZBC Verification Method E1/VM4<sup>8</sup> to determine the soakage capacity of the soils within the site. Tests were undertaken on the northern end of the access area and the south western corner of Lot 2.

Typical ground conditions within the soakage tests consisted of topsoil underlain by silts and sands consistent with the Hinuera Formation.

We determined percolation rates to be the following and can be seen in Appendix B:

Soakage test	Depth of test (m bgl)	Soakage rate (mm/hr)	Design soakage rate (mm/hr)*
ST01	2.0	268	133
ST02	2.0	182	90

<sup>\*</sup> a 50% reduction rate has been applied as per the RITS<sup>9</sup>

The design soakage rates shown are below the minimum soakage threshold of 150 mm/hr specified in the RITS. Based on the soakage rate and soils encountered we consider disposal of stormwater via soakage not viable for the proposed lot. We recommend a detention type system with a slow release to the kerb.

## Preliminary foundation recommendations

The strength requirements of 'good ground' according to NZS 3604:2011 were not met below the topsoil across the site up to a depth of 1.3 m. For a suspended timber floor building we expect standard NZS3604:2011 foundations deepened to at least 1.3 m bgl to be suitable. For a concrete floor building we expect standard NZS3604:2011 foundations with ground improvements or a raft type foundation designed for a reduced bearing capacity to be suitable.

Ground improvements would include excavation to remove topsoil and any soft, loose or unsuitable material. Any excavation would be replaced with compacted hardfill. Our preliminary testing indicates excavation depth would be to approximately 1.3 m.

<sup>&</sup>lt;sup>8</sup> Ministry of Business, Innovation & Employment, 'Acceptable Solutions and Verification Methods, For New Zealand Building Code Clause, E1 Surface Water', 1st Edition Amendment 10, dated 1 January 2017

<sup>&</sup>lt;sup>9</sup> 'Regional Infrastructure Technical Specification' v1.0, Waikato Local Authority Shared Services, dated May 2018

## Summary

- ground conditions were consistent with the mapped Hinuera Formation
- the degree of liquefaction induced ground damage is likely to be 'none to minor'
- disposal of stormwater via soakage is not viable for the proposed lot and we recommend a detention type system
- the strength requirements for 'good ground' in accordance with NZS3604:2011 are not achieved due to low strength soils encountered
- deepened foundations are suitable for a suspended timber floor structure
- standard foundation (according to NZS3604:2011) with ground improvements or a raft type foundation designed for low bearing capacity are suitable for a concrete floor building

Further investigation, assessment and design will be required at building consent stage.

## Limitation

This report has been prepared for our client, Errol Hall, their professional advisers and the relevant local authority for the purposes detailed above and may not be relied on by any other party for any other purposes. This report contains a preliminary assessment to establish suitability for subdivision based on a site walkover and testing in discrete locations. Inferences about the conditions at the site have been made based on the testing undertaken and our understanding of the geological environment in which the site lies. We recommend that a suitably qualified engineer is engaged to undertake further testing and assessment for building consent, and to observe works during the site preparation.

# **APPENDIX A – SUBDIVISION SCHEME**

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HD1934 | 171 Taylor Street, Cambridge | Reference: PGR | A



# APPENDIX B – SITE INVESTIGATION RESULTS







Job No.: **INVESTIGATION LOG** HD1938 Errol Hall Client: No.: HA01 Project: 171 Taylor St, Cambridge 26.03.21 Date: Northwestern corner of Lot 2. Location: Logged By: AM Co-ordinates: 1817917mE, 5804569mN Checked By: BS Elevation: Ground Vane Shear Strength Geology Depth (m) Legend Scala Penetrometer Water **Geological Interpretation** (kPa) (Blows / 100 mm) (refer to separate Geotechnical and Geological Information sheet for further information) Vane: 2108 150 200 10 12 14 16 18 5 TOPSOIL; dark brown. Dry; rootlets. 3 4 4 CLAY, with some silt; yellowish brown. Stiff; dry; moderate 0.4 plasticity. Loam 5 3 3 Silty SAND; light greyish yellow. Loose; dry; low dilatency; uniformly 0.8 graded; sand, fine. 2 3 Silty CLAY, with some sand; white with minor orange mottles. Stiff to very stiff; dry; moderate plasticity; sand, fine. 3 3 3 SAND, with minor gravel; greyish brown. Medium dense to dense; Groundwater Not Encountered moist; well graded; sand, coarse; gravel, fine. 6 1.2 m - 1.3 m: Changes to Silty SAND; brownish orange. 5 Hinuera Formation 5 8 10 2.2 12 10 11 9 8 2.7 m - 2.9 m: Changes to brownish orange. 9 8 EOH: 3.00 m Photo Remarks

NO 1936

NO

End of log at 3.0 m - target depth.

ZZZ Remoulded

Shear Vanes Water Investigation Type

■ Peak ■ Standing Water Level Hand Auger

▼ Standing Water Level
 Out flow
 In flow

Machine Borehole

Investigation Pit

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b
<b>GFO</b>

	INVESTIGATION LOG	Job No.:	
	INVESTIGATION LOG	HD	1938
Client: Errol	Hall	No.:	
Project: 171	Taylor St, Cambridge	S <sup>-</sup>	Γ01
Location:	Northern end of access are of Lot 2.	Date:	26.03.21
Co-ordinates:	1817934mE, 5804587mN	Logged By:	AM
Elevation:	Ground	Checked By:	BS

	Elevation. Ground					enconou by:	
Geology	Geological Interpretation (refer to separate Geotechnical and Geological Information sheet for further information)	Depth (m)	Legend	Scala Penetr (Blows / 100	mm)	Vane Shear Strength (kPa) Vane:	Water
			TS	2 4 6 8 10	12 14 16 18	50 150 200	
	TOPSOIL; brown; bedded. Rootlets.		TS TIS				
			TS TS TE				
Topsoil		0.2	TS TIS				
-			TS " " " "				
			# TS # # IS				
-		0.4	11 TS 11				
Loam	Sandy SILT; light yellowish brown. Medium dense; dry; moderate dilatency; sand, fine.	L	×				
으							
	Sandy SILT; light greyish yellow. Medium dense to dense; dry;	0.6	×				
	moderate plasticity.	L -					
		1					
		8.0_					Ъ
		+ -	ļ 				ounter
	SAND; dark yellowish brown. Medium dense to dense; moist; well graded; sand, coarse.	1.0					Groundwater Not Encountered
							iter No
							empur
_		1.2					Grot
Formation							
ra For		-					
Hinuera		1.4_					
	Silty SAND, with minor clay, with trace gravel; light grey. Dense; moist; well graded; sand, coarse; gravel, fine.						
	, g,, g,		×				
		1.6					
			x :: x				
		1.8					
		L _					
	EOH: 2.00 m						
	201. 2.00 11	2.0_					
		<u> </u>					
	Photo				Remarks		
1 -			1				

HO 1936

STOLL

ACT 3 East

G 1 20

General Control

Gene

End of log at 2.0 m - target depth.

ZZZ Remoulded

Shear Vanes Water Investigation Type

Peak ▼ Standing Water Level Hand Auger

$\checkmark$	Machine Borehole			
	Page 1 of 1			

Investigation Pit

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bal
GFO

INIVESTICATION LOC	Job No.:		
INVESTIGATION LOG	HD1938		
Client: Errol Hall	No.:		
Project: 171 Taylor St, Cambridge	S <sup>-</sup>	T02	
Location: Southeastern corner of Lot 2.	Date:	26.03.21	
Co-ordinates: 1817935mE, 5804559mN	Logged By:	AM	
Elevation: Ground	Checked By:	BS	

Geology	Geological Interpretation (refer to separate Geotechnical and Geological Information sheet for further information)	Depth (m)	Legend	Scala Penetrometer (Blows / 100 mm)  Vane Shear Strength (kPa) Vane:	Water
ြ	iniornation sneet for further information)	٥		2 4 6 8 10 12 14 16 18	
Topsoil	TOPSOIL; dark brown. Rootlets.		18 T T T T T T T T T T T T T T T T T T T		
Loam	SILT, with some sand, with minor clay; brownish orange. Medium dense; dry; low dilatency.	0.2	TS W		
	Sandy SILT, with some clay; light greyish yellow. Medium dense to dense; dry; moderate plasticity.	0.4			
		0.6			
		0.8			7
					Groundwater Not Encountered
ion		1.0			ındwater Not
Hinuera Formation	SAND, with trace gravel; light greyish brown. Dense; moist; well graded; sand, coarse; gravel, fine.	1.2			Grou
Η̈́		1.4			
		-			
		1.6	1		
		1.8			
	EOH: 2.00 m				

HD 1939

THE STO2

LEAD

26 05 2621

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Photo

End of log at 2.0 m - target depth.

ZZZ Remoulded

 Shear Vanes
 Water
 Investigation Type

 ■ Peak
 ▼ Standing Water Level
 Hand Auger

Remarks

→ Out flow

☐ In flow

	investigation Pit
7	Machine Borehole

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 Job name
 171 Taylor Street

 Job number
 HD1938

 Date
 31/03/2021

**Plotted by** BK **Reviewed by** BS

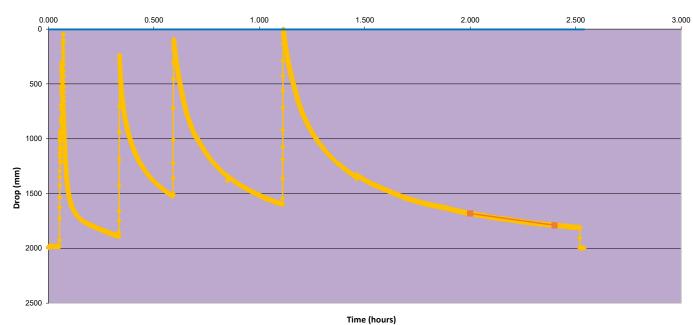
## Perc test results ST01

Calculated in general accordance with E1/VM1 - Surface Water, Section 9.0

Hole depth: Ground water depth:

2.000 m None m

Notes:



### Percolation rate calculation:

Time (Hour)	Drop (mm)			
2.00	1683	Minimum slope (lower)	Percolation rate =	268 mm/hr
2.40	1790	Minimum slope (upper)	50% Percolation rate =	133 mm/hr

Document Set ID: 10624160 Version: 1, Version Date: 28/05/2021



171 Taylor Street Job name HD1938 Job number 31/03/2021 Date

Plotted by ВК

Reviewed by BS

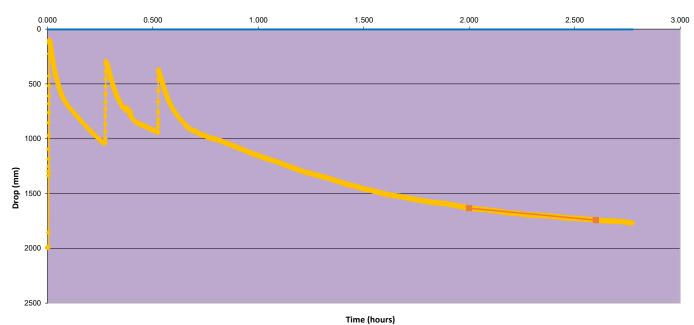
## Perc test results ST02

Calculated in general accordance with E1/VM1 - Surface Water, Section 9.0

Hole depth: Ground water depth:

2.000 m None m

Notes:



#### Percolation rate calculation:

Time (Hour)	Drop (mm)			
2.00	1634	Minimum slope (lower)	Percolation rate =	182 mm/hr
2.60	1743	Minimum slope (upper)	50% Percolation rate =	90 mm/hr

Document Set ID: 10624160 Version: 1, Version Date: 28/05/2021

# APPENDIX C – HCC LIQUEFACTION OUTPUTS

hdgeo.co.nz

HD1934 | 171 Taylor Street, Cambridge | Reference: PGR | C

#### CPT based liquefaction analysis Scenario: 1000 yr ARI (Mw=5.9,PGA = 0.28g) 4m GWD Histograms of LSN values by geomorphic group Low Hills (n=176) Recent Gullies/River Terraces (n=114) 120 25 100 20 CPT Count CPT Count 80 15 60 10 40 5 20 0 15-18 18-21 24-27 27-30 30-33 33-36 39-42 42-45 0-3 3-6 6-9 15-18 18-21 21-24 24-27 27-30 30-33 33-36 36-39 38-42 42-45 45-48 LSN LSN Peat (n=94) Alluvial Plains (n=514) 25 100 20 80 CPT Count CPT Count 15 60 10 40 5 20 0-3 3-6 6-9 9-12 112-15 115-18 21-24 27-27 27-30 33-33 33-36 33-42 42-45 42-45 21-24 24-27 27-30 30-33 33-36 36-39 39-42 LSN LSN Box and whisker plot of LSN values by geomorphic group Low Hills Geomorphic Group Recent Gullies/ River Terraces Peat: Alluvial Plains 20 30 40 0 10 50 LSN **Notes** Legend 1. Refer to Table 2.2 and Appendix A of the MBIE liquefaction guidance Degree of liquefaction Characteristic LSN values document for further information about land damage categories. induced ground damage $(P_L=15\%)$ 2. Liquefaction analyses are undertaken assuming a probability of None to Minor 0-13 liquefaction (P<sub>I</sub>) of 15%. (Refer to section 4.2 for further detail) Minor to Moderate 13-18 3. Box and Whisker plot key: Moderate to Severe >18 These values are intended only for use in area-wide hazard assessment using the MBIE (2017) performance criteria. Different values may be

Min

Median

more appropriate for other purposes (such as site-specific design)



## **APPENDIX D**

**CCTV REPORT** 



- JETTING & CLEANING WORKS
- CCTV MAINLINE UPTO 2.3M DIAMETER
- CCTV PUSH ROD CAMERAS
- TRACE AND LOCATING
- SUBDIVISONS AND FAULT DIAGNOSIS
- VACCUM LOADING AND SEPTIC TANKS

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## **Documentation For**

## **CCTV-inspection**

Contractor Assassin Drainage Solutions

Ltd

Client Kotare Consultants

Contract No.

**Project** 171 Taylor Street

Standard New Zealand Pipe Inspection

created with can3D®

Comments:

Disclaimer:Assassin Water Jetters Ltd accepts no liability for peg markings or any other markings once Assassin have left the property/site.

The Attached Photos/CCTVMAP is a reference only and should in no circumstances be used as a site plan for further work

If piling within 3m of pipes a Manual visual inspection is required to verify pipe location

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- JETTING & CLEANING WORKS
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## Measurements report

Operator Client Aaron

Kotare Consultants

Contract No.

Contractor Assassin Drainage Solutions Ltd

Length of all pipes 0.00 24.80
Sum of all images 14

No.	Upper point	Lower point	Date	Street	Material/Dia	Kind of water	m	Insp	
1	171 Taylor St…	1332648	26/02/20	171 Taylor Street	EW /100	Foul/sanitary se…		24.8	14
EW - 100/100								24.8	14

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- JETTING & CLEANING WORKS
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Comments

## **Pipe Summary Report**

Inspection Completion Operator

**Time Started Date Started** 26/02/2021

Inspection Complete

4:07 PM

Client **Camera Operator** Kotare Consultants Aaron

**New Zealand Pipe Inspection** 

Contract No.

Surface

**Currency of Inspection Current Inspection** Weather

Status of Pipe

Drawing No. Dry Weather **Original Condition** Inspection No. Asset No. 171 Taylor Street Set-up upstream

Circular Pipe Shape Material Earthenware

**Upper Node** 171 Taylor Street Line Length (m) **Lower Node** 1332648

Use Foul/sanitary sewer

Surveyed Length (m) 24.8 Mown Lawn Pipeline Height (Diameter in mm) Location Private Property - Yard

Grading Service Service Peak Service Mean Service Total Grading Structural Structural Mean Structural Peak Structural Total 50.0 4.1 101.0 10.0 1.0 25.0

### 171 Taylor Street

$\downarrow$	1/1 Taylor Stre	1/1 Taylor Street					
	0.00m (25.49m)	IS	00:05	Inspection Start			
	1.63m (23.86m) 2 5	JFL	00:17	Joint, Faulty, Large, 3-11 o'clock, Joint has root intrusion .			
	1.63m (23.86m) 5	RIS	00:17	Root Intrusion, Small, 8-3 o'clock			
	2.31m (23.18m) 1	JFS	00:21	Joint, Faulty, Small, 8-3 o'clock, Joint has root intrusion .			
	2.31m (23.18m) 5	RIS	00:21	Root Intrusion, Small, 8-3 o'clock			
ļ	2.86m (22.63m) 2 5	JFL	00:27	Joint, Faulty, Large, 12-12 o'clock, Circumferential crack in joint zone and root intrusions.			
	2.86m (22.63m) <mark>5</mark>	RIS	00:27	Root Intrusion, Small, 12-12 o'clock			
	3.50m (21.99m) 2 5	JFL	00:31	Joint, Faulty, Large, 9-3 o'clock, Joint has root intrusion .			
	3.50m (21.99m) 5	RIS	00:32	Root Intrusion, Small, 9-3 o'clock			
	5.49m (20.00m) 2 5	JFL	00:42	Joint, Faulty, Large, 9-3 o'clock, Joint has root intrusion .			
	5.49m (20.00m) 5	RIS	00:42	Root Intrusion, Small, 9-3 o'clock			
	6.59m (18.90m)	MC	00:49	Material Change, EW TO PVC			
	6.59m (18.90m) 0	JDS	00:50	Joint, Displaced, Small, Jointing method has moved .			
	6.81m (18.68m)	LR	00:54	Line Deviates, Right			
	7.98m (17.51m)	LL	00:59	Line Deviates, Left			
	8.24m (17.25m)	MC	01:06	Material Change, PVC TO EW			
	8.24m (17.25m) 0	JDS	01:06	Joint, Displaced, Small			
	23.77m (1.72m)	CF	02:41	Construction Feature, 11-1 o'clock Inspection Lid.			
	24.91m (0.58m)	LD	02:49	Line Deviates, Down			
	25.49m (0.00m)	IE	02:56	Inspection Ends, Connection to Main .			

1332648

# ASSOSSIN PANAGE SOLUTIONS LID

## "It's A Dirty Job But Someone Has To Do It!"

- JETTING & CLEANING WORKS
- CCTV MAINLINE UPTO 2.3M DIAMETER
- CCTV PUSH ROD CAMERAS
- TRACE AND LOCATING
- SUBDIVISONS AND FAULT DIAGNOSIS
- VACCUM LOADING AND SEPTIC TANKS

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Image report pipe

Time Started

Date Started

Asset No. 171 Taylor Street
Upper Node 171 Taylor Street

Lower Node 1332648
Pipeline Height (Diameter in... 100

Pipeline Width (mm)

Contract No.

Parallel Line

4:07 PM

26/02/2021

Line Length (m)

Material Shape Earthenware Circular Pipe

Joint Spacing

Joint S

Operator

Surveyed Length (m)

Camera Operator Aaron

Currency of InspectionCurrent InspectionInspection CompletionInspection Complete

Set-up upstream

Inspection No. 1

Inspection Start

Position 0.00m

Code abbreviations IS

Damage category



Joint, Faulty, Large, 3-11 o'clock, Joint has root intrusion .

Position 1.63m

Code abbreviations JFL

Damage category 25





- JETTING & CLEANING WORKS
- CCTV MAINLINE UPTO 2.3M DIAMETER
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- SUBDIVISONS AND FAULT DIAGNOSIS
- VACCUM LOADING AND SEPTIC TANKS

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#### Contract No.

# Image report pipe

171 Taylor Street Asset No. 171 Taylor Street **Upper Node** Material 1332648 **Lower Node** 

Pipeline Height (Diameter in... 100

Pipeline Width (mm)

Line Length (m)

Earthenware Shape Circular Pipe

**Joint Spacing** 

Joint, Faulty, Small, 8-3 o'clock, Joint has root intrusion.

Position 2.31m

Code abbreviations **JFS** 

Damage category



Joint, Faulty, Large, 12-12 o'clock, Circumferential crack in joint zone and root intrusions.

Position 2.86m

Code abbreviations JFL

Damage category



Document Set ID: 10624160

Version: 1, VN csites protest 28/9/5/28/92 for any measurements.



- JETTING & CLEANING WORKS
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- CCTV PUSH ROD CAMERAS
- TRACE AND LOCATING
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#### Contract No.

Asset No.

**Upper Node** 

# Image report pipe

Line Length (m)

171 Taylor Street 171 Taylor Street

Material

1332648 **Lower Node** Pipeline Height (Diameter in... 100

Earthenware Shape Circular Pipe

Pipeline Width (mm)

**Joint Spacing** 

Joint, Faulty, Large, 9-3 o'clock, Joint has root intrusion.

Position 3.50m

Code abbreviations JFL

Damage category



Joint, Faulty, Large, 9-3 o'clock, Joint has root intrusion .

Position 5.49m

Code abbreviations JFL

Damage category



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#### Contract No.

Asset No.

**Upper Node** 

**Lower Node** 

# Image report pipe

Line Length (m)

171 Taylor Street 171 Taylor Street

1332648 Pipeline Height (Diameter in... 100

Material Shape

Earthenware Circular Pipe

Joint Spacing

Pipeline Width (mm)

Material Change, EW TO PVC

Position

6.59m

Code abbreviations MC

Damage category



Joint, Displaced, Small, Jointing method has moved.

Position

6.59m

Code abbreviations JDS

Damage category





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#### Contract No.

Asset No.

# Image report pipe

Line Length (m)

171 Taylor Street 171 Taylor Street

**Upper Node** 1332648 **Lower Node** Pipeline Height (Diameter in... 100

Pipeline Width (mm)

Material Earthenware Shape Circular Pipe

**Joint Spacing** 

Line Deviates, Right

Position 6.81m

Code abbreviations LR

Damage category



Line Deviates, Left

Position 7.98m

Code abbreviations LL

Damage category



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#### Contract No.

# Image report pipe

171 Taylor Street Asset No. 171 Taylor Street **Upper Node** 

1332648 **Lower Node** Pipeline Height (Diameter in... 100

Pipeline Width (mm)

Line Length (m)

Material Earthenware

Shape Circular Pipe

**Joint Spacing** 

Material Change, PVC TO EW

Position 8.24m

Code abbreviations MC

Damage category



Construction Feature, 11-1 o'clock Inspection

Position 23.77m

Code abbreviations

Damage category



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#### Contract No.

Asset No.

**Upper Node** 

# Image report pipe

171 Taylor Street 171 Taylor Street

1332648 **Lower Node** Pipeline Height (Diameter in... 100

Pipeline Width (mm)

Line Length (m)

Material Earthenware Shape Circular Pipe

**Joint Spacing** 

Line Deviates, Down

Position 24.91m

Code abbreviations LD

Damage category



Inspection Ends, Connection to Main .

Position 25.49m

Code abbreviations ΙE

Damage category



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## **APPENDIX E**

**Application Plan** 

